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DEVELOPMENT OF SURGE II PROGRAM WITH APPLICATION TO THE SABINE---ETC(U)

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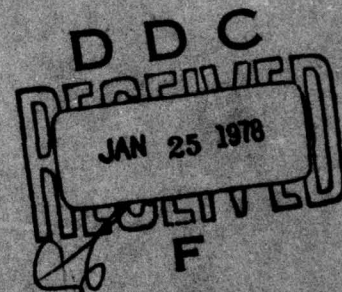
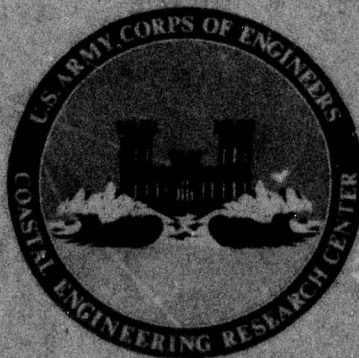
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Development of Surge II Program With Application to the Sabine-Calcasieu Area for Hurricane Carla and Design Hurricanes

by

Robert O. Reid, Andrew C. Vastano, and Thomas J. Reid

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hurricane surges, is made for the Sabine-Calcasieu region which straddles the Texas and Louisiana boundary. For normal tide conditions, cities such as Beaumont, Orange, and Lake Charles are connected to the sea via rivers, which in the numerical model must be represented as subgrid scale channels as long as the basic grid scale is of the order of a nautical mile. Under hurricane surge conditions, however, the overland flooding can greatly expand their connection to the sea.

Calibration of channel friction is carried out via the astronomical tide simulation. Calibration of the block friction is carried out using data on a previous storm of record, Hurricane Carla. An example application is provided for standard project hurricanes (SPH). The response for a large radius SPH of slow speed and one of moderate speed of translation is examined. Also, the effect of rainfall is examined by running the latter storm with and without rainfall.

PREFACE

This report is published to assist coastal engineers in the study of storm surge and inland flooding for use in the planning and design of protective coastal works. The work was carried out under the coastal processes program of the U.S. Army Coastal Engineering Research Center (CERC).

The report was prepared by Robert O. Reid, Andrew C. Vastano, and Thomas J. Reid, Coastal Studies, Inc., who are also on the staff of the Department of Oceanography, Texas A & M University, College Station, Texas, under CERC Contract No. DACW64-74-C-0015 to the U.S. Army Engineer District, Galveston.

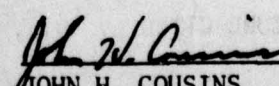
The authors acknowledge the help of many individuals of the Galveston District, in providing most of the data necessary in schematizing the Sabine-Calcasieu system, the data for tidal calibration, the wind fields and observed water level data for Hurricane Carla, and the necessary input data for the Standard Project Hurricanes. G. Marinos and M. Choate assisted with various stages of the development and carried out the runs for the Standard Project Hurricane via the GE series 400 computer.

G. Marinos was the Galveston District contract monitor for the report under the general supervision of S. Tanner, Chief, Coastal Planning Section. Dr. Jon Hubertz was the CERC technical monitor of the report under the general supervision of Dr. D.L. Harris, Chief, Coastal Oceanography Branch, Research Division.

Comments on this publication are invited.

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CONVERSION FACTORS, U.S. CUSTOMARY TO METRIC (SI)
UNITS OF MEASUREMENT

U.S. customary units of measurement used in this report can be converted to metric (SI) units as follows:

Multiply	by	To obtain
inches	25.4	millimeters
	2.54	centimeters
square inches	6.452	square centimeters
cubic inches	16.39	cubic centimeters
feet	30.48	centimeters
	0.3048	meters
square feet	0.0929	square meters
cubic feet	0.0283	cubic meters
yards	0.9144	meters
square yards	0.836	square meters
cubic yards	0.7646	cubic meters
miles	1.6093	kilometers
square miles	259.0	hectares
knots	1.8532	kilometers per hour
acres	0.4047	hectares
foot-pounds	1.3558	newton meters
millibars	1.0197×10^{-3}	kilograms per square centimeter
ounces	28.35	grams
pounds	453.6	grams
	0.4536	kilograms
ton, long	1.0160	metric tons
ton, short	0.9072	metric tons
degrees (angle)	0.1745	radians
Fahrenheit degrees	5/9	Celsius degrees or Kelvins ¹

¹To obtain Celsius (C) temperature readings from Fahrenheit (F) readings, use formula: $C = (5/9) (F - 32)$.

To obtain Kelvin (K) readings, use formula: $K = (5/9) (F - 32) + 273.15$.

SYMBOLS AND DEFINITIONS

A	cross-sectional area of a channel
A_b	effective surface area of a block
A_c	cross-sectional area of a channel
A_s	surface area of an estuary at MSL
a_0	amplitude of input tide to an estuary
B	$8/3\pi m(A_s\omega)^2 a_0$, a parameter which determines the phase lag of tidal response in an estuary
BN	right-hand side of equation (48)
BP	right-hand side of equation (46)
b	$(\partial A/\partial s)_H$ const., a characteristic of a channel
C_d	dimensionless discharge coefficient characterizing a constricted opening between bay and sea
C_g	admittance coefficient (with dimensions of velocity); nominally represents the wave speed in the sea
C_o	dimensionless overflow coefficient (generally less than 0.5 for a broad-crested barrier)
C_s	dimensionless discharge coefficient for a submerged barrier (generally less than $\sqrt{2}$)
D	total depth of water at position x,y at time t
\bar{D}	a mean depth for the effective fetch across a block; also mean depth for a channel $(D_N + D_p)/2$
D_b	depth of water over the crest of a barrier
D_c	effective depth of a channel A_c/w
D_{max}	maximum depth to be expected anywhere in the system during a storm surge
F_L	contribution to the forcing term in equation (17) due to lateral transfer of mass and momentum
f	dimensionless bed resistance coefficient for blocks
f_c	channel bed friction coefficient

SYMBOLS AND DEFINITIONS--Continued

G	damping factor for channels, see equation (44)
G_1	damping factor for x-transport on blocks, see equation (35)
G_2	damping factor for y-transport on blocks, see equation (36)
g	acceleration due to gravity
H	water level elevation relative to local MSL datum
H _B	water elevation on the water-connected block of a channel
H _C	common water elevation for a channel junction
H _M	mean water level anomaly of connected channel and blocks
H _X	water level at the lower end of an x-channel
H _Y	water level at the left end of a y-channel
H _A	H at point B in a channel
H _b	water level on the high side of a barrier
H _g	input tide level at time t outside a bay entrance
H(i,j)	water level anomaly H for block identified by x and y indexes i,j
H*	tentative predicted H for a ponding block in the absence of any contribution by longitudinal discharge to or from the channel which terminates adjacent to that block
H'	value of H at new time level
H' _p	new H value at point p in channel
H ₁ & H ₂	water levels on the two sides of a barrier (both of which exceed Z _b), equation (10)
i	x-index for grid blocks
j	y-index for grid blocks
K	dimensionless wind-stress coefficient, equation (6)
L	effective fetch length

SYMBOLS AND DEFINITIONS--Continued

L_f	net time rate of gain of water volume per unit distance along the channel by lateral transfer and rainfall
L_m	net time rate of gain of momentum (divided by water density) per unit distance along channel
m	$fL/gD_c A_c^2$ or $1/g(C_d A_d)^2$
N	denotes negative characteristic
n	time index
P	wind "push" term $X\Delta t$ or $Y\Delta t$; also denotes positive characteristic
Q	volume transport through cross-sectional area of a channel
\bar{Q}	mean Q value for channel, equation (45)
$QCXP_K$	flow at the upper end of an x-channel for channel block K
$QCYN_K$	flow at the left end of a y-channel for channel block K
$QCYP_K$	flow at the right end of a y-channel for channel block K
$QCXN_K$	flow at the lower end of an x-channel for channel block K
Q_A	Q at point A of positive characteristic
Q_B	Q at point B of negative characteristic
Q_d	discharge from channel to ponding block
q_f	the flow (per unit length of channel) from the channel to the adjacent block
q_i	lateral volume flux per unit length into the channel
q_n	outward component of volume flux at a boundary
q_o	lateral volume flux per unit length out of the channel
q_t	flow (per unit length of channel) from the channel block to the channel (across the interior side of the channel)
Q'	new Q value
Q'_N	new Q at point N

SYMBOLS AND DEFINITIONS--Continued

Q_p	new Q at point P
Q_r'	specified river discharge
R	rainfall rate
$R(i,j)$	rainfall rate for block i,j
r	relative amplitude response
s	distance along the axis of a channel
T	tidal period
T_s	longitudinal component of wind stress (divided by water density) or appropriate wind-stress component (X or Y) corresponding to time level t for the associated channel block
t	time
U	vertically integrated x-component of volume transport per unit width
$UCF(K)$	lateral transport, per unit width per unit time, nominally from an x-channel of block K to an adjacent block; also denoted UCF_K
$UCT(K)$	lateral transport, per unit width per unit time, nominally to an x-channel from the interior of block I ; also denoted UCT_K
UN	U value on left side of block
$U(i,j)$	value of U at the left side of block i,j
$U(i+1,j)$	value of U at the right side of block i,j
u	typical fluid speed in the bay
U'	value of U at new time level
V	vertically integrated y component of volume transport per unit width
$VCF(K)$	lateral transport per unit width per unit time, nominally from an y-channel of block K to an adjacent block; also denoted VCF_K

SYMBOLS AND DEFINITIONS--Continued

VCT(K)	lateral transport per unit width per unit time, nominally to an y-channel from the interior of block K; also denoted VCT_K
VN_I	value of V at the lower side of a block
$V(i,j)$	value of V at the lower side of block i,j
$V(i,j+1)$	value of V at the upper side of block i,j
V'	value of V at new time level
W	windspeed at 10-meter elevation over the water
W_c	a critical speed taken as 14 knots (7 meters per second)
w	surface width of a channel (conveyance width)
X	x-component of the wind stress divided by the density of the water
$X(i+1,j)$	value of X for right side of block i,j
x	horizontal Cartesian coordinate nominally alongshore, positive to the right when facing shore
Y	y-component of the wind stress divided by the density of the water
$Y(i,j+1)$	value of Y for top side of block i,j
y	horizontal Cartesian coordinate nominally normal to shore, positive landward
Z	elevation of the seabed relative to MSL datum
$Z(i,j)$	value of Z for block i,j
Z_b	barrier crest elevation
Z_c	channel bed elevation
α	$(gD)^{1/2} \Delta t / \Delta s$ (Courant number); also $L_c / D_c A_c$, equation (77)
Γ	$L(C_b D_b)^2 / \overline{D} \Delta t$
ΔH	a head differential dependent upon barrier type
Δq	net lateral flow to the channel per unit length of channel

SYMBOLS AND DEFINITIONS--Continued

Δs	grid size for blocks (distance between successive H values in both the x and y directions); also written ΔS or DELS
Δt	time step (time interval between successive H values at given location); also written DELT
θ	the angle between the wind velocity vector and the x-axis
λ	$w (gD)^{1/2}/G$
π	3.14159 ...
σ	$wf Q /A^2$
ϕ	latitude
Ω	absolute angular speed of the earth
ω	radian frequency $2\pi/T$

DEVELOPMENT OF SURGE II PROGRAM WITH APPLICATION TO THE
SABINE-CALCASIEU AREA FOR HURRICANE CARLA AND DESIGN HURRICANES

by
Robert O. Reid, Andrew C. Vastano, and Thomas J. Reid

I. INTRODUCTION

Numerical techniques for the solution of equations representing storm surges in coastal areas were significantly augmented in 1966 by the development of a two-dimensional model (referred to in this study as the SURGE I program) for the U.S. Army Engineer District, Galveston (Reid and Bodine, 1968). At about the same time a number of bay models emerged. Notable among these are the models of Leenderste (1967) and Masch, et al. (1969), which have been applied to problems of both surge and circulation in bays. These models include the Coriolis force which is neglected in the Reid-Bodine model. However, the Reid-Bodine model produced the first successful inclusion of flooding, recession, barriers, and flow over barriers in the study of inundation of low-lying coasts. The actual model is a nonlinear system of equations and boundary conditions solved by numerical integration of time-dependent, forced motion. Its use produces the water response to stormwinds over the region for a given storm tide at the seaward boundary. The initial application was a hindcast of the Hurricane Carla surge generated in Galveston Bay during 9 to 12 September 1961.

During Hurricane Carla, the wetted perimeter of Galveston Bay essentially doubled, as accurately reproduced in the hindcast computations. Serial observations of water levels for the storm period available from stations throughout the bay were compared to levels computed with the numerical algorithm. These records produced a standard deviation of less than 4 inches, overall. The maximum deviation of the water level prediction was 1.5 feet and occurred at the grid square corresponding to the location of the Pelican Island Bridge which spans the channel between Galveston and the Pelican Islands. Although this disparity was relatively large, its effect on the computations was effectively reduced by the smoothing operation of the numerical integration. However, this difference points out a basic problem confronting any model--the minimum definition of topographic features.

The basic problem of indicating subgrid scale effects in numerical modeling is normally solved by parameterization of the omitted physical mechanism. Often, an analytic relationship is introduced that requires the specification of empirically derived constants; e.g., the wind-stress equation for the transfer of momentum from wind to water. Another simple and pertinent instance is the *a priori* rotation of wind vectors over certain grid squares in the Hurricane Carla computations for Galveston Bay. The model Galveston entrance channel was not in the proper orientation on the Cartesian numerical grid system and, as a result, did not admit a realistic amount of water to the bay. A programed shift in the wind vectors indicated this subgrid scale feature.

SURGE I has been applied to the study of Texas coastline surge susceptibility. The topographic features of this region are characterized by barrier islands and shallow, river-fed bay systems surrounded by near sea level land and marshes. The specific applications of the program have therefore centered interest on the immediate environs of a bay. The requirement for surge studies of appreciable distances inland from the bay system has only recently been placed on the numerical model. The propagation of the surge to higher ground through necessary subgrid scale topographic features has required an extension of the basic algorithm.

The new algorithm developed for the study of the Sabine-Calcasieu region is referred to as the SURGE II program. This program incorporates all the features of SURGE I with the further option of representing variable depth and width channels along the sides of each grid square. The flow computations for the channels interact with the normal grid square computations and permit a complete suite of flooding conditions for overtopping of levees. In this manner SURGE II provides a time-dependent, subgrid scale transport of water through the model.

II. THEORETICAL DEVELOPMENT FOR SURGE II

1. Summary of Two-Dimensional Theory.

The development of SURGE II was based on the SURGE I concept by Reid and Bodine (1968). A part of this study is presented here to provide a complete description of SURGE II.

The advection of momentum (or field acceleration) is considered negligible except at singular regions of the bay (submerged barriers and narrow channels) where the effect is included implicitly through the use of appropriate nonlinear discharge relations. The effect of the earth's rotation is also neglected; this approximation appears justifiable for systems of small spatial scale and shallow depth where frictional forces are more dominant.

Within the normal domain of the bay and immediate adjoining sea, the vertically integrated equations of motion and of continuity appropriate to the problem are taken as follows:

$$\frac{\partial U}{\partial t} + gD \frac{\partial H}{\partial x} = X - fqUD^{-2} \quad (1)$$

$$\frac{\partial V}{\partial t} + gD \frac{\partial H}{\partial y} = Y - fqVD^{-2} \quad (2)$$

$$\frac{\partial H}{\partial t} + \frac{\partial U}{\partial x} + \frac{\partial V}{\partial y} = R, \quad (3)$$

where

x and y = horizontal Cartesian coordinates;

t = time;

U and V = vertically integrated x and y components, respectively, of transport per unit width;

g = gravity;

H = water level elevation relative to the local mean sea level (MSL) datum;

D = depth of water at position x, y at time t;

q = magnitude of the transport per unit width;

f = dimensionless bed-resistance coefficient;

R = rainfall rate;

X and Y = x and y components of the wind stress divided by the density of the water (the density assumed constant).

Normal values of f are in the range 10^{-3} to 10^{-2} for typical seabed conditions.

The value of q is obtained from U and V by

$$q = (U^2 + V^2)^{\frac{1}{2}} \quad (4)$$

which is a positive quantity.

The kinematic forms of the wind-stress components in the absence of rainfall are taken as

$$\begin{aligned} X &= K W^2 \cos \theta \\ Y &= K W^2 \sin \theta, \end{aligned} \quad (5)$$

where W is the windspeed at a 10-meter elevation over the water, and θ is the angle between the wind velocity vector and the x-axis. The dimensionless coefficient, K, used in the calculations is presumed to be a function of windspeed as implied by the van Dorn (1953) relation for wind stress. Specifically, it is assumed that

$$\begin{aligned} K &= K_1 && \text{for } W \leq W_c \\ K &= K_1 + K_2 \left(1 - \frac{W_c}{W}\right)^2 && \text{for } W \geq W_c, \end{aligned} \quad (6)$$

where the constants K_1 and K_2 are taken as 1.2×10^{-6} and 1.8×10^{-6} , respectively, and W_c is a critical speed which is taken as 14 knots (7 meters per second). For large windspeeds, K approaches the limiting value of 3.6×10^{-6} which corresponds to a resistance coefficient of about 3.0×10^{-3} if the ratio of air density to water density is taken as 1.2×10^{-3} .

In the presence of rainfall an added flux of momentum proportional to RW occurs (van Dorn, 1953). The effect can be included by augmenting K by R/W . For heavy rainfall, the resulting K is increased about 10 percent.

The variables H and D are related by the simple expression,

$$D = H - Z, \quad (7)$$

where Z is the elevation of the seabed relative to the MSL datum. Presumably, Z is a function of x and y only; i.e., the time-dependent scour of the seabed is ignored.

The above equations ignore the direct effect of variable atmospheric pressure which is relatively minor in a small, shallow bay. The effect over the sea is included implicitly through the specification of an appropriate surge height versus time in the adjoining sea where the combined effects of winds and differential atmospheric pressure give rise to a coastal storm surge. This is presumed to be determined independently of the detailed calculations for the bay and enters as a boundary condition.

a. Boundary Conditions. Four different types of boundary conditions are used in this system of computations. Two of these conditions apply to the water-land boundary, one condition applies to the artificial boundary representing the seaward end of the bay system, and one applies at partial barriers internal to the system. (Additional internal conditions are needed in the presence of imbedded channels as discussed later in Section III,2.) All four conditions relate the normal component of flow at the boundary to the state of the water level at the boundary.

In general, the boundary between bay water and land depends on the water elevation and the land topography. The shoreline for different uniform elevations of the surface of the bay is readily established from a knowledge of the topography. For a bay with low-lying terrain, the rate of increase of surface area of water per unit increase of water level can be considerable. In the actual rising stage of storm tide the amount of inundation is controlled by the rate at which the water can flow into the potential ponding areas. In the present scheme, which uses a representation of the bay in terms of a discrete grid, the elevation of the seabed or land is regarded as uniform over each grid square, thus forming a two-dimensional, staircase-type approximation of the actual topography. The boundary condition on the normal component of flow, q_n , at the juncture of a flooded square and a dry square is taken as

$$q_n = 0, \quad (8)$$

if the elevation, H , of the water is less than that of the adjacent dryland. However, if the water level is greater than that of the dryland, then the rate of flooding, q_n , per unit length of land barrier, is given by

$$q_n = \pm C_o D_b (g D_b)^{\frac{1}{2}}, \quad (9)$$

where D_b is water depth over the crest of the barrier, and C_o is an appropriate dimensionless overflow coefficient, generally less than 0.5 for a broad-crested barrier. The choice of sign depends on whether the flooding is from bay to land or from flooded land back to the bay during the recession stage.

Equation (9) is considered valid for any barrier within or at the boundary of the system for which the water level on one side of the barrier is greater than the barrier crest elevation, Z_b , and for which the water level on the other side is less than Z_b . Moreover, D_b is simply $H_b - Z_b$, where H_b is the water level on the high side.

In the case where the water level on both sides of an internal barrier exceeds the barrier-crest elevation, the discharge is taken as that for a submerged wier,

$$q_n = \pm C_s D_b (g |H_1 - H_2|)^{\frac{1}{2}}, \quad (10)$$

where D_b is the water depth over the crest of the barrier, H_1 and H_2 are the water levels on the two sides of the barrier (both of which exceed Z_b), and C_s is an appropriate dimensionless discharge coefficient for the submerged barrier (generally less than $\sqrt{2}$). In this case, D_b is taken as $(H_1 + H_2)/2 - Z_b$. Again, the sign is taken such that the flow is directed toward the low-head side of the barrier. Both equations (9) and (10) presume that the velocity of approach to the barrier is much less than the velocity over the barrier.

In the numerical computational scheme, emphasis is placed on the evaluation of flow and water levels within a bay which is connected to a sea of essentially unlimited extent. An appropriate boundary condition is required either at the mouth of the bay system or along some line within the sea which delineates the outer limit of the computational grid. The correct approach would be to treat the development of the surge in the sea and bay as a single problem. However, the difference in spatial resolution required for the two different regions of the system, as well as computer storage limitations, makes this impractical. The assumption is made that the effect of the conditions in the bay has only a minor influence on the development of the surge in the sea and over the Continental Shelf. The evaluation of the latter can be determined independently of the bay problem or obtained from observation and used as an outer boundary condition for the bay.

The simplest condition at the seaward boundary is of the form

$$H = H_g, \quad (11)$$

where H_g is the prescribed water level which would exist in the absence of the bay at time t at the outer boundary of the bay system. SURGE II presently uses this condition at the seaward boundary and at lateral boundaries on the limited shelf part of the system. An alternative condition for the lateral boundaries on the shelf is to prescribe that $\partial U / \partial x = 0$ at these boundaries where x is taken alongshore (Jelesnianski, 1966, 1967). An alternative condition for the seaward boundary is one which allows for radiation of energy to the sea. The latter condition is of the form

$$H = H_g + q_n / C_g, \quad (12)$$

where q_n is taken positive outwards from the bay to the sea, and C_g is an appropriate admittance coefficient (with dimensions of velocity). Nominally, C_g represents the wave speed in the sea. The generalized condition (eq. 12) is nearly equivalent to the simplest condition (eq. 11) if C_g greatly exceeds the wave speed for the bay.

b. Initial Conditions. Since the system includes allowance for frictional dissipation as well as radiation of energy, the solution for given fields of X and Y and given boundary function, H_g , should be reasonably insensitive to the nature of the initial conditions after a suitable lapse of time from the initial state. Thus, the initial conditions can be somewhat arbitrary. As in the laboratory model experiments, it is reasonable to start from a state of equilibrium in which U and V are zero and H is uniform throughout the system, in order to minimize the introduction of transient oscillations related to the starting conditions. Moreover, a reasonable period (depending on the characteristic decay time) can be allowed for the system to reach that state where its response reflects only the effect of the forcing functions.

2. Theory of Embedded Channels.

Let s denote distance along the axis of a channel whose cross-sectional area is A and surface width is w at position s and time t . Let Q be the volume transport through A in the positive sense of s , and let H be the water elevation above MSL datum at the same section. In general, A and w are known functions of H for a given cross section, as determined by the geometry of the cross section (Fig. 1). In particular, $\partial A / \partial H = w$ for given s . The width w is to be the "conveyance" width, as used by Dronkers (1964).

The channel is considered an "open system" in the sense that water and momentum may enter or leave the channel laterally; i.e., exchange of fluid with adjacent bay area or flooded land can exist. If the longitudinal velocity in the channel is considered uniform for evaluating the

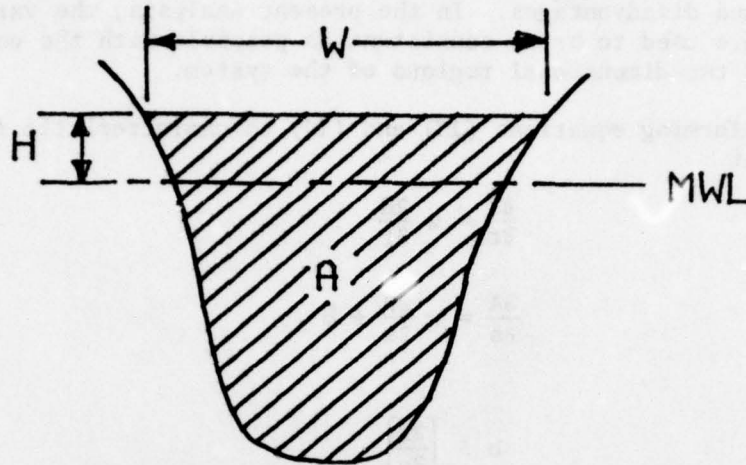


Figure 1. Schematic channel cross section showing pertinent parameters.

longitudinal transport of momentum, then the equations of motion and continuity for a given channel reach are (Stoker, 1957, Ch. 11; Dronkers, 1964, Ch. 9)

$$\frac{\partial Q}{\partial t} + \frac{\partial}{\partial s} (Q^2/A) + gA \frac{\partial H}{\partial s} = wT_s - \sigma Q + L_m \quad (13)$$

$$\frac{\partial A}{\partial t} + \frac{\partial Q}{\partial s} = L_f, \quad (14)$$

where

T_s = longitudinal component of wind stress (divided by water density);

$\sigma = wf|Q|/A^2$ where f is a dimensionless channel-friction coefficient;

L_f = net time rate for gain of water volume per unit distance along the channel by lateral transfer and rainfall;

L_m = associated net time rate of gain of momentum (divided by water density) per unit distance along channel.

The units of L_f are square feet per second; L_m has the units cubic feet per second squared.

It is convenient in the analysis of the channel dynamics to transform the above equations into a characteristic form. There are several different possible characteristic forms. The approach used by Stoker (1957) is to work with u and H (where $u \equiv Q/A$) as the dependent variables. Dronkers (1964) works with either Q and H directly or with Q and total head $(H + (Q/A)^2/2g)$. Each method has certain

advantages and disadvantages. In the present analysis, the variables Q and H are used to be as consistent as possible with the computations in the two-dimensional regions of the system.

In transforming equations (13) and (14) to characteristic form, it is noted that

$$\begin{aligned}\frac{\partial A}{\partial t} &= w \frac{\partial H}{\partial t} \\ \frac{\partial A}{\partial s} &= w \frac{\partial H}{\partial s} + b ,\end{aligned}\quad (15)$$

where

$$b \equiv \left(\frac{\partial A}{\partial s} \right)_{H \text{ const}} . \quad (16)$$

(For a channel of uniform cross section the latter quantity would be zero.) It can be shown, following Dronkers' (1964) analysis and considering equation (15), that a characteristic form of equations (13) and (14) is

$$\frac{dQ}{dt} + w \left(-\frac{Q}{A} \pm \sqrt{\frac{gA}{w}} \right) \frac{dH}{dt} = \left\{ wT_s - \sigma Q + L_m + b \left(\frac{Q}{A} \right)^2 + \left(-\frac{Q}{A} \pm \sqrt{\frac{gA}{w}} \right) L_f \right\} \quad (17)$$

along the path $s(t)$ where

$$\frac{ds}{dt} = \frac{Q}{A} \pm \sqrt{\frac{gA}{w}} . \quad (18)$$

The path line where the plus or minus sign is taken in equation (18) is referred to as the positive P characteristic or the negative N characteristic path, respectively. These are illustrated in Figure 2 where x corresponds to s , the two paths having point C in common. Equation (17) with the upper sign applies along P and equation (17)

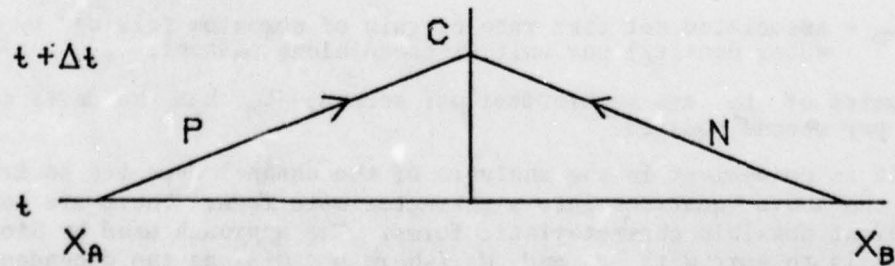


Figure 2. Schematic positive and negative characteristic paths to a common point in the x, t diagram.

with the lower sign applies on path N. Thus, information with regard to Q and H at points x_A and x_B at time t and along the two paths can, in principle, be used to predict the values of Q and H at point C from two equations.

For a laterally closed channel ($L_f, L_m = 0$) of a uniform cross section ($b = 0$) without friction ($\sigma = 0$), in the absence of wind stress ($T_s = 0$), then the quantity in braces on the right-hand side of equation (17) vanishes. In this case only the information at points A and B of Figure 2 is needed to predict values of H and Q at C. To show that equation (17) is consistent with Stoker's (1957) analysis for this special case, let $u = Q/A$ and $D = A/w$. For a uniform cross section at given H, $dH/dt = dD/dt$, so equation (17) reduces to

$$\frac{d(DU)}{dt} + (-u \pm \sqrt{gD}) \frac{dD}{dt} = 0 \quad (19)$$

along

$$\frac{ds}{dt} = u \pm \sqrt{gD}. \quad (20)$$

Equation (19) simplifies further to

$$wD \frac{d}{dt} (u \pm 2\sqrt{gD}) = 0. \quad (21)$$

Thus, for this special case $(u + 2\sqrt{gD})$ is conserved along P where $dx/dt = u + \sqrt{gD}$, while $(u - 2\sqrt{gD})$ is conserved along N where $dx/dt = u - \sqrt{gD}$. Thus, u and D (hence, Q and H) can readily be evaluated at C.

In the more general case the time integral of the right-hand side of equation (17) must be estimated in a rational way. This is considered later in Section III,2. Also, in the general case it is usually not possible to put the left-hand side of equation (17) in the simple form shown in equation (21).

a. Lateral Transfer Terms. In the absence of direct rainfall, L_f must equal the net gain of volume per unit length per unit time due to lateral flow into the channel on either or both sides. Let q_i and q_o , respectively, represent the volume fluxes per unit length into and out of the channel. Then, $L_f = q_i - q_o$ in the absence of rainfall, or

$$L_f = q_i - q_o + wR \quad (22)$$

with rainfall. The corresponding lateral transfer of momentum (divided by water density) is

$$L_m = q_i u_i - q_o u_o, \quad (23)$$

the transfer from rainfall being included in the wind-stress term as discussed in Section II,1. In equation (23) the quantity u_o is simply Q/A for the channel while u_i is the channel-directed component of velocity of fluid from the adjoining block water area. In equation (17) the terms L_m and L_f contribute to the right-hand side the quantity,

$$F_L \equiv L_m - \frac{Q}{A} L_f \pm \sqrt{\frac{gA}{w}} L_f. \quad (24)$$

Using equations (22) and (23) yields

$$F_L = q_i (u_i - u_o) - wRu_o \pm \left(\frac{gA}{w}\right)^{\frac{1}{2}} (q_i - q_o + wR). \quad (25)$$

The lateral flows into or out of the channel can be evaluated by relations such as equations (8), (9), and (10). This is also discussed in Section III,2.

b. Simplifications. The SURGE II program uses certain simplifications of the above equations. For normal conditions, the propagational speed $(gA/w)^{\frac{1}{2}}$ significantly exceeds the speeds u_i or u_o ; i.e., Q/A . Accordingly, F is approximated by

$$F_L = \pm \left(\frac{gA}{w}\right)^{\frac{1}{2}} L_f. \quad (26)$$

Elsewhere in equations (17) and (18), Q/A is neglected compared with $(gA/w)^{\frac{1}{2}}$. Moreover, each channel reach within a grid block is considered of uniform width and bottom elevation Z_c ; however, w and Z_c vary from one reach to another. Thus, $b = 0$ for each reach and

$$A/w = D = H - Z_c. \quad (27)$$

Under these conditions equations (17) and (18) take the form,

$$\frac{dQ}{dt} \pm w\sqrt{gD} \frac{dH}{dt} = \{wT_s - f|Q|Q/(D^2w) \pm \sqrt{gD} (q_i - q_o + wR)\} \quad (28)$$

along

$$\frac{ds}{dt} = \pm \sqrt{gD} \quad (29)$$

where $T_s = X$ or Y as $s = x$ or y , depending on channel orientation. Equation (28) can also be expressed in the form,

$$\frac{d}{dt} (Q \pm \frac{2}{3} wD\sqrt{gD}) = F \quad (30)$$

for a given channel reach where F is the right-hand side of equation (28). The neglect of Q/A relative to \sqrt{gD} in the above approximate channel equations is tantamount to neglect of longitudinal advection of momentum in the original equation (13), an approximation already made in the two-dimensional equations in Section II, 1.

III. SURGE II PROGRAM

Numerical algorithms for two-dimensional blocks and subgrid scale channels are given in this section, and the coupling between these is discussed. A complete listing of the SURGE II program is in Appendix A. A description of the program, as adapted for the GE-400 computer, and the required input and output options are discussed in Appendix B. Appendix C is a user's guide to the SURGE II program. The block algorithm is essentially as discussed by Reid and Bodine (1968) except for a change in the barrier computation and incorporation of coupling with the subgrid scale channels.

1. Block Algorithm.

In the numerical analog of the prognostic equations (1), (2), and (3), values of H are evaluated on a uniform Cartesian mesh at spacing, Δs , for uniform time steps, Δt . The values of H are representative of the water level for the grid square i, j which is centered at $x = (i - 1/2) \Delta s$, $y = (j - 1/2) \Delta s$, at time $n\Delta t$, in which i, j , and n are integers. Values of Z are specified as permanent storage for the same locations as H so that D can be evaluated as needed at these locations. Values of U are evaluated at even half steps of x , odd half steps of y , and odd half steps of t (Fig. 3). This staggered system gives the least storage consistent with a given spatial resolution. It corresponds to the simplest scheme discussed by Platzman (1958) and requires only half the storage compared with the coupled scheme used by Miyazaki (1963).

The variables X and Y are supplied at spatial locations consistent with U and V , respectively, but at even half steps of t . Values of H_g are supplied for positions and times on the outer boundary of the bay consistent with the locations and times for the H values on that line. Values of R are supplied at locations consistent with H but at a one-half time step out of phase with H . Arrays of X, Y , and R , for a single value of j and n , and the array of H_g values for given n are read from tape as required. The fields of X and Y are generated from a coarse spatial and temporal array evaluated from the basic meteorological data and then evaluated for the detailed mesh by linear interpolation.

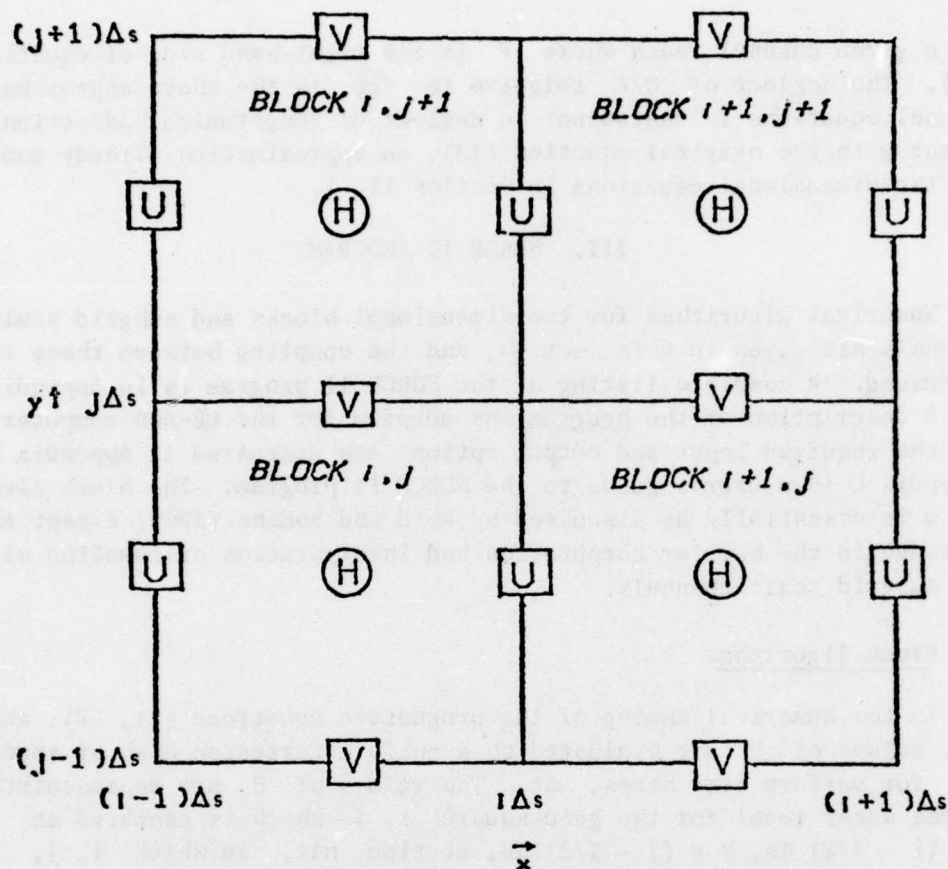


Figure 3. Example of grid blocks showing staggered arrangement of variables U, V, and H.

Information pertinent to the position, elevations, and discharge coefficients for barriers (those not resolved by the limitations of the grid system) is stored as permanent storage along with the field of Z .

The numerical analogs of equations (1), (2), and (3) use values of U , V , H , Z , X , Y , and R at locations shown in Figure 4 for a typical calculation. In the present application a common value of R for given time is used for the whole spatial array. The following notation is used in the recursion equations: $H(i,j)$ represents H centered in block i, j at $t = n\Delta t$; $U(i,j)$ represents U for the left side of block i, j at $t = (n - 1/2)\Delta t$; $v(i,j)$ represents V for the lower side of block i, j at $t = (n - 1/2)\Delta t$.

Primed symbols are used to denote values of these variables at time step Δt later. Thus, the difference $U' - U$ is centered in time at the level of H , and the difference $H' - H$ is centered in time at the level of U' or V' . The notation for Z or D is consistent with that of H .

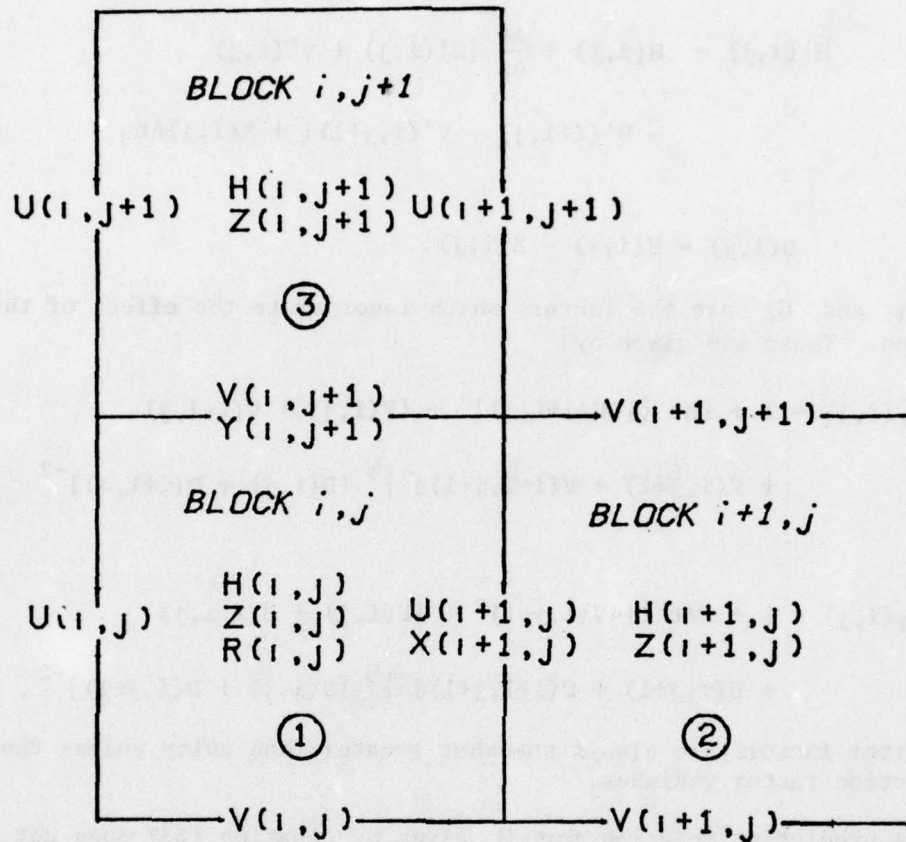


Figure 4. Basic block triad showing variables used in computation of U , V , and H for block 1.

The frictional terms in equations (1) and (2) are represented by $fAU'D^{-2}$ and $fQV'D^{-2}$, respectively, where the estimation of Q and D is centered spatially at the position for U' or V' . Since U , V , and D are not available at common locations, this requires a suitable spatial average in order to obtain centered values of Q and D . The resulting recursion equations for U , V , and H , using centered differences for the spatial derivatives, are as follows:

$$U'(i+1,j) = \frac{1}{G_1(i,j)} \left\{ U(i+1,j) + \frac{g\Delta t}{2\Delta s} [D(i+1,j) + D(i,j)] [H(i,j) - H(i+1,j)] + X(i+1,j)\Delta t \right\} \quad (31)$$

$$V'(i,j+1) = \frac{1}{G_2(i,j)} \left\{ V(i,j+1) + \frac{g\Delta t}{2\Delta s} [D(i,j+1) + D(i,j)] [H(i,j) - H(i,j+1)] + Y(i,j+1)\Delta t \right\} \quad (32)$$

$$H'(i,j) = H(i,j) + \frac{\Delta t}{\Delta s} [U'(i,j) + V'(i,j) - U'(i+1,j) - V'(i,j+1)] + R(i,j)\Delta t, \quad (33)$$

where

$$D(i,j) = H(i,j) - Z(i,j), \quad (34)$$

and G_1 and G_2 are the factors which incorporate the effect of the friction. These are given by:

$$G_1(i,j) = 1 + f\Delta t \{ [4U(i+1,j)]^2 + [V(i,j) + V(i+1,j) + V(i,j+1) + V(i+1,j+1)]^2 \}^{\frac{1}{2}} [D(i,j) + D(i+1,j)]^{-2} \quad (35)$$

and

$$G_2(i,j) = 1 + f\Delta t \{ [4V(i,j+1)]^2 + [U(i,j) + U(i+1,j) + U(i,j+1) + U(i+1,j+1)]^2 \}^{\frac{1}{2}} [D(i,j) + D(i,j+1)]^{-2}. \quad (36)$$

The latter factors are always somewhat greater than unity unless the flow or friction factor vanishes.

The prediction relation for H given by equation (33) does not consider any possible contribution of flow to or from the block due to the presence of a subgrid scale channel. This will be considered in a subsequent section.

It should also be emphasized that the effect of Coriolis force is not considered. The relative importance of the Coriolis force compared with bottom friction can be estimated in terms of the ratio, r , of these two forces which is of the order,

$$r = \lambda D / fu, \quad (37)$$

where

λ = Coriolis parameter ($2\Omega \sin \phi$, Ω being the absolute angular speed of the earth and ϕ the latitude);

D = mean depth;

f = bottom-friction coefficient;

u = typical fluid speed in the bay.

For 30° latitude $\lambda = 7.3 \times 10^{-5}$; typical D and f for gulf coast bays are 10 feet and 2×10^{-3} , respectively. For $u = 3$ feet per second, which is reasonable for storm conditions, r is only $1/10$. However, for normal circulatory regimes u may be only a fraction of 1 foot per second and r is of order unity. Hence, while it may be justifiable to neglect the Coriolis term for short-duration storm surge studies for shallow bays of limited horizontal dimensions it cannot be neglected in long-term circulatory studies.

Although it does not appear difficult to add the effect of Coriolis force, it can be shown (Platzman, 1958) that a different scheme for the U , V , and H arrays is necessary for numerically stable computations using an explicit time-marching procedure as used here. The coupled scheme required for stable explicit computations at least doubles the computing time. The present scheme could be used with an implicit time-marching procedure to maintain stability and similar accuracy, but this too can be achieved only at the cost of an increase in computing time by a factor of at least two. In the presence of friction, the destabilizing effect of the Coriolis terms in an explicit scheme such as that used by Masch (1969) is suppressed; however, this is accomplished only at the sacrifice in rendition of the frictional terms. Thus, the omission of the Coriolis force from a program intended primarily for gulf coast estuaries is motivated primarily for reasons of economy of operation, in respect to surge calculations.

a. Stability. Numerical stability requires that Δt be taken at less than the value $\Delta S / (2gD_{\max})^{1/2}$, where D_{\max} is the maximum depth to be expected anywhere in the system during the storm surge (Platzman, 1958).

b. Barrier Algorithm. Equations (9) and (10) are assumed to apply for values of q_n , D_b , and ΔH at the same time and in the immediate vicinity of the barrier. In the grid scheme used, however, the flow and the water level are staggered in time; moreover, the water levels like H_1 and H_2 represent in effect the spatial average for blocks 1 and 2, respectively, at a given time rather than local values in the vicinity of a given barrier, which in the schematization are presumed to occur on lines separating two blocks. As a consequence the above relations cannot be applied directly. Instead, the evaluation of U or V across a barrier (if the water level allows such flow) is carried out by a modified version of the predictive equations (1) and (2), or their numerical counterparts, equations (31) and (32), where f is replaced by an effective value related to the barrier discharge coefficient so as to be consistent with equations (9) or (10). The effect is to maintain proper time phasing and to consider possible tilt of water level across the block; i.e., difference of H at barrier relative to the mean value for the block.

Specifically, the frictional terms in equation (1) or (2) are taken as $(D/LC_b^2) |q'_n| q'_n / D_b^2$ where C_b is the barrier discharge coefficient

(C_o or C_s , depending on type of barrier), q_n' is the transport per unit width normal to the barrier (either U' or V' , depending on barrier orientation), D_b is the water depth over the barrier, and \bar{D} is a mean depth for the effective fetch L across the blocks. The gravitational slope term involves the same scale length, L , and mean depth, \bar{D} . The resulting relation for prediction of q_n' at a barrier, given q_n at the previous time step, is:

$$|q_n'|q_n' + \Gamma q_n' = F, \quad (38)$$

where

$$\Gamma \equiv \frac{L(C_b D_b)^2}{\bar{D} \Delta t} \quad (39)$$

and

$$F \equiv g(C_b D_b)^2 \Delta H + \Gamma \cdot (q_n + P), \quad (40)$$

P being the wind "push" term ($X\Delta t$ or $Y\Delta t$), and ΔH a head differential dependent on barrier type. For steady state ($q_n' = q_n$) and no wind ($P = 0$), the above reduces to

$$q_n' = \pm C_b D_b \sqrt{g|\Delta H|}, \quad (41)$$

which is consistent with equation (9) or (10) with C_b and ΔH taken as C_o and D_b or C_s and $(H_1 - H_2)$, respectively, depending on the barrier. The more general relation (eq. 38) provides an added effect of the wind and of the inertia of the water on the blocks. For a submerged barrier, L is taken equal to ΔS ; i.e., from the center of block 1 to the center of block 2. For an overflow barrier, L is taken as half this distance since the inertia and wind setup are effective only on the higher of the two blocks.

Thus, C_b , L , H , and D_b are taken as follows:

Submerged barrier ($H_1 > Z_b$ and $H_2 > Z_b$)

$$\begin{aligned} C_b &= C_s \\ L &= \Delta S & \Delta H &= H_1 - H_2 \\ D_b &= [(H_1 + H_2)/2] - Z_b \end{aligned} \quad (42)$$

Overflow barrier ($H_1 > Z_b$ or $H_2 > Z_b$)

$$\begin{aligned} C_b &= C_o \\ L &= \Delta S/2 & \Delta H &= \begin{cases} H_1 - Z_b & (a) \\ \text{or} \\ Z_b - H_2 & (b) \end{cases} \\ D_b &= |\Delta H| \end{aligned}$$

where Z_b is the elevation of the barrier crest, relation (a) being for $H_1 > Z_b$ and (b) for $H_2 > Z_b$. If Z_b exceeds both H_1 and H_2 , then $q_n' = 0$. The meaningful solution of the quadratic equation (38) is

$$q_n' = \pm \{ [|F| + (r/2)^2]^{1/2} - r/2 \}, \quad (43)$$

where the sign is taken as that of F , as verified from equation (38).

The above relations for barriers differ from that used in Reid and Bodine (1968) and in the original SURGE I program. The present barrier relations have a more realistic response when applied to the numerical simulation of a natural oscillation of a bay having a submerged barrier across it.

c. Barrier Specification. Since only certain blocks contain barriers, they must be identified by I, J location; specifically, the program identifies the K th barrier block by location $I = IB(K)$ and $J = JB(K)$, $K = 1, 2 \dots KM$. A given barrier block potentially has a barrier on the right and upper side of the block in an x, y plot. These are designated x and y , respectively; i.e., an x barrier is one normal to the x -axis (the flow over it being in the x sense). For both potential barriers on a barrier block, values of Z_b , C_o , and C_s must be prescribed. A real barrier is one where Z_b is larger than the Z value for either of the adjoining blocks. A null barrier is one where Z_b equals the larger of the Z values for the adjoining blocks (thus, in effect, the higher block is a potential barrier). The program requires that information pertinent to both null barriers (Z_b , C_o , and C_s) and real barriers be provided.

d. Volume Check. During the recession stage of flooding when water is draining off flooded blocks (via the barrier overflow relation), it is possible for the volume leaving in one time step as computed from $q_n' \Delta t$ to exceed the available volume. Therefore, a test is included in the program such that if this occurs, the flow is adjusted to only drain the block dry ($D = 0$), and the flow to adjacent blocks adjusted to be consistent.

e. Depth Check. When the water depth is very shallow the effect of the wind is such that a given block could become partially dry unless the fluid is flowing fast enough for the bottom stress to balance the wind stress. To avoid anomalous computations for very small D (e.g., in areas where rainfall is occurring over regions above the surge level), the wind stress is arbitrarily set zero when D is less than 0.1 foot.

2. Channel Algorithm.

a. Channel Specification. As in the case of barriers, those blocks on which channels occur are identified by the I and J values; for channel block K these are denoted by $ICG(K)$ and $JCG(K)$, respectively, where $K = 1, 2 \dots KCM$. Also each "channel block" may contain two channels, one on the right denoted the x channel and one on the upper side denoted the y channel. Each of these channel reaches is characterized by a

channel width (w), a channel-bed elevation (Z_c), and a channel-friction coefficient (f_c). Figure 5 shows a schematic of a channel block indicating nomenclature for dimensions as used in the SURGE II program. Figure 6 shows the dependent variables pertinent to the channels as used in the program and stored for the channel block K . These include the channel flows, Q , at each end of the channel, one end designated N , the other P (corresponding to the negative and positive characteristic ends of the channel, respectively). Also included is the height, H , of the water level at the point in common to the two channels for block K ($HC(K)$). The lateral transport (per unit width per unit time) nominally to the channel from block K and from the channel is also indicated: $UCT(K)$ and $UCF(K)$, respectively, for the channel normal to the x -axis, and $VCT(K)$ and $VCF(K)$, respectively, for the channel normal to the y -axis. In the formulas in this study, these are referred to as q_t and q_f , respectively. Note that $UCF(K)$ and $VCF(K)$ correspond to U and V , respectively, on the right and upper sides of the general block flow. Also, the quantity $HP(K)$ corresponds to the block (pool) height for the channel block. Values of H at the "negative" ends of the channels for channel block K are stored as HC values in adjacent channel blocks to minimize duplication of storage.

b. Computation of Channel Variables. The time phasing of block variables versus channel variables is indicated in Table 1. The H values occur at common times thus facilitating evaluation of head differentials used in determining lateral flow between channel and adjacent blocks.

Table 1. Time phasing of computations for blocks and channels.

Time	Block	Channel
$t + \Delta t$	H	H, Q
$t + \Delta t/2$	Q	
t	H	H, Q
$t - \Delta t/2$	Q	
$t - \Delta t$	H	H, Q

For a given channel reach, application of equations (28) and (29) can be made for two characteristic paths, as shown schematically in Figure 7. As in the case of the block computations, the friction term in equation (28) is taken proportional to the product of a new Q and

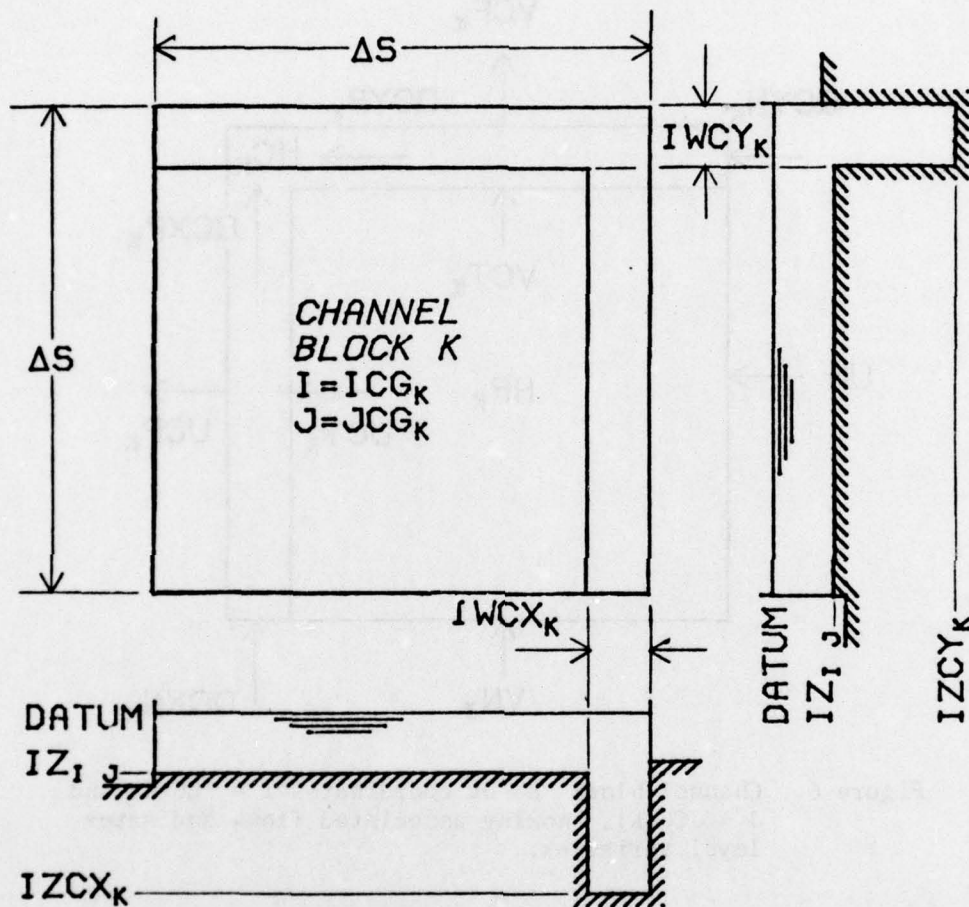


Figure 5. Channel block, showing channels and their dimensions.

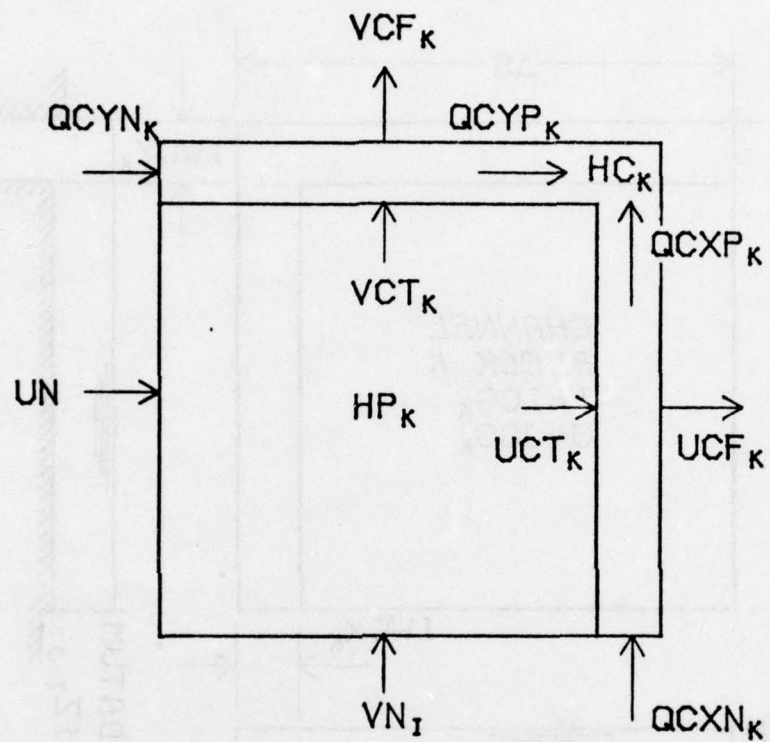


Figure 6. Channel block K at coordinates $I = ICG(K)$ and $J = JCG(K)$, showing associated flows and water level variables.

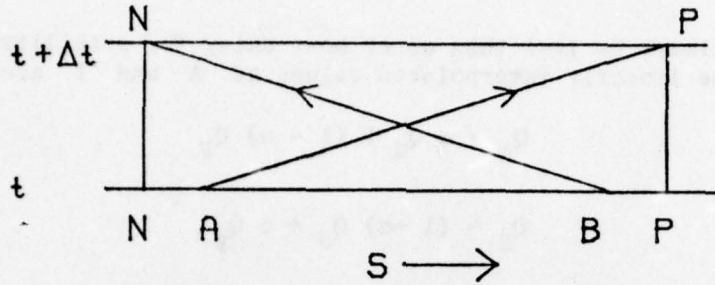


Figure 7. Characteristic paths on the time-distance diagram for an individual channel reach.

the absolute value of the old Q . Specifically, for the positive characteristic path from A to P' in Figure 7, equation (28) is approximated by

$$(Q'_P - Q_A) + w\sqrt{gD} (H'_P - H_A) = [WT_s - f_c |\bar{Q}| Q'_P / (\bar{D})^2 w + \sqrt{gD} \Delta q] \Delta t, \quad (44)$$

where $\bar{D} = (D_N + D_P)/2$, T_s is the appropriate wind-stress component (X or Y) corresponding to time level t for the associated channel block, Δq is the net lateral flow per unit width, and \bar{Q} is taken as

$$\bar{Q} = [(Q_N^2 + Q_P^2)/2]^{1/2}. \quad (45)$$

The subscripts on Q , H , and D designate the points at which these apply (see Fig. 7) and primes denote new time level.

After regrouping terms, equation (44) can be written as

$$Q'_P + (w\sqrt{gD}/G)H'_P = [(Q_A + w\sqrt{gD}H_A) + (WT_s + \sqrt{gD}\Delta q)\Delta t]/G, \quad (46)$$

where

$$G \equiv 1 + f_c \Delta t |\bar{Q}| / (\bar{D})^2 w. \quad (47)$$

Similarly, for the negative characteristic from B to N',

$$Q'_N - (w\sqrt{gD}/G)H'_N = [(Q_B - w\sqrt{gD}H_B) + (WT_s - \sqrt{gD}\Delta t)/G], \quad (48)$$

where \bar{D} and G are as defined for the positive characteristic.

The values of Q and H at points A and B are determined by interpolation from values at N and P at time t , using equation (29) for the path. The distance from A to P or B to N, using the mean wave speed for the channel at time t is $\sqrt{gD}\Delta t$. The interval N to P is equal to Δs . Let

$$\alpha \equiv \sqrt{gD} \Delta t / \Delta s; \quad (49)$$

this should always be less than or at most unity for stability of computation. The linearly interpolated values at A and B are then

$$\begin{aligned} Q_A &= \alpha Q_N + (1 - \alpha) Q_P \\ Q_B &= (1 - \alpha) Q_N + \alpha Q_P, \end{aligned} \tag{50}$$

and similarly for H_A and H_B in terms of H_N and H_P .

The evaluation of Δq is the most sensitive part of the computations and is discussed in a subsequent section. Presuming Δq is known, the problem of evaluating the new Q and H individually at the channel-end points is considered. Note that equations (46) and (48) yield predictions for linear combinations of Q and H at two different points. Thus, information from adjoining channels, or other information in the case of channel end points, is needed to solve for the new channel Q and H . For a simple continuous channel without branches and consisting of a series of reaches of length Δs but not necessarily of equal width or depth, then Q and H are readily solved at a common junction, using the information from the positive characteristic from one channel and the negative characteristic from the adjoining channel. However, branches do occur and it is therefore desirable to use a sufficiently general procedure which will accommodate either branching channels or continuous channels.

In the scheme chosen for representing channels in SURGE II it is possible to have four channels merging at a common junction. Figure 8 shows this junction with four different volume transports, but with a common H . The designation of the different Q shown in this figure is that used in the coded program (see App. B); Q_C for channel transport, X or Y denoting the channel (not the direction of flow), and N or P denoting whether the flow is at the negative or positive end of a given channel reach. Each is identified by a channel block index K .

For any given channel reach equations (46) and (48) predict, for a given point, values of the quantities

$$\begin{aligned} BP &\equiv Q' + \lambda H' \\ BN &\equiv Q' - \lambda H', \end{aligned} \tag{51}$$

where

$$\lambda \equiv w \sqrt{gD}/G. \tag{52}$$

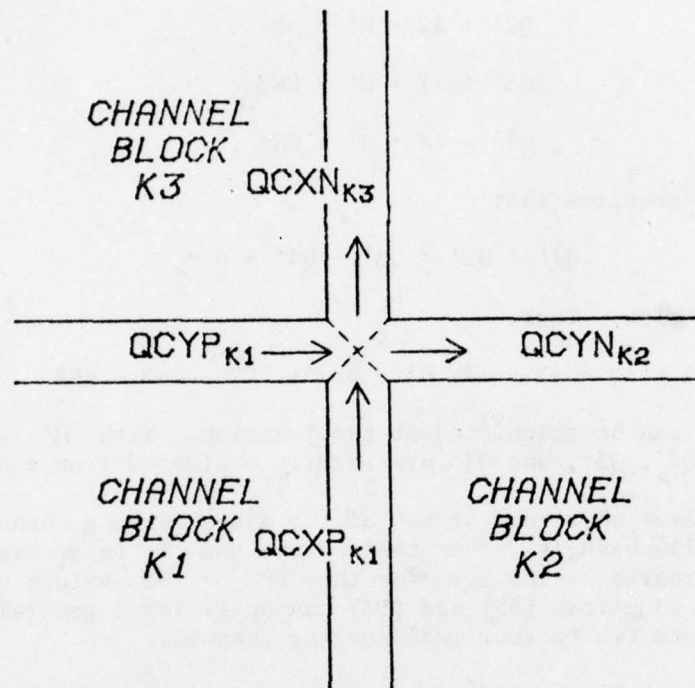


Figure 8. General channel junction, showing flows and channel identification.

For simplicity of notation let 1, 2, 3, and 4 denote the merging channels with 1 being the lower channel, 2 the left channel, 3 the upper channel, and 4 the right channel (Fig. 8). Then, with this notation

$$\begin{aligned} Q1' + \lambda 1 \cdot H' &= BP1 \\ Q2' + \lambda 2 \cdot H' &= BP2 \\ Q3' - \lambda 3 \cdot H' &= BN3 \\ Q4' - \lambda 4 \cdot H' &= BN4 \end{aligned} \quad (53)$$

Now, continuity requires that

$$Q1' + Q2' - Q3' - Q4' = 0 \quad (54)$$

at a common junction. Thus,

$$(\lambda 1 + \lambda 2 + \lambda 3 + \lambda 4) H' = BP1 + BP2 - BN3 - BN4 \quad (55)$$

from which H' can be calculated at the junction. With H' known, the values of $Q1'$, $Q2'$, $Q3'$, and $Q4'$ are readily evaluated from equation (40).

For those cases where one or two of the above merging channels do not exist (i.e., null channels), then their width and λ value are zero. Moreover, the program yields zero for the BP or BN values of any null channel. Thus, equations (55) and (53) can apply for a general junction consisting of from two to four real merging channels.

c. Net Lateral Flow. The net time rate of water accumulation in the channel per unit length due to lateral exchange with blocks and by rainfall is

$$\Delta q = q_t - q_f + wR, \quad (56)$$

where q_t corresponds (if positive) to the flow (per unit length of channel) from the channel block to the channel (across the "interior" side of the channel, Fig. 6) and q_f (if positive) is the flow (per unit length of channel) from the channel to the adjacent block. These flows can be positive, negative, or zero. To allow for channels which have widths w much smaller than the block grid size Δs , and since the above q values are comparable to those which exist across the sides of blocks, the change in channel water level can be very sensitive to the difference $q_t - q_f$. Hence, special care must be taken in the model to avoid possible instabilities caused by improper calculation of these transverse flows. However, there is no particular difficulty with the rainfall term in equation (56) which is generally at least one order of magnitude smaller than that of the "net" lateral flow. In a sense, the potential difficulty with the transverse flows, q_t and q_f , arises because the Δt chosen for stable calculation on the blocks is usually

too large for stable calculation for narrow channels, unless the coupling with blocks exists only in respect to longitudinal flow from the channels to blocks at end points of such channels.

On a given side of a channel, basically four physically distinct situations can occur: (a) a barrier (levee) or block ground level of sufficient height exists to prevent lateral flow; (b) overflow exists from an adjacent flooded block into a channel where the water level is less than the adjacent barrier or ground level; (c) overflow of adjacent barrier (levee) exists from the channel to an adjacent dry block or one where the water level is lower than the barrier elevation; or (d) both the channel water level and the water level on the adjacent block exceed the height of any intervening barrier and the lateral flow depends on the difference of water level. These four situations are illustrated in Figure 9. In the fourth situation, the water level could also be lower on the channel side with the associated lateral flow reversed.

For situation (a) there is no problem, the appropriate lateral flow (q_t or q_f) being constrained to zero value. For situation (c), the predictive-type barrier relation (eq. 55), with auxiliary relations (eqs. 39 and 40), could be used. In principle, the above predictive barrier relations should apply for situation (b) as well, provided that L in equation (39) is taken as the channel width w . However, since w can be much less than Δs for many applications, Γ can be so small that the relation for q_n' reduces virtually to a diagnostic-type relation of equation (40), or more specifically of equation (9) for barrier overflow. Since situation (b) might occur on one side of the channel and situation (c) on the other, and since both should be evaluated by relations compatible with a common time level, the simple diagnostic relation (eq. 9) has been adopted for both situations in the SURGE II program. This, however, still demands special checks and possible adjustments, as will be discussed later. Finally for situation (d), a submerged barrier-type calculation might seem appropriate if the depth over such a barrier is small compared with that of the channel or adjacent block; however, use of such relations in preliminary versions of the program proved to be very vulnerable to numerical instability. The reason for this is related to the above discussion concerning the usual case where $w/\Delta s$ is very small. As a consequence, for situations of type (d), a special calculation is required which treats the channel as essentially an integral part of the associated channel block or the adjacent block.

As stated above, for overflow situations (b) or (c), i.e., to or from the channel, the relation,

$$q_n = \pm C_o D_b (g D_b)^{1/2}, \quad (57)$$

is used where $D_b = H - Z_b$, H being the water level on the high side of the barrier. While this relation gives a valid value of q_n (q_t or q_f) at the time t , the value of q_n may change significantly over the prediction interval Δt if $(g D_b)^{1/2} > w/\Delta t$.

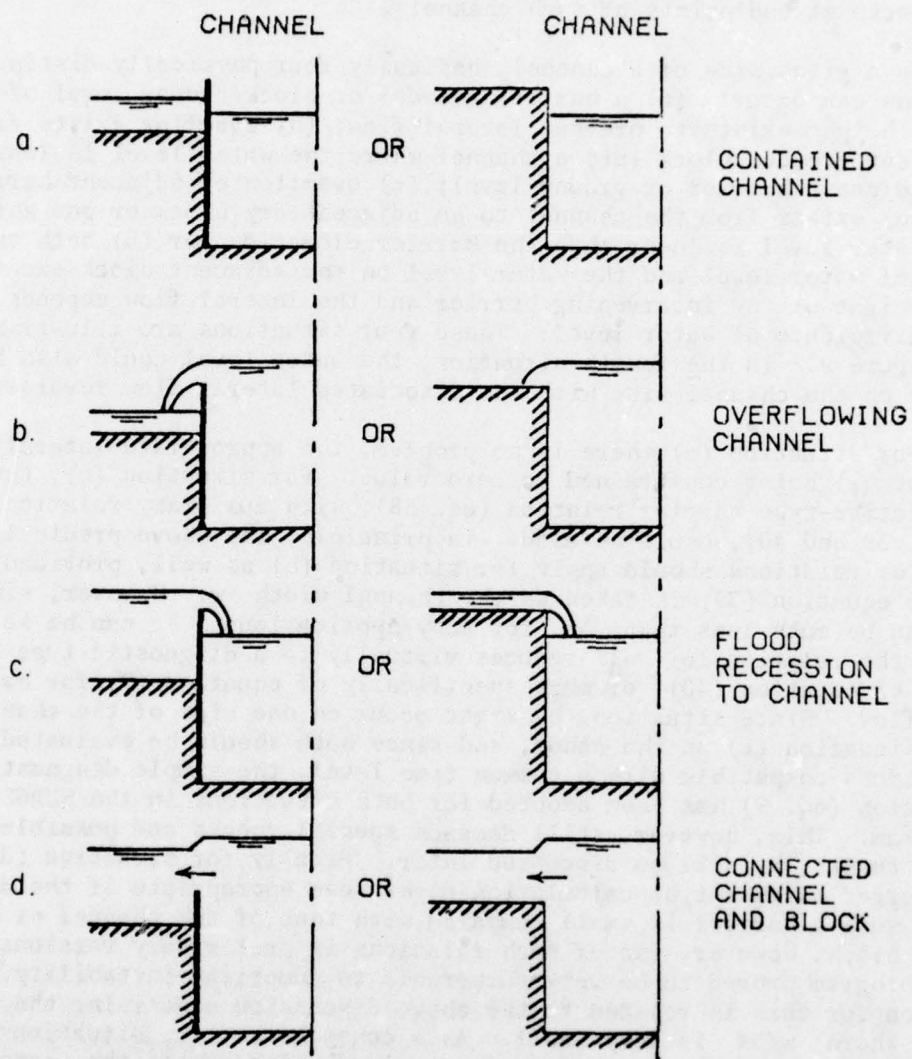


Figure 9. Different situations along a given side of a channel.

Under such circumstances, an approximate prediction based on the initial values of q_n could lead to physically impossible changes of channel level. Thus, tests are included in the program to constrain the lateral flow, such that $q_t - q_f$ alone will not cause the channel level, H_c , to fall below a minimum possible value nor rise above a maximum possible value, depending on the situation. Six different situations requiring tests are illustrated in Figure 10 (the "mirror" version of each is also a possible situation). Situations where one side of the channel is blocked are special cases of those indicated. For situations A, C, and E, outflow exceeds inflow and the horizontal dashline represents a minimum level based on the sill depth of the channel. On the other hand, for situations B, D, and F, the horizontal dashline represents a maximum possible level. In each case, the maximum possible change in H_c is indicated as ΔH_c .

For any of the situations illustrated in Figure 10, the SURGE II program compares $|q_t - q_f|$ with $|wH_c/\Delta t|$. If the latter is exceeded by the trial value of $|q_t - q_f|$ then an adjustment is made in q_t or q_f such that $|q_t - q_f|$ equals $|w\Delta H_c/\Delta t|$. For cases A, B, C, and D, both q_t and q_f are prorated by a common factor to satisfy the above constraint. For cases E and F, only the overflow q is adjusted to be consistent with the above constraint.

For situation (d) where the channel and block are connected by a continuous water surface (Fig. 10), the net lateral flow to the channel, Δq , is taken to be that which would be required to bring H_c to a value equal to the existing mean level, HM , of the connected channel and block. For a channel connected to a block on one side only then,

$$HM = \frac{HB \cdot L + HC \cdot W}{(L+W)}, \quad (58)$$

where HB is the water elevation on the water-connected block, L is its width, while HC and W are the water elevation and width for the channel. The block width L is $\Delta S - W$ if the connected block is the channel block containing the channel, or is ΔS for an adjacent water-connected block. If the channel is water connected on both sides, then the above relation is replaced by an appropriate average over both blocks plus the channel.

The Δq for either of these situations is taken as

$$\Delta q = (HM - HC)w/\Delta t. \quad (59)$$

To determine the individual q_t and q_f on either side of the channel, the mean of these is taken to be that which is calculated as the flow

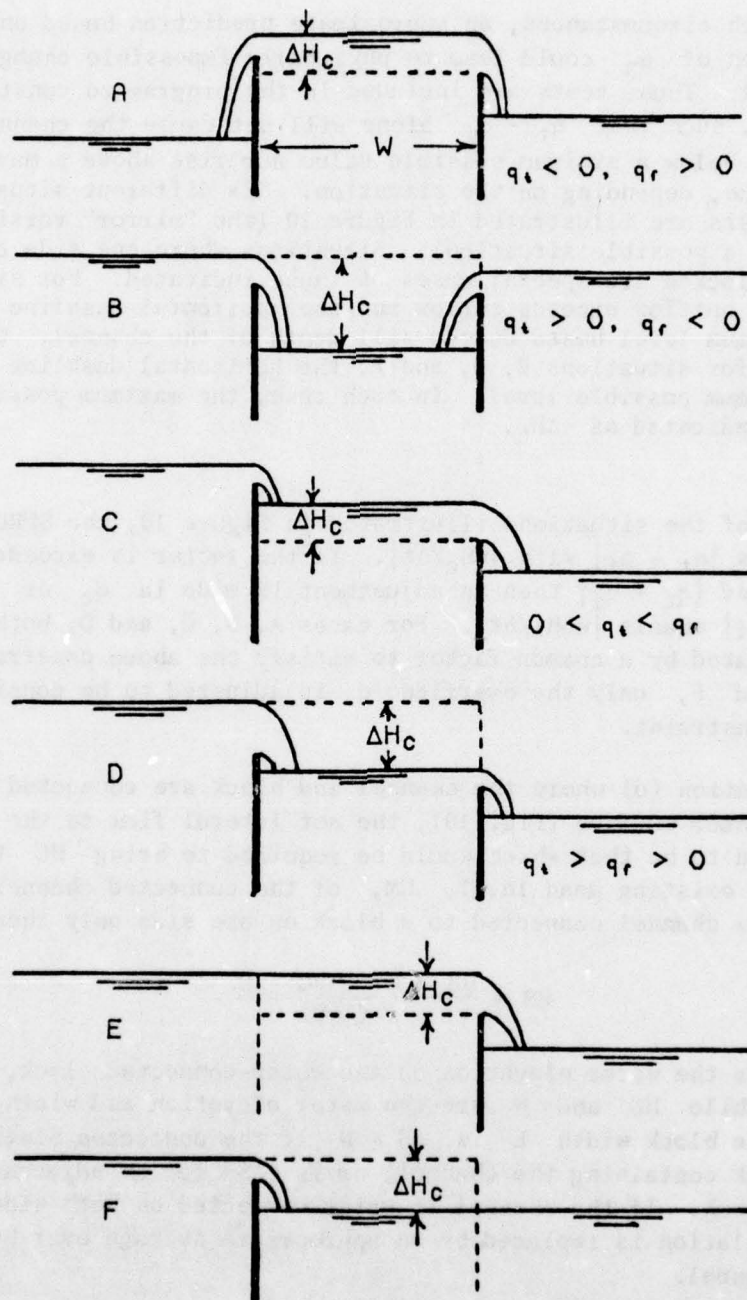


Figure 10. Situations involving overflow to or from a channel which require special checks.

between blocks, ignoring the presence of the channel (but considering barriers). Letting this be denoted q_m , then

$$\begin{aligned} q_t &= q_m + \Delta q/2 \\ \text{and} \quad q_f &= q_m - \Delta q/2 . \end{aligned} \tag{60}$$

This system of calculation leads to stable results.

d. Channel End-Point Computations. At the end point of a given channel system, special computations are required. Two types of end conditions are used: an "H-end condition" is used where a channel discharges into a lake, bay, or sea, in which case the channel H value at the end point is taken equal to the H of the adjacent channel block into which the channel discharges (or vice versa); a "Q-end condition" is used at the head of a channel or river at which point the discharge is specified.

For a Q-end point

$$\begin{aligned} Q' &= \pm Q'_r \\ H' &= (Q' - B)/\lambda , \end{aligned} \tag{61}$$

where Q'_r is the specified river discharge (taken as zero if not specified); B equals BP or $-BN$, as defined by equation (51), for end points occurring at the positive or negative end of the channel reach, respectively, and λ is as defined in equation (52). The sign of Q' is taken such that Q' is directed into the channel, depending on the channel-end orientation. There are four possible orientations (see App. B, Fig. B-3).

The H-end points also have four possible configurations; these are depicted along with the associated adjacent "ponding" areas (i.e., a block with $Z < 0$) in Figure 11. For an H-end point neither the longitudinal flow to or from the channel nor the H at the junction with the ponding block is specified *a priori*. It is required only that the predicted H at the channel-end point and that of the ponding block be the same. Let H^* be the (tentative) predicted H for the ponding block in the absence of any contribution by longitudinal discharge to or from the channel which terminates adjacent to that block. Thus, H^* corresponds to the H resulting from the routine block calculation using equation (33) with appropriate adjustments for contained channels as might occur for situations 3 and 4 shown in Figure 11. These adjustments are discussed in a subsequent subsection. The correct predicted H for the ponding block in the presence of longitudinal discharge from a channel is given by

$$H' = H^* + (Q'_d + Q_d)\Delta t/2A_b , \tag{62}$$

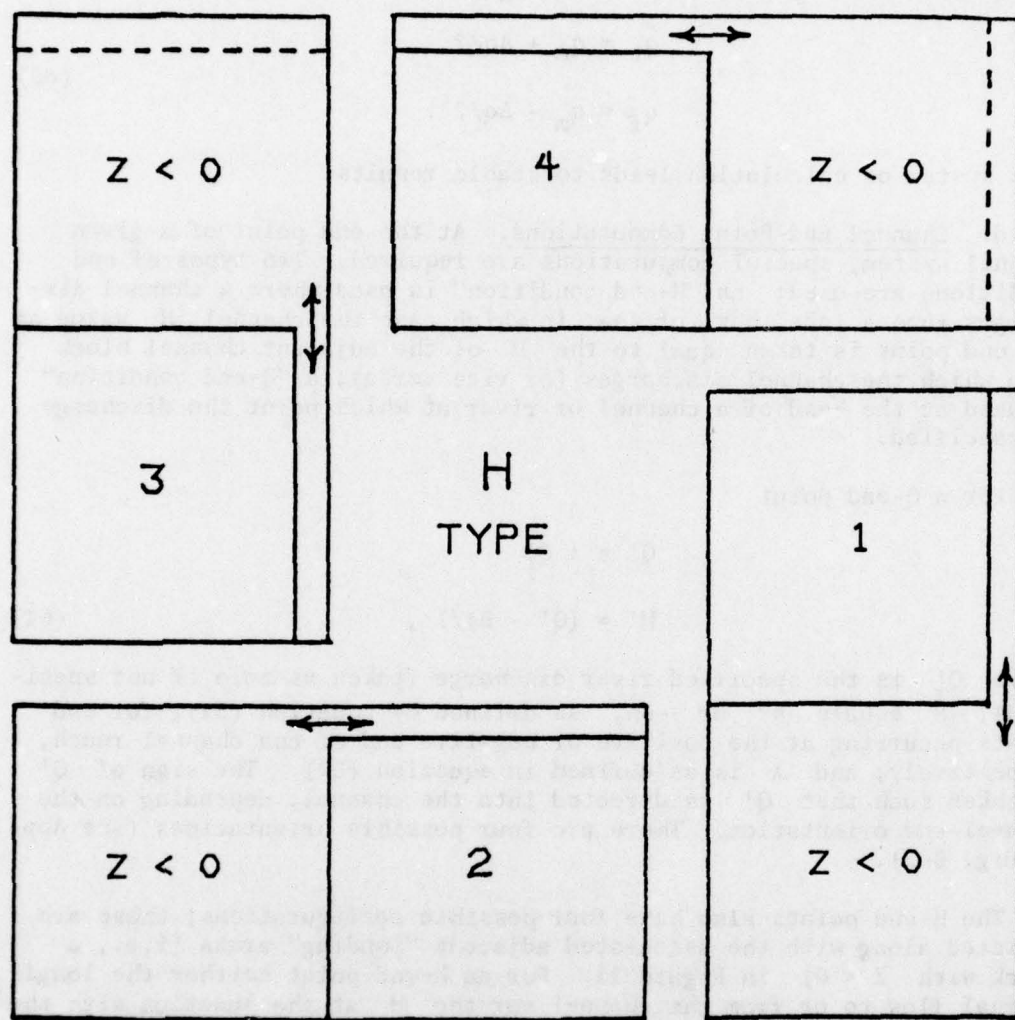


Figure 11. Possible end-point configurations and index identification (1 to 4).

where Q'_d and Q_d are the new and previous values, respectively, of the discharge from channel to ponding block, and A_b is the effective surface area of the block. For situations 1 and 2 in Figure 11, $A_b = (\Delta s)^2$, but for situations 3 and 4 a channel might exist on the ponding block in which case $A_b = (\Delta s - w)\Delta s$.

Equation (62) involves two unknowns H' and Q'_d . However, for the channel,

$$Q'_d + \lambda H' = B, \quad (63)$$

where $B = -BN$ for end-point type 1 or 2 and $B = BP$ for end-point type 3 or 4, BN , BP and λ being those quantities defined by equations (46) to (52). Note that for end-point type 1 or 2, Q'_d is the negative of the QC value for the channel.

The resulting H' and Q'_d for an "H-end" condition are

$$H' = (F + B\Delta t/2A_b)/(1 + \lambda\Delta t/2A_b) \quad (64)$$

$$Q' = (B - \lambda F)/(1 + \lambda\Delta t/2A_b), \quad (65)$$

where

$$F \equiv H^* + Q_d\Delta t/2A_b. \quad (66)$$

e. Calculation of H on Channel Blocks. For blocks with $D > 0$ and containing one or two channel reaches, the prediction relation for H given by equation (33) is not valid. The correct relation for a channel block k having location i, j is

$$\begin{aligned} H'(i, j) = & H(i, j) + [U'(i, j) - UCT'(k)]\Delta t/(\Delta s - wx) \\ & + [V'(i, j) - VCT'(k)]\Delta t/(\Delta s - wy) \end{aligned} \quad (67)$$

where UCT and VCT are as shown in Figure 6 and correspond to the q_t discussed previously. If only one channel exists (i.e., if wx or wy is zero), then

$$UCT'(k) = U'(i + 1, j) \text{ if } wx = 0$$

or

$$VCT'(k) = V'(i, j + 1) \text{ if } wy = 0.$$

IV. APPLICATION TO THE SABINE-CALCASIEU SYSTEM

1. Adopted Grid and Simulated Topography.

The Sabine-Calcasieu system geographically bridges the Texas-Louisiana border and is physically linked by a system of manmade channels and a low-lying region extending 25 miles between Sabine Lake and Lake Calcasieu.

A local chart of the region is shown in Figure 12. The rectangular border indicates the region included in the numerical analog. The selection of the size of this rectangle is dictated by the basic hydrodynamic features required to adequately represent the region and then the logistical and economic limitations placed on the computations by the availability of computer storage. The region selected is 56×40 nautical miles. The grid size (DELX) is taken as 2 nautical miles, so that $IM = 28$ and $JM = 20$.

Figure 13 is a contoured plot of the schematized topography superimposed on the selected grid system. The offshore topography is regular with the exception of a shallow region adjacent to Sabine Pass and a slight embayment lying between Sabine Pass and the outlet from Lake Calcasieu at Cameron. Both lakes are adequately represented by the grid interval of 2 nautical miles. Figure 14 clearly delineates three high topographic areas in the numerical model: the Beaumont rise in the northwest, the Orange rise, and a more gradual rise northeastward to the Lake Charles area. The low-lying region between the lakes, immediately behind the shoreline barrier, and forward of the rises, forms a large ponding area during the inundation sequences. Between each rise a major channel is present, the Neches River, the Sabine River, and in the Lake Charles region, the Calcasieu River runs northeastward from Lake Calcasieu.

The deepest block in the system is -24 feet (MSL). Assuming a 10-foot surge, a value of DELT equal to or less than 260 seconds (Sec. III, 1,b) is required. The value chosen for DELT is 240 seconds.

2. Channel and Barrier Schematization.

The numerical discretization of the area shown in Figure 12 is given as an overlay in Figure 15. In this illustration the channel network (shown by full lines) shows the landward interconnection of Sabine and Calcasieu as well as the link with the Intracoastal Waterway as the lower left- and right-hand channels. Each channel segment has been provided the physical characteristics of width and cross-sectional area that best reproduce the pertinent information for the channel reach that was provided by the Corps of Engineers. The extent of the channel system was chosen on the basis of past inundation history and the judgment of the authors.

The barrier system, also shown in Figure 15, represents the major manmade and natural obstructions to flow above MSL. At the shoreline the major dune line is continuous with the exception of an apparent open area east of Sabine Pass. The block elevation of that area equals the adjacent barrier heights. Jetties are included at each of the openings to the Gulf of Mexico. Within the region the majority of barriers are manmade levees erected for protection. The heights of all barriers were chosen on the basis of data provided by the Corps of Engineers.

Appendix D has a listing of all data used for the Sabine-Calcasieu region in the simulation of the Hurricane Carla surge. The topography,

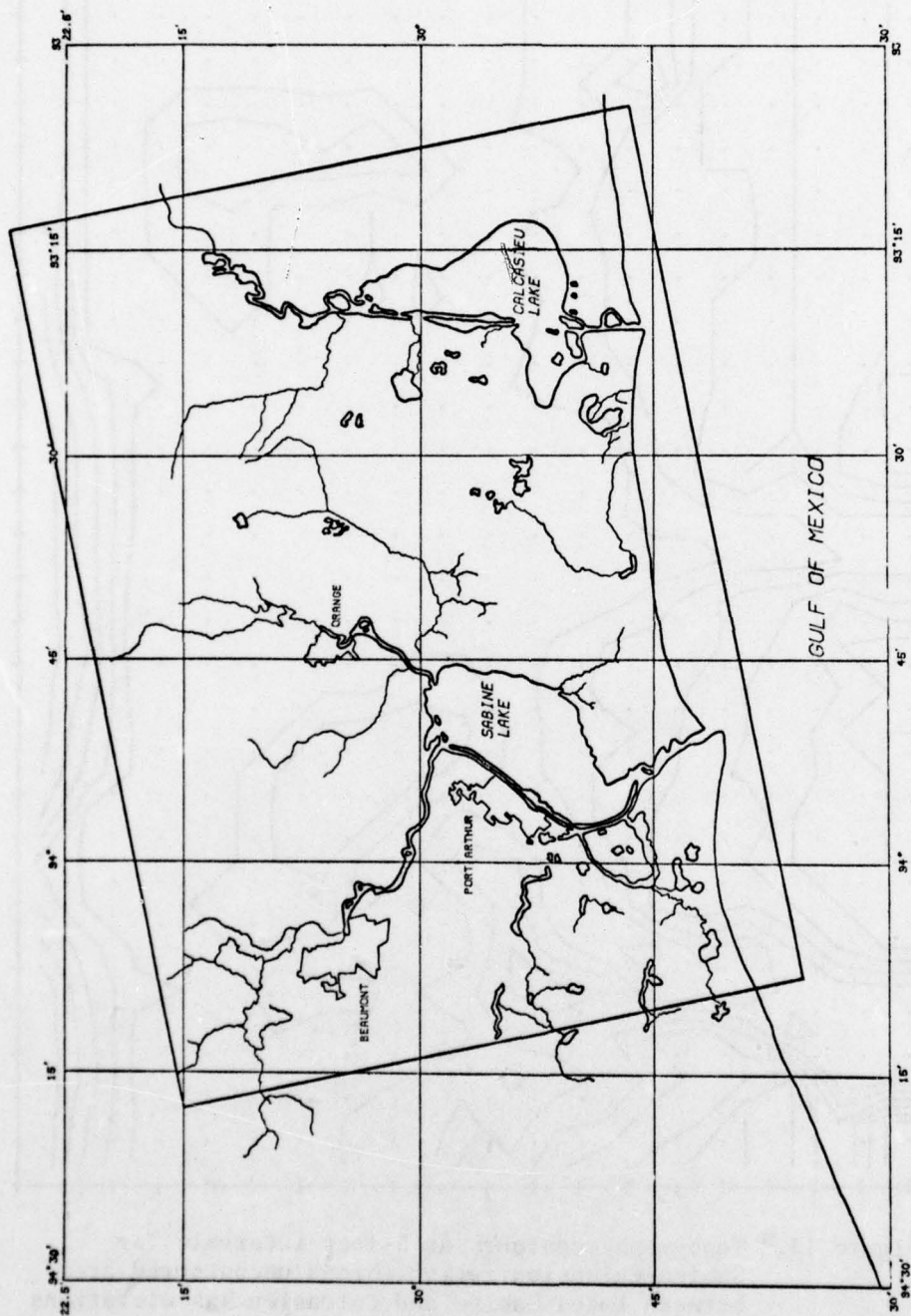


Figure 12. Map of Sabine-Calcasieu region showing grid boundary.

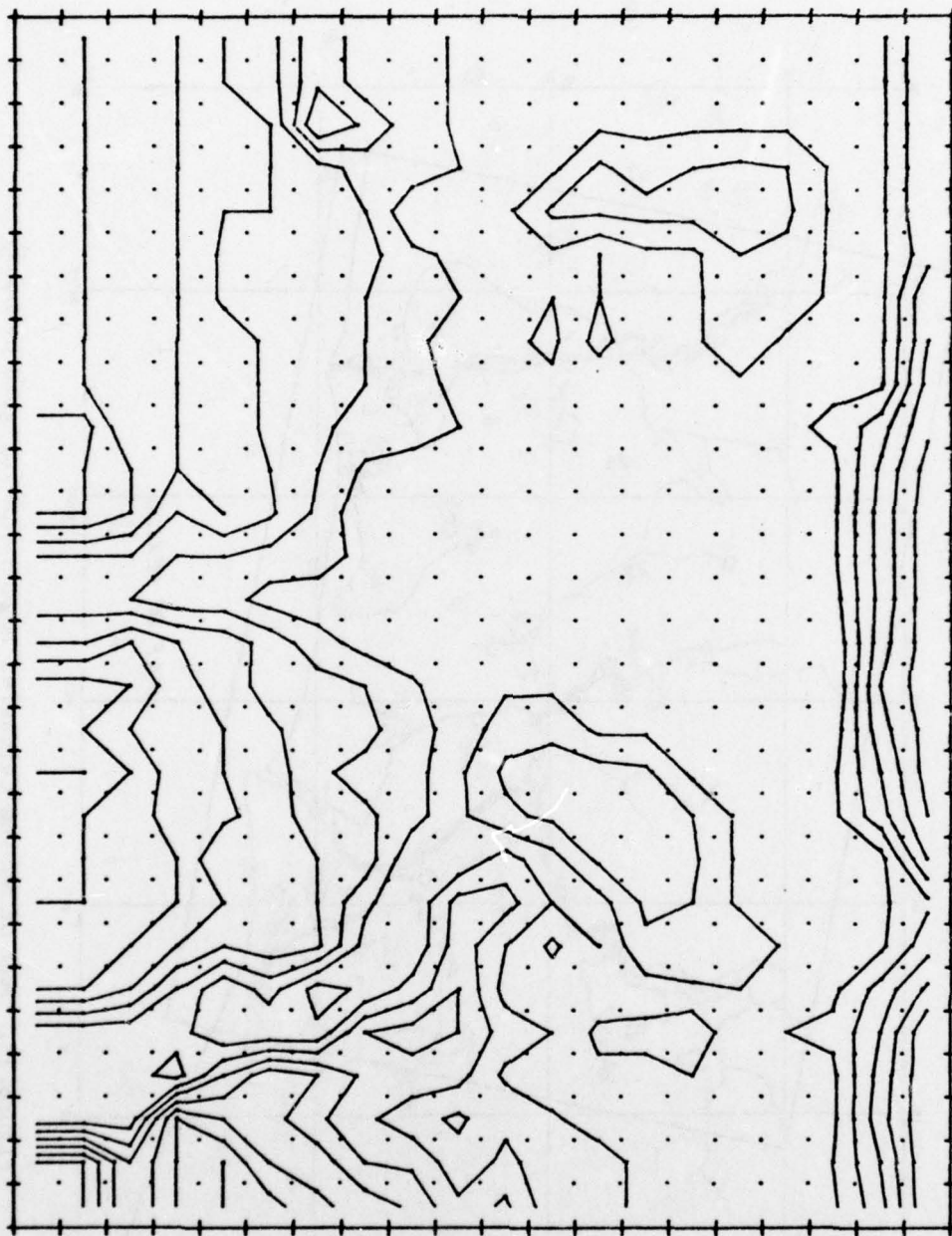


Figure 13. Topography contours at 5-foot intervals for Sabine-Calcasieu region (broad uncountoured area between Lakes Sabine and Calcasieu has elevations between 0 and 5 feet).

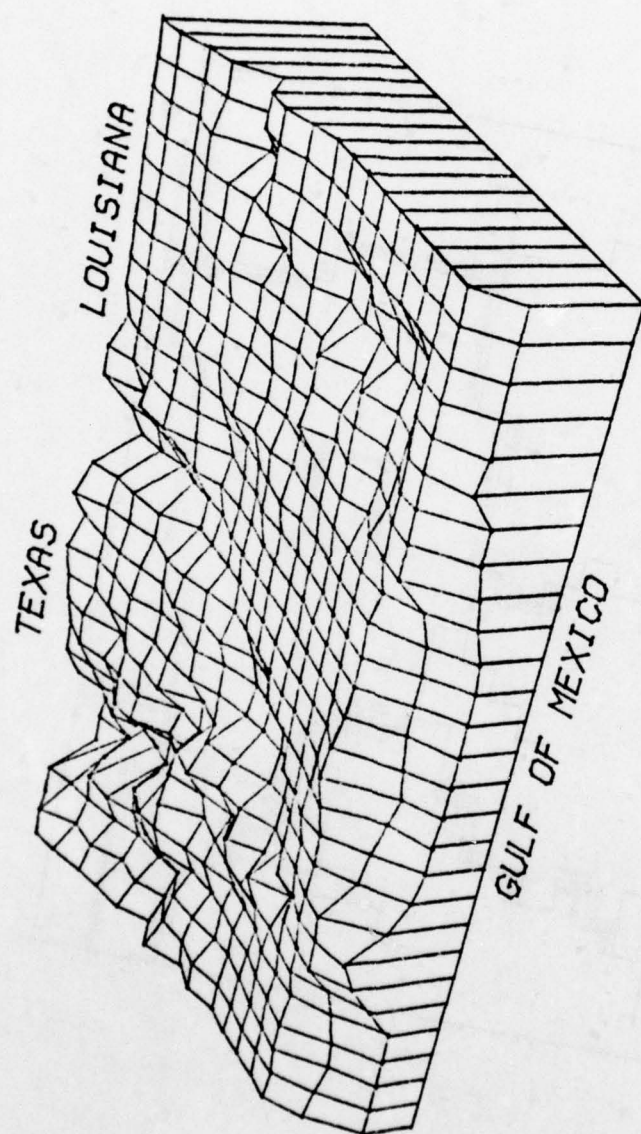


Figure 14. Topography in perspective for the Sabine-Calcasieu region.

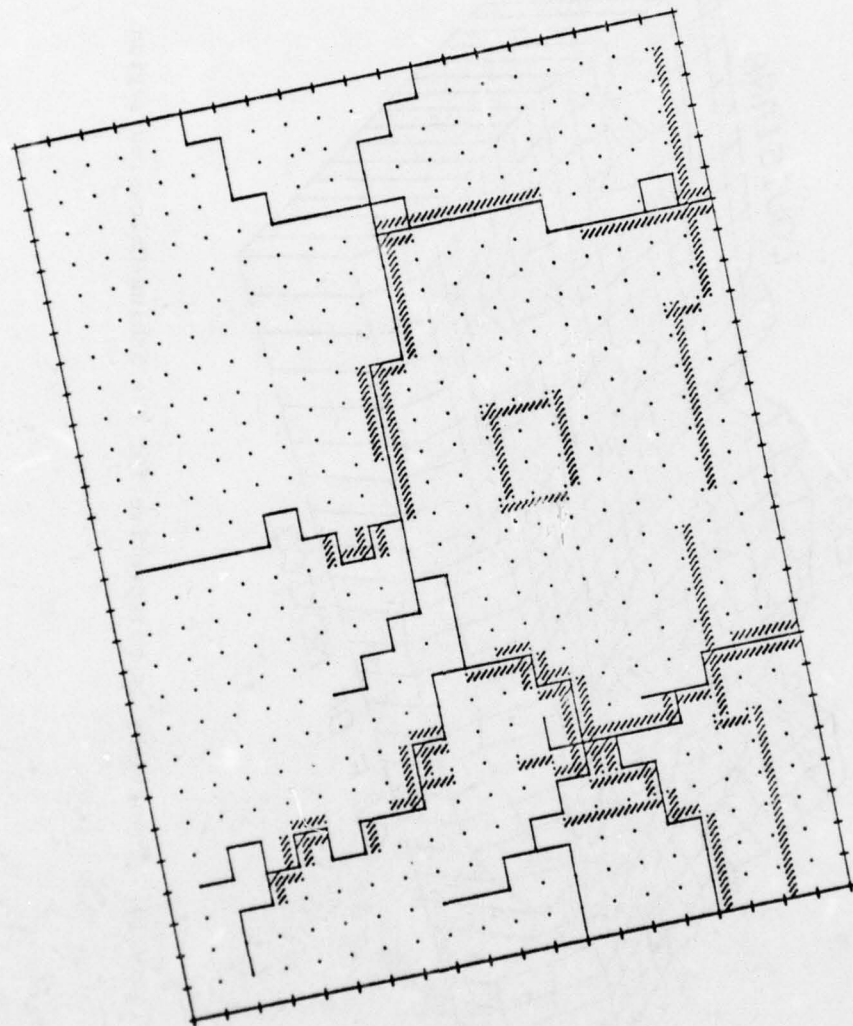


Figure 15. Overlay of grid (dots), channels (full lines) and barriers (hatched) on scale of Figure 12.

barrier data, and channel data are the same for the astrotide simulations and the standard project storms. There are 91 barrier blocks and 121 channel blocks of which 53 are common to barrier blocks. Examples of null channel blocks are for $K = 4, 6, 18, 21, 24$, etc., a total of 19.

Appendix E shows a plot of the block topography with the channel and barriers superimposed. This plot is given on two pages; x (or I) runs from left to right and y (or J) runs from bottom to the top of the page (I values are indicated along the top of both pages and J along the left side of the first page). Also in Appendix E is a listing of the key arrays for channels as generated by the program. Note that the final array size for channels is 128 (KCMP), there being 6 channels which terminate on the boundary of the grid.

As an illustration of barrier input note from Appendix E that for block (2,2) a y barrier exists, but not an x barrier. The bed elevation of block (2,2) is -10 feet while that of block (3,2) is -13 feet. Thus, a value of ZX of -10 feet should have been input for this block. The listing of the barrier input data in Appendix D gives the information for block (2,2) at $K = 12$ with $ZX = -100$ (tenths of feet) which checks. The actual barrier on the upper side indicates a positive 6 feet. However, barrier block $K = 13$ at the adjacent block (3,2) shows a ZX value of -12 feet. Reference to the topography in Appendix E indicates that this is the elevation of adjoining block (4,2) which is higher than block (3,2) and hence is the correct entry.

For an illustration of the sign coding concerning barriers along channels, refer to the channel input data in Appendix D and the plot in Appendix E. Channel block $K = 1$ located at (8,1) shows a negative $IWCX$ and a negative $IZCX$ which is the coding for double levees of equal height with the channel in between. This is the location of the double jetty entrance channel for the Sabine region. Channel block 5 at location (7,4) shows a (+,-) signature for the x channel and a (+,+) signature for the y channel. Hence, the barrier for the x channel is on the inner lateral boundary while that for the y channel is on the outer lateral boundary (see App. C,6). Reference to Appendix E key array listings shows $KCB = 37$ for channel block 5. Barrier block 37 has the same location (7,4) and indicates valid barriers of a 5-foot elevation above MSL for both the x and y channels.

3. River Input and Hydrograph Gage Locations.

There are three river discharge locations provided for the Sabine-Calcasieu region. These locations, as given in block 9 of the input (App. D), are (28,15), (4,19), and (14,19) which are respectively for the Calcasieu River near Lake Charles, the Neches River north of Beaumont, and the Sabine River north of Orange.

Nine gage locations for the astrotide calibration and Hurricane Carla simulation are shown as small circles in Figure 16. All of these with the exception of the North Sabine Lake gage are located on channels.

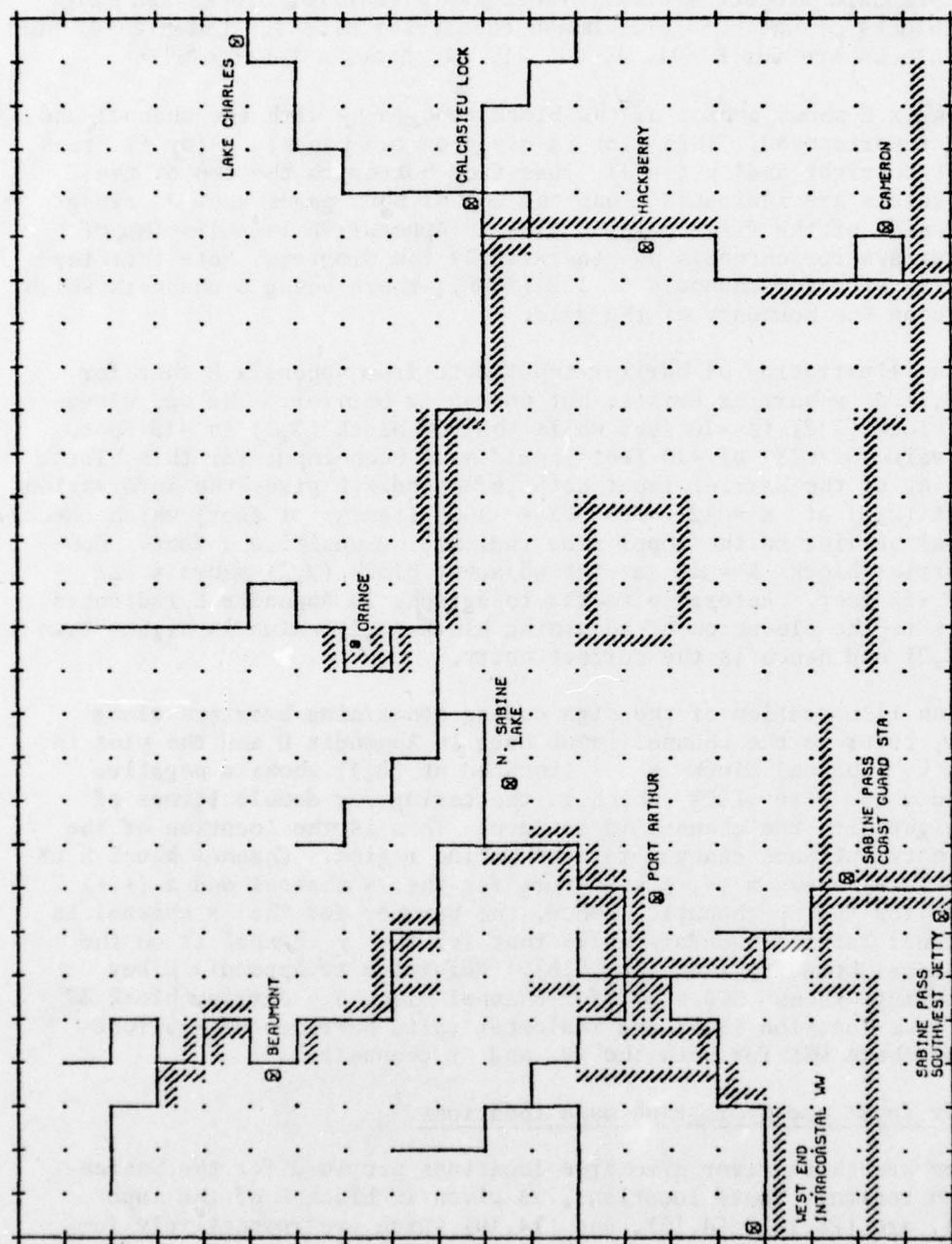


Figure 16. Plot of channels and barriers showing locations of hydrograph gages (X).

V. TIDAL CALIBRATION

1. Tide Data.

The tidal calibration is a required step in the preparation of the numerical storm surge model. These computations permit the adjustment of the parameters representing the frictional effects in the channels and low-lying regions of tidal inundation. The calibration adjusts the tidal flows in order to adequately predict proper phasing and tidal excursions in the model region. Comparisons are made with actual tide records from geographical locations corresponding to blocks or channels in the grid.

Calibration of the Sabine-Calcasieu region was carried out for the springtide conditions that existed from 0000 hours, 22 August to 0000 hours, 27 August 1973. Tide recordings at nine locations in the region were furnished by two U.S. Army Engineer Districts (Fig. 16): Sabine Pass (southwest jetty), Port Arthur, north Sabine Lake, Brakes Bayou (Beaumont), and Orange, Texas, provided by the Galveston District; Cameron, Hackberry, Calcasieu Lock, and Lake Charles, Louisiana, provided by the New Orleans District.

The tidal calibration must be accomplished during a period when the tide is effectively the only forcing function operating on the system. This requires no abnormal riverflows into the region and winds which will not substantially alter the slope of the water surfaces. Such conditions existed for the first 96 hours of the 120-hour record period and this interval was used in the tidal calibration.

2. Estimation of f_c for Entrance Channels.

Many of the bays or lagoons along the Texas coast are of such dimensions that their largest natural period is small compared with the tide period. Moreover, virtually all have narrow connections with the Gulf of Mexico. These two features conspire to produce a reduction of tidal range and a significant lag within the bay compared with the gulf tide. In addition, the tidal range is nearly uniform throughout the bay except possibly in some of the upper reaches of adjoining rivers. For these systems, the approximate response can be calculated in terms of the channel-friction coefficient, f_c , (or discharge coefficient) plus appropriate dimensions of the bay and entrance channel (Love, 1959). These relations can be used to get at least a preliminary estimate of f_c from the observed response.

Consider a bay of total MSL surface area, A_s , which is connected to the sea by a channel of cross-sectional area A_c , surface width W , effective depth D_c (defined as A_c/W), length L_c , and channel-bed friction coefficient f_c .

Let H be the volumetric response in the bay at time t (where $H \cdot A_s$ represents the impounded tidal volume above MSL at time t); let Q be the tidal flux from the sea to the bay. Then,

$$A_s \frac{dH}{dt} = Q. \quad (68)$$

Neglecting the inertia effects in the channel for the slow tidal variation, the slope force in the channel is balanced by friction at any time t ; thus,

$$H_g - H = m|Q|Q, \quad (69)$$

where H_g is the given tide level at time t outside the bay entrance and m is a dimensional constant for the system given by

$$m = \frac{fL}{gD_c A_c^2}. \quad (70)$$

This can also be written in the form,

$$m = \frac{1}{g(C_d \cdot A_c)^2},$$

where C_d is the discharge coefficient characterizing the constricted opening between bay and sea.

Assuming the input tide H_g is simple harmonic with period T and amplitude a_0 then,

$$H_g = a_0 \cos \omega t, \quad (71)$$

where $\omega = 2\pi/T$. Ignoring the second-order compound tide due to non-linearity in equation (69), the response will be roughly of the form,

$$H = ra_0 \cos (\omega t - \phi), \quad (72)$$

where ϕ is a phase lag and r is the relative amplitude response. If these are substituted into equations (68) and (69) and the quantity $|Q|Q$ expanded in the Fourier series form, it can be shown that

$$\text{and} \quad r = \cos \phi \quad (73)$$

$$\sin \phi = \frac{\sqrt{1 + B^2 - 1}}{B}, \quad (74)$$

where

$$B = \frac{8}{3\pi} m(A_s \omega)^2 a_0 \quad (75)$$

(Love, 1959). A plot of r and ϕ versus the dimensionless parameter B is shown in Figure 17. The timelag of the high tide in the bay relative to that outside the bay is simply $T = \phi/2\pi$, for ϕ in radians (or $T = \phi/360$ for ϕ in degrees).

Thus, if r or ϕ is estimated from observations it is possible to get an estimate of B . Generally, the value obtained from the observed r will differ from that obtained from the observed ϕ ; in this event an average of the values of B can be used to estimate f_c . In terms of B , f_c is given by

$$f_c = \frac{3\pi}{8} \frac{gB}{\alpha^2 A_s^2 \omega^2 a_o}, \quad (76)$$

where

$$\alpha^2 = \frac{L_c}{D_c A_c^2}. \quad (77)$$

It is emphasized that the above analysis pertains to a bay system connected to the sea by a single channel of uniform dimensions. The results can be generalized for the case of a series of N channels of different dimensions or of N channels in parallel or combination of both (as in the Sabine-Calcasieu system) by using an effective value of α^2 .

Let α_n^2 designate the value of α^2 for an individual channel as evaluated by equation (77). Then, the effective value of α^2 for a series of N channels is simply

$$\alpha_s^2 = \sum_{n=1}^N \alpha_n^2. \quad (78)$$

However, for N channels in parallel the effective α^2 is given by

$$\alpha_p^2 = \left(\sum_{n=1}^N \alpha_n^{-1} \right)^{-2} \quad (79)$$

For a series containing a parallel subset, the effective α^2 , for the latter is used in equation (78). If two or more complex entrance channels are in parallel then the effective values of α are used in place of α_n in equation (79).

The use of this procedure will be illustrated for the Sabine-Calcasieu system. In the numerical simulation scheme there is a total of 40 blocks of 2×2 nautical miles covered with water and in communication with the sea. This represents a surface area of 5.91×10^9 square feet. In addition, the channels contribute a total of 0.64×10^9 square feet. Thus, the total surface area for the combined system is $A_s = 6.55 \times 10^9$ square feet (3.77×10^9 square feet for the Sabine part and 2.78×10^9 square

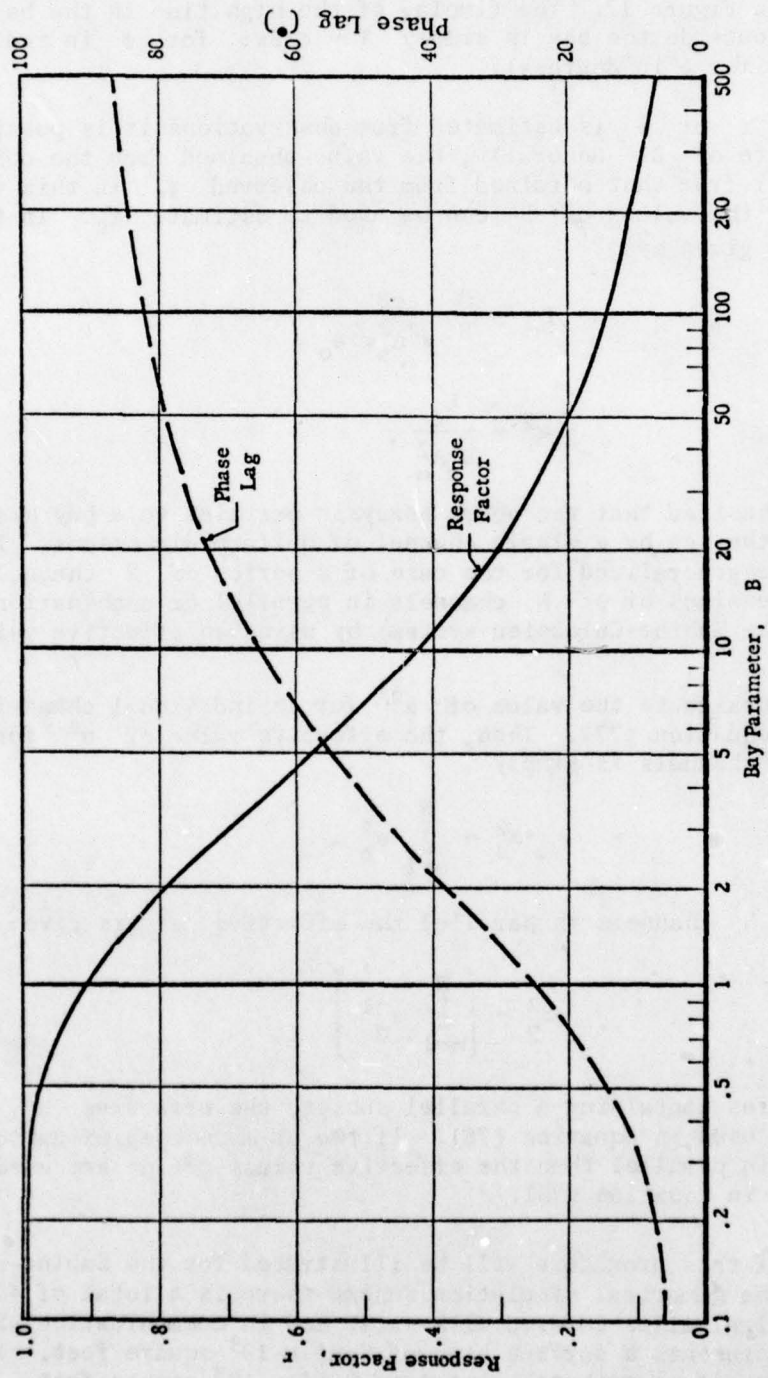


Figure 17. Amplitude response factor (r) and phase lag (ϕ) versus the dimensionless parameter B characterizing a constricted bay.

feet for the Calcasieu part). The two parts of the system are coupled via the Intracoastal Waterway and their responses are about the same, so the combined system is treated as one.

A summary of data and calculations pertinent to the entrance channels for the Sabine-Calcasieu system is given in Table 2 (see also Fig. 15 and App. D). The simulated Sabine Pass between the gulf and Lake Sabine consists of two sections (1 and 2 in Table 2) of different dimensions in series. However, Calcasieu Pass consists of a pair of parallel channels (4 and 5 in Table 2) in series with a simple channel (3 in Table 2). The individual α^2 for each channel is also shown in Table 2. The effective α^2 for Sabine Pass is the first partial sum shown in the last column. The effective value of α^2 for the parallel part of Calcasieu Pass is shown in the last column, opposite entries 4 and 5. The effective value for Calcasieu Pass is the partial sum indicated in the last column. The effective value for the entire pass system is evaluated from the Sabine Pass and Calcasieu Pass values, using equation (79) for parallel systems:

$$\alpha^2 = 0.32 \times 10^{-6} \text{ (square feet)}^{-1} .$$

Table 2. Data on simulated Sabine Pass and Calcasieu Pass.

n	W_c (ft)	D_c (ft)	A_c (ft ²)	L_c (ft)	$\alpha^2 \times 10^6$ (ft ⁻²)	$\alpha^2 \times 10^6$ (ft ⁻²)
Sabine Pass						
1	2,330	20	46,600	24,360	0.561	0.561
2	2,860	21	60,060	36,480	0.482	0.482
Subtotal						1.043
Calcasieu Pass						
3	800	32	25,600	24,360	1.162	1.162
4	500	40	20,000	12,160	0.760	0.455 ¹
5	1,000	16	16,000	34,480	8.960	
Subtotal						1.617

¹Evaluated by parallel channel relation.

The observed ranges and times of minimum tide for 25 August 1973 for the Sabine-Calcasieu system are given in Table 3. Gage 1 is used as the input gulf tide. The average of all other gages is used as the response. The indicated amplitude response is

$$r = \frac{1.50}{2.59} = 0.58 .$$

Using a tidal period of 25 hours the indicated phase lag is

$$\phi = (20.8 - 17.5) \frac{360}{25} = 47^\circ .$$

Table 3. Ranges and times (c.d.t.) of available observed tides in the Sabine-Calcasieu system for 25 August 1973.

Gage No.	Place	Range (ft)	Time (hr)
1	Sabine Pass, southwest jetty	2.59	17.5
2	Port Arthur	1.53	19.0
3	North Sabine Lake	1.40	21.5
4	Beaumont	1.52	21.5
5	Orange	1.40	23.0
6	Cameron	2.05	17.5
7	Hackberry	1.06	22.0
8	Calcasieu Lock, west	1.45	20.5
9	Lake Charles	1.60	21.5
Average of 2 to 9, inclusive		1.50	20.8

From Figure 17 the corresponding values of B are 4.7 and 3.6, respectively, with an average of 4.1. The tidal frequency is

$$\omega = \frac{2\pi}{25 \times 3,600} = 7.0 \times 10^{-5} \text{ radians per second}$$

and $a_0 = 2.57/2$ or 1.3 feet. Consequently, the estimated f_c for the entrance channels is from equation (76): $f_c = 0.0018$.

The final selected value of f_c for the entrance channels is 0.0015 as determined by trial runs. This is somewhat less than the above estimate. The difference might be accounted for by the fact that the tidal hydrograph is not really simple harmonic but contains compound tides (of higher frequency) giving the sharp minimum and broad or double-peaked maxima. The effective frequency is consequently somewhat greater than the ω given above, thus yielding a smaller f_c closer to 0.0015.

3. Final Calibration for Tide.

The major control on the response of the bay to the tides are the dimensions and friction factor for the entrance channels as discussed above. In this connection, it should be pointed out that channel dimensions (width and depth) were taken such that the average cross-sectional area (under MSL conditions) for a given reach is represented by the product of these dimensions. Thus, if the depth is taken as the mean for the reach, then the width will be somewhere between the width of the dredged channel and the surface width of the natural channel.

The values of channel friction for the remaining channels and of the block friction were selected by a trial-and-error procedure, starting with a uniform value throughout. The final values of channel friction for the upper reaches of the Neches and Sabine Rivers were taken as

0.0025 to give a reasonable agreement for the Beaumont and Orange tide response; it was necessary to use a low value (0.0005) for the upper reach of the Calcasieu River to reproduce the Lake Charles tidal hydrograph. The latter three gages (Beaumont, Orange, and Lake Charles) have connections to the inner bay areas only via channels, hence their responses are fairly sensitive to the channel friction. The low value for the Calcasieu River may be due to underestimates of the effective channel widths, which would demand a less than normal friction factor.

The block friction for the tide calculations was taken as 0.0015 to get a reasonable agreement for the north Sabine Lake gage. However, later calculations for the Hurricane Carla simulation (which is more sensitive to block friction than the astrotide) indicated that 0.0025 (as used in the Galveston Bay simulations) was more appropriate.

The results of the final astronomical tide simulations for a 96-hour period starting 0000 hours c.d.t., 22 August 1973, are given in Figures 18 to 26, and Appendix F. The input tide (Fig. 18) corresponds to the observed tide for the period at Sabine Pass (southwest jetty). In the subsequent eight figures the computed (full line) and observed (line with circles) are compared for the eight different gages within the system; the gages are identified in the figures. Note that the observed values for each gage have been adjusted with respect to a local datum, taken as the gage mean for a 120-hour period starting 0000 hours c.d.t., 22 August 1973. In all cases, the computed ranges are in fairly good agreement with the observed; however, there seems to be a consistent tendency for the computed to lag the observed. This might be due to a possible time-shift error for the input gage. Although a lowering of the frictional coefficient for the entrance channel would decrease the lag within the system, it would also increase the range of the tide everywhere in the system. It was felt that it was more important to reproduce the range than the times of high and low water, and hence the value of $f_c = 0.0015$ for the entrance channels was retained.

For the upper Calcasieu River (Figs. 25 and 26) the computed water level (which refers to a common MSL datum for the system) and the observed water level display an apparent vertical shift. This could be related to possible wind effects in the second part of the record, which have been ignored in the computations.

The steady river discharges adopted in the astrotide runs were 800 cubic feet per second for Calcasieu River, 1,100 cubic feet per second for Neches River, and 1,500 cubic feet per second for Sabine River.

Serial listings of the computed water levels at the gages discussed above are given in Appendix F, along with listings of volume transport at six channel positions. Flow at points 1 and 2 correspond to input (if positive) to the system through Sabine Pass and Calcasieu Pass, respectively. Since the tide amplitude is less than the seaward barriers, the two passes represent the only source of water for normal conditions.

ASTRONOMICAL TIDE

STARTING 0000 CDT, AUG. 22, 1973

DATUM- INDIVIDUAL GAGE MEAN
WATER LEVEL FOR THE 120 HR.
PERIOD STARTING 0000 HRS.,
AUG. 22, 1973

□ Observed
— Calculated

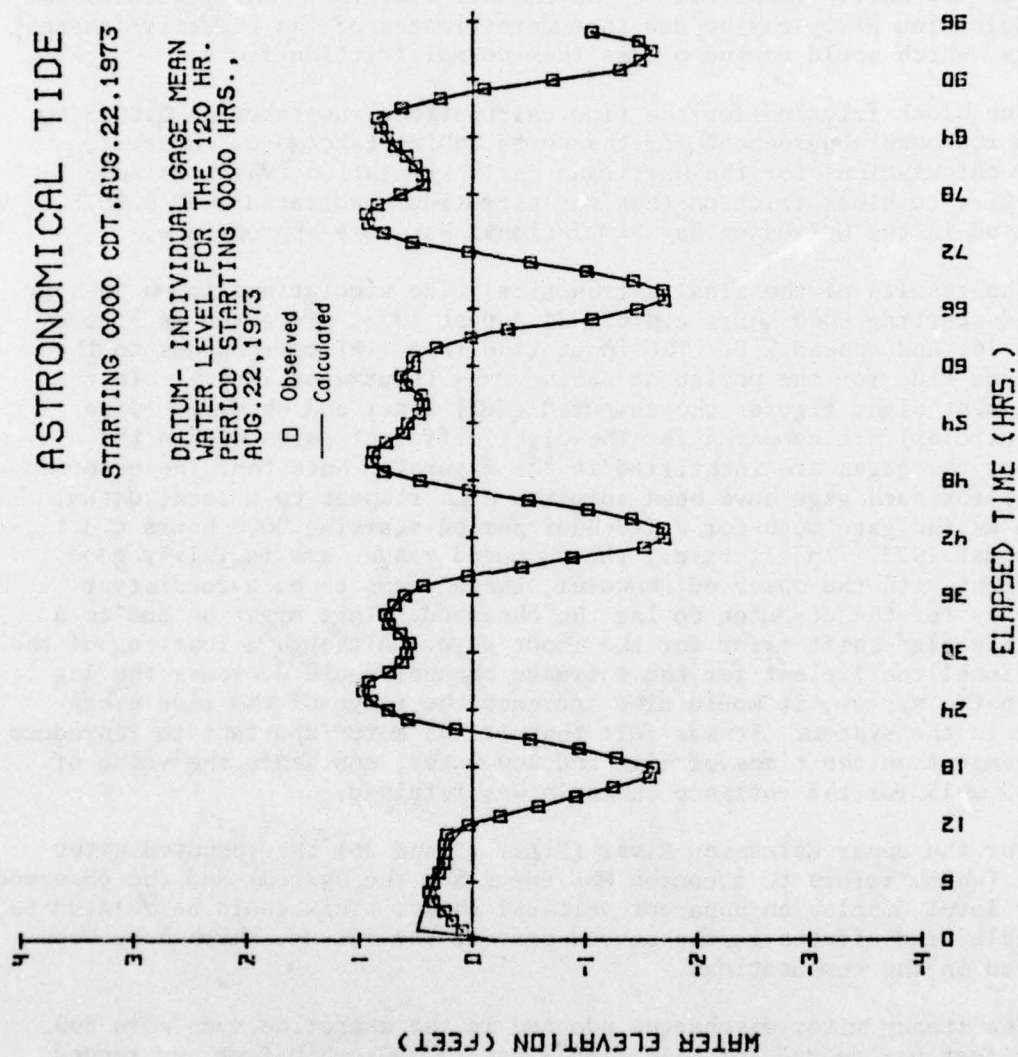


Figure 18. Astronomical tidal hydrograph for Sabine Pass, southwest jetty
(input for tide calibration).

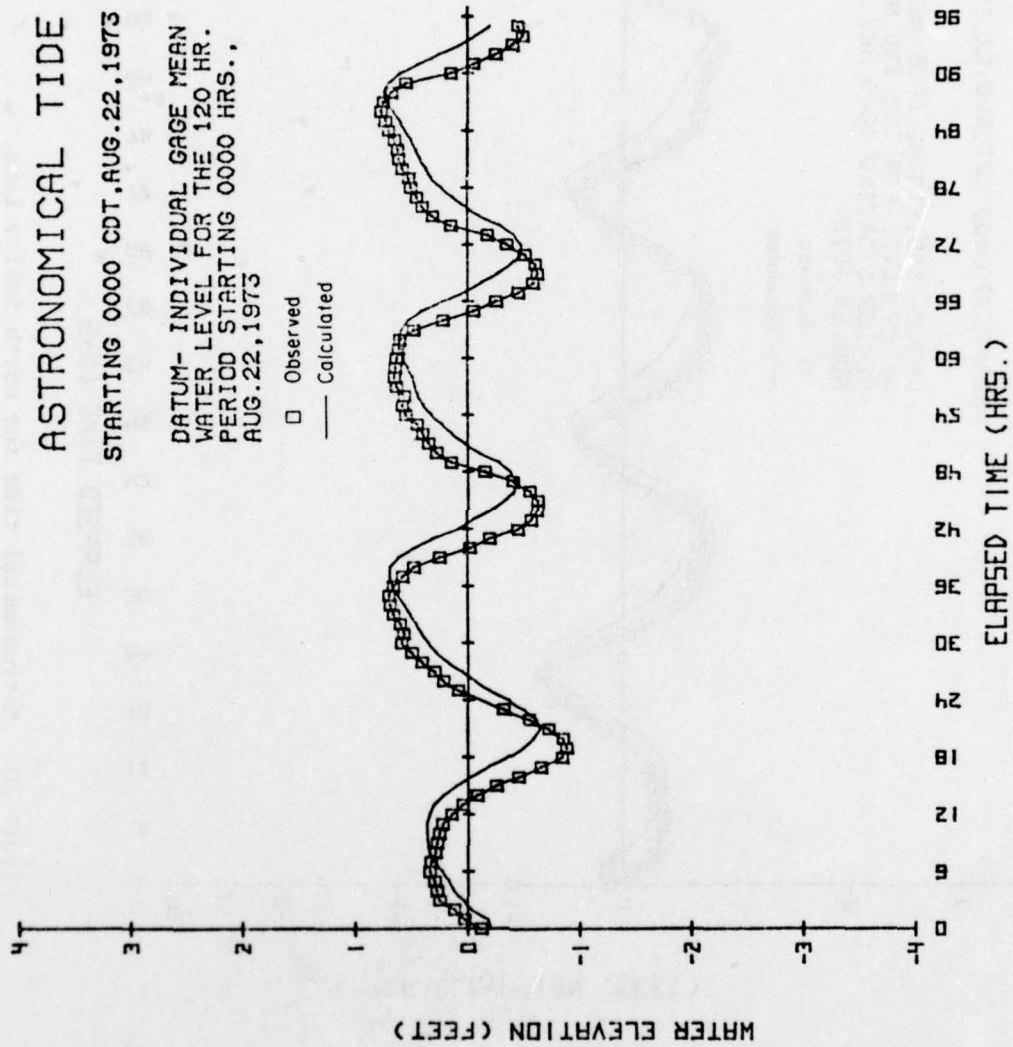


Figure 19. Astronomical tide for Port Arthur corresponding to input of Figure 18.

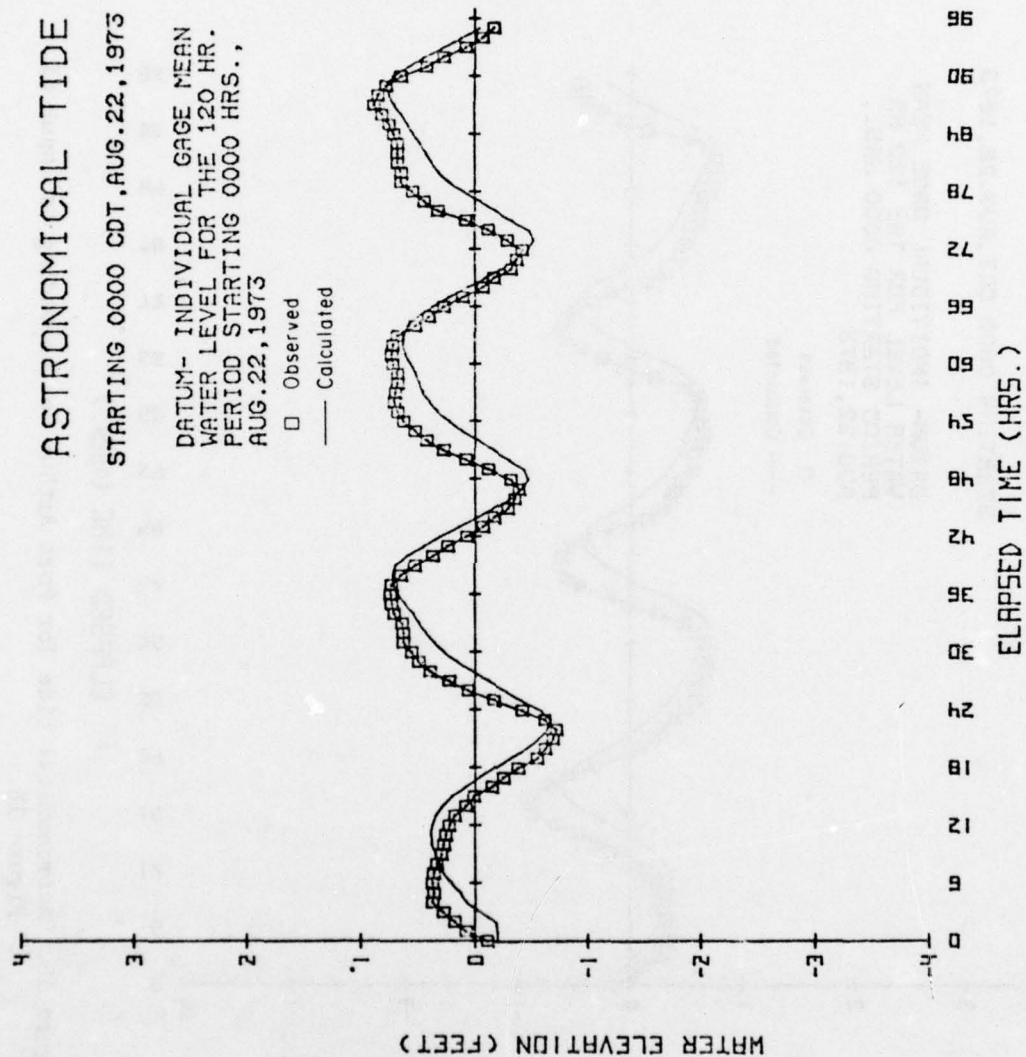


Figure 20. Astronomical tide for north Sabine Lake.

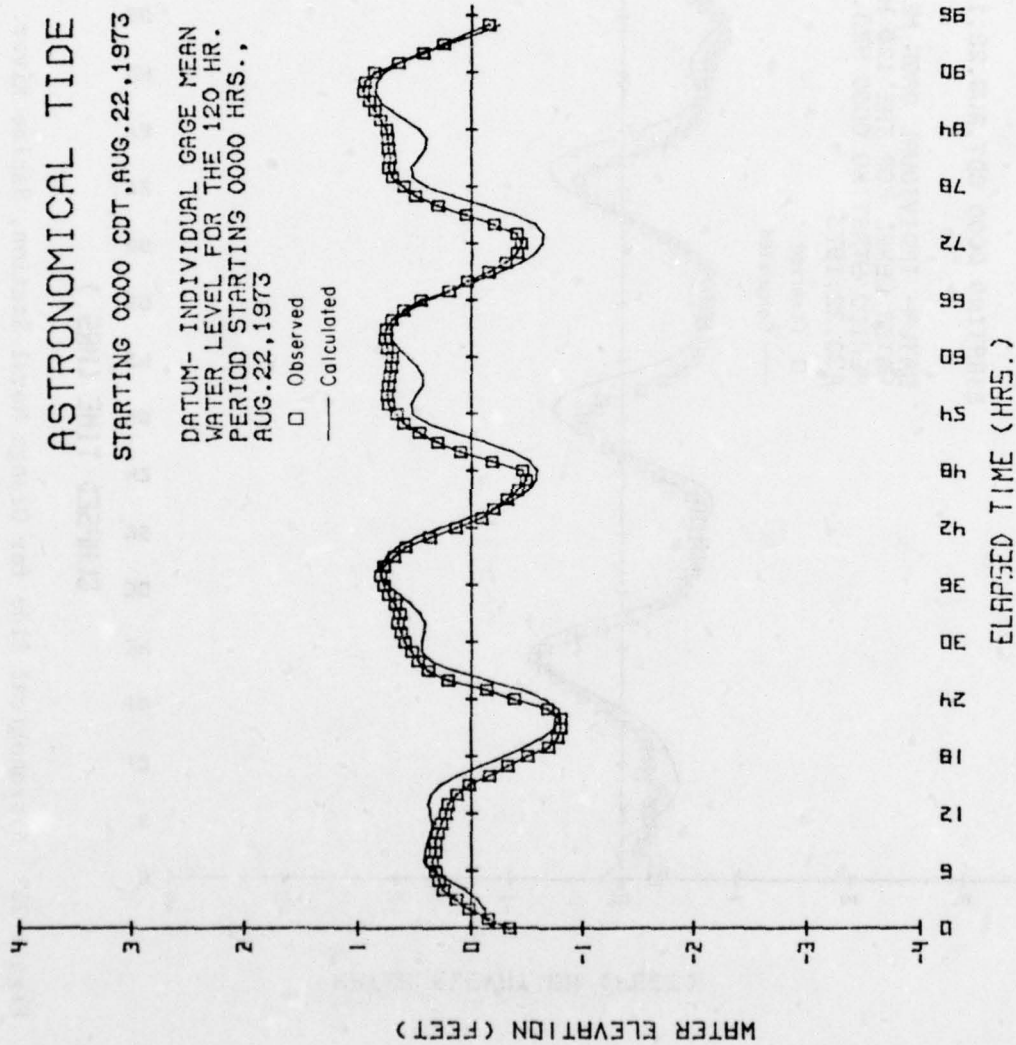


Figure 21. Astronomical tide for Beaumont, Neches River, and Brakes Bayou.

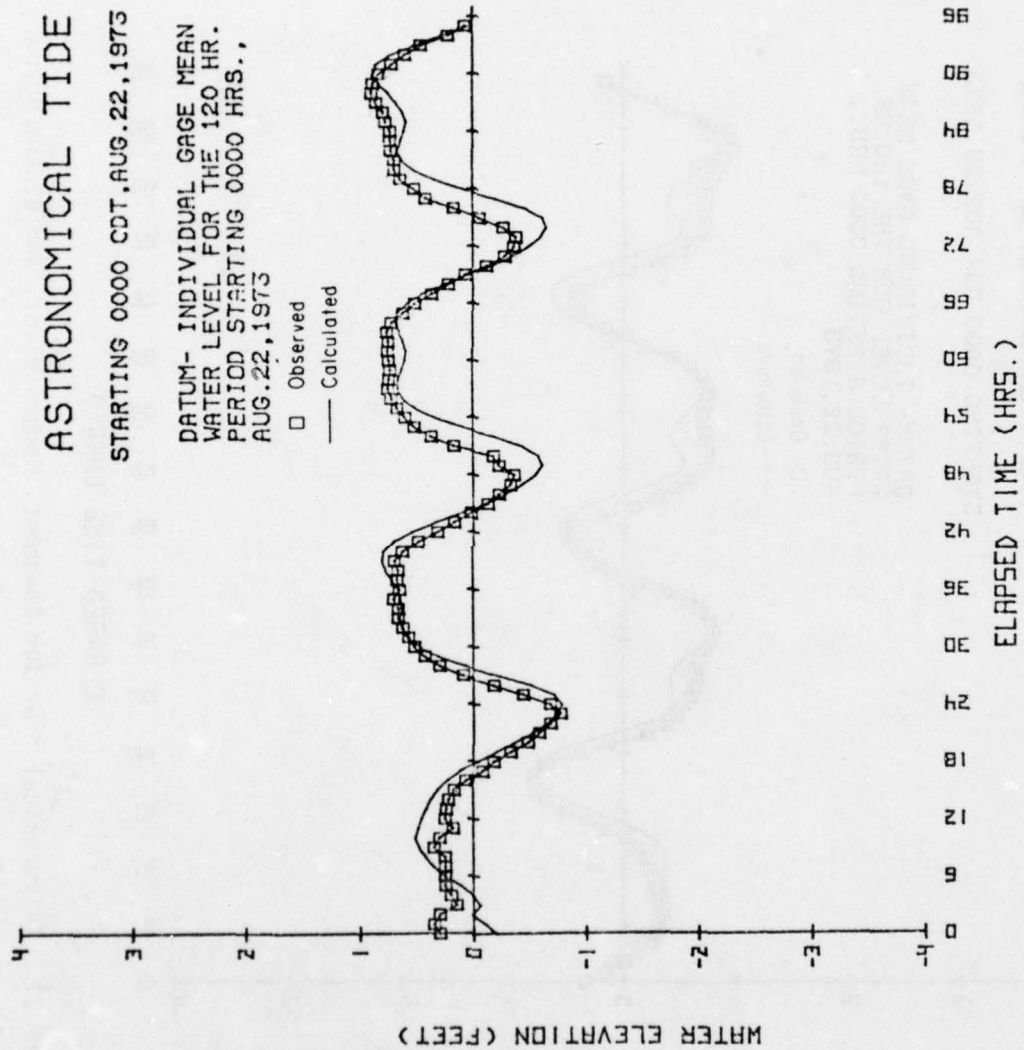


Figure 22. Astronomical tide for Orange Naval Station, Sabine River.

ASTRONOMICAL TIDE

STARTING 0000 CDT, AUG. 22, 1973

DATUM- INDIVIDUAL GAGE MEAN
WATER LEVEL FOR THE 120 HR.
PERIOD STARTING 0000 HRS.,
AUG. 22, 1973

□ Observed
— Calculated

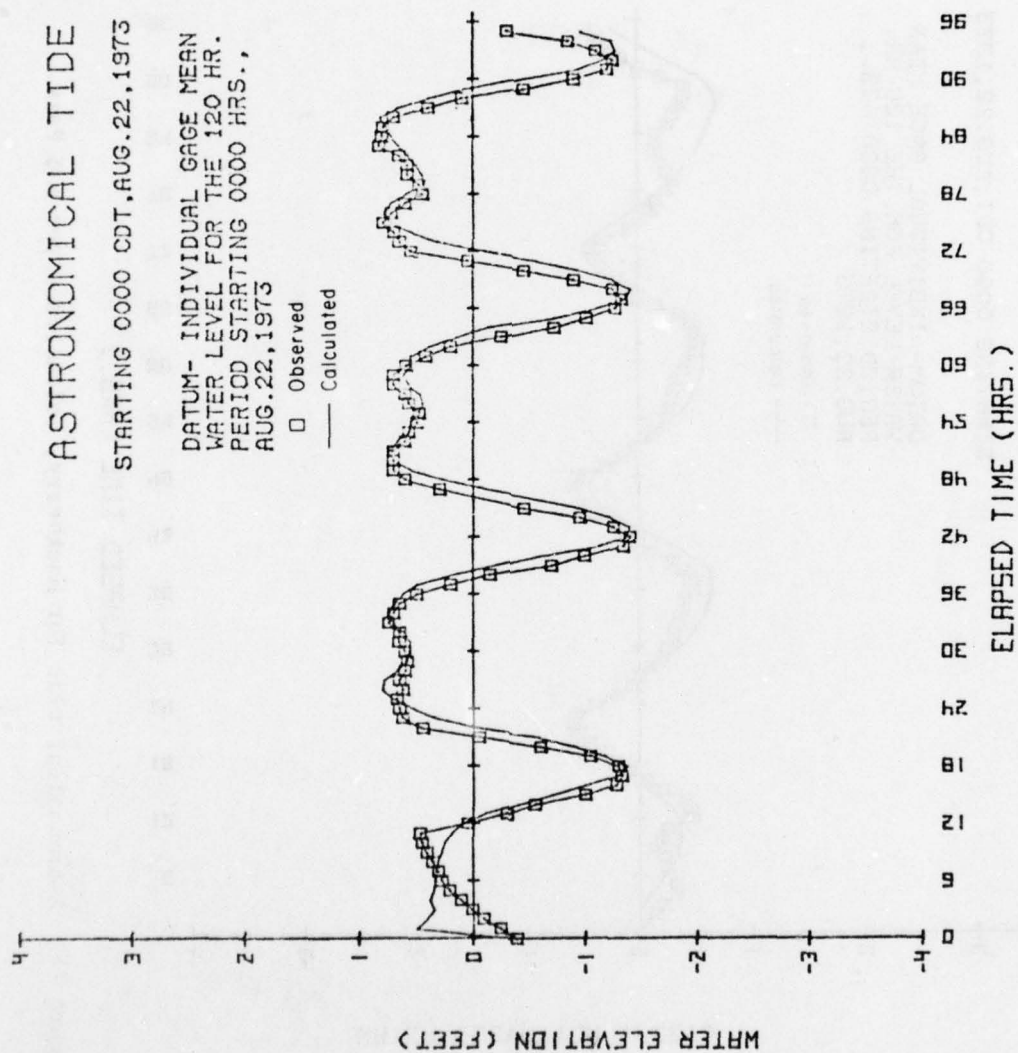


Figure 23. Astronomical tide for Cameron, Calcasieu Pass.

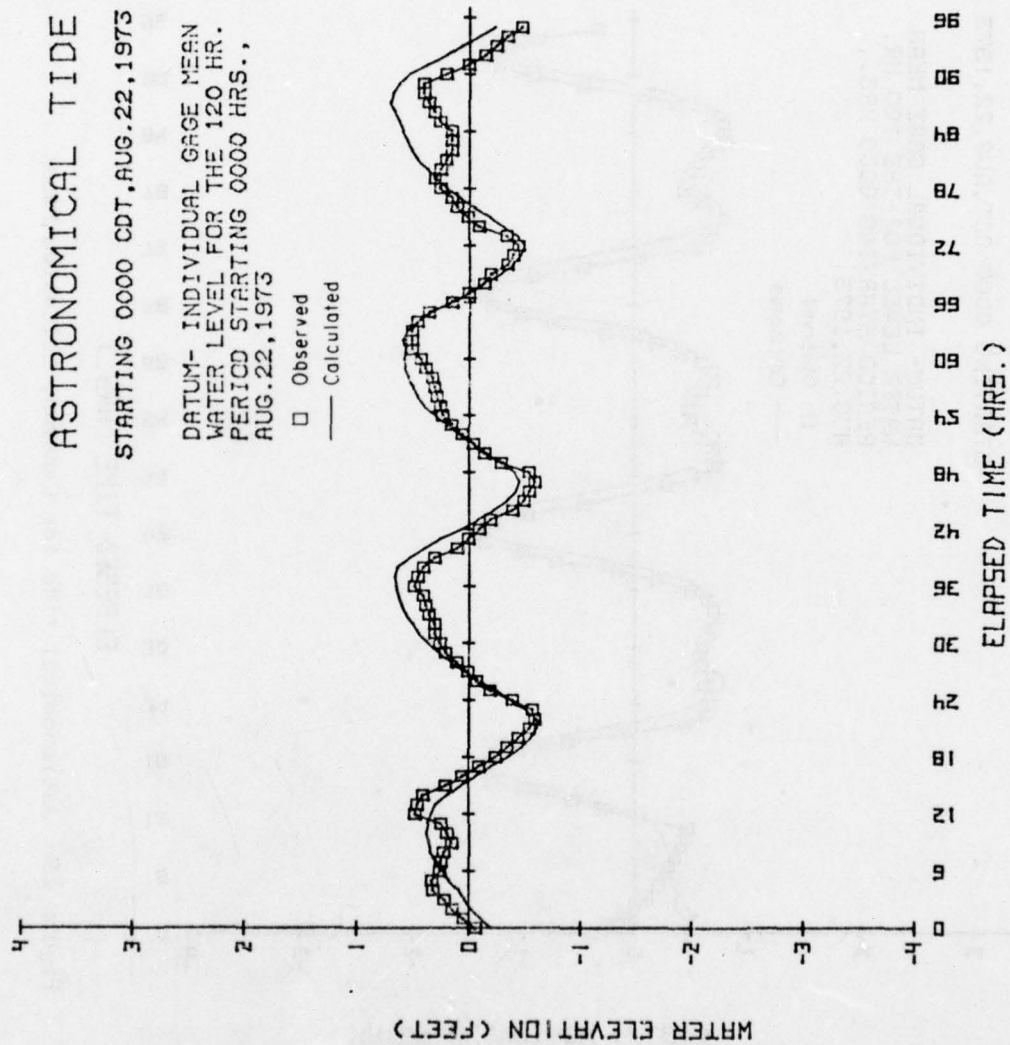


Figure 24. Astronomical tide for Hackberry, Calcasieu River and Pass.

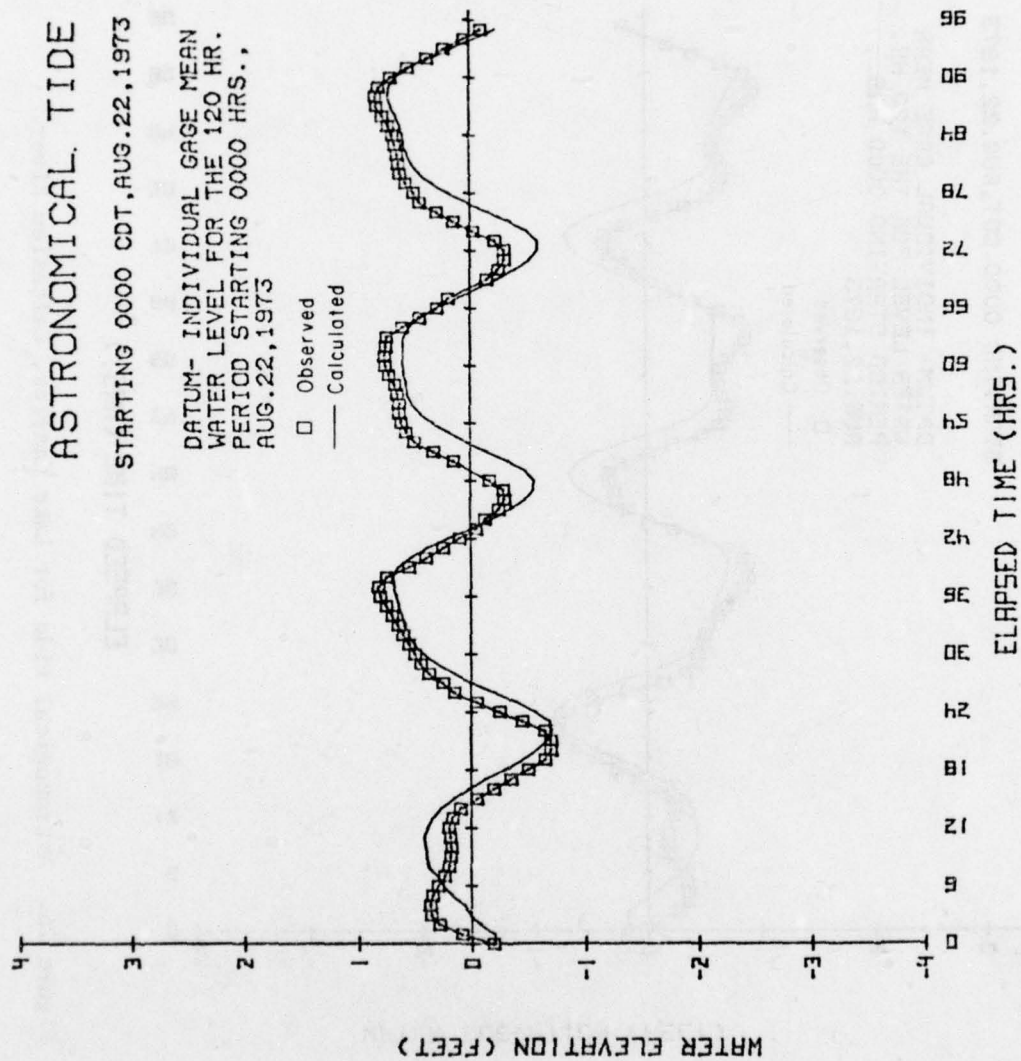


Figure 25. Astronomical tide for Intracoastal Waterway at Calcasieu Lock, west.

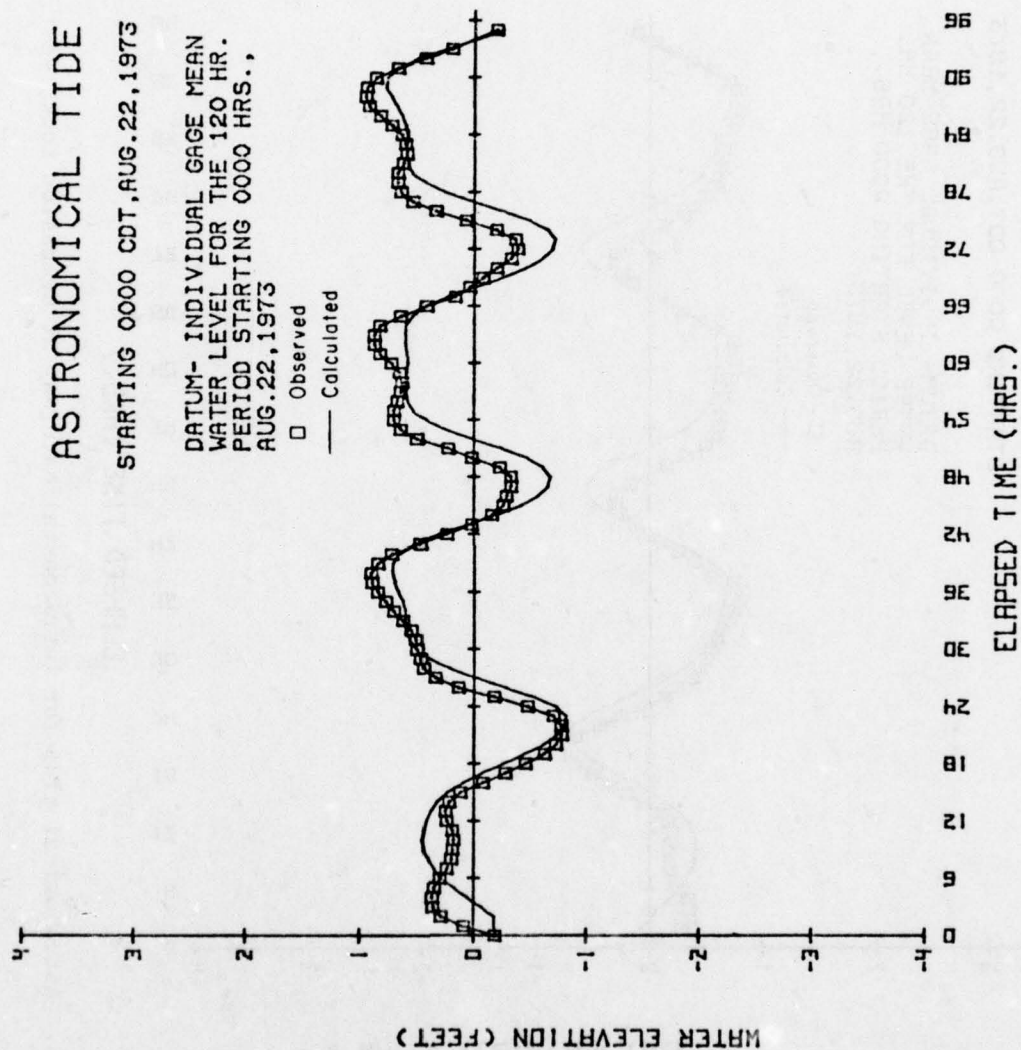


Figure 26. Astronomical tide for Lake Charles, Calcasieu River.

Reproductions of channel output at three different times (30, 60, and 90 hours from start) are shown in Appendix F. The output shows flows (in cubic feet per second), direction of flow, and water level along the various channel reaches at the specified times.

VI. HURRICANE CARLA VERIFICATION

1. Forcing Function Input.

a. Wind-Stress Fields. The x and y components of the wind stress for each 3 hours in a 72-hour period for an 8 by 6 coarse grid for Hurricane Carla are given in the input listings in Appendix D. For convenience in spotting possible errors in input, the wind-stress vectors were plotted, based on the above input, by a special subprogram. Samples of these plots for each 12 hours are shown in Figures 27 to 32. The plots showed suspect entries, which were subsequently corrected before any runs were attempted, and have I increasing upward and J increasing to the left; i.e., the seaward boundary is on the right.

b. River Discharge Input. The river discharges for the Calcasieu River, Neches River, and Sabine River for each 3 hours are listed as block (IDENT) 12 in Appendix D.

c. Gulf Hydrograph Input. The final input for HG, the water level input along the seaward boundary, was taken as interpolated values between Sabine Pass and Calcasieu Pass with input sequences at those passes adjusted to match the observed values at the Sabine Pass U.S. Coast Guard Station and Cameron after some modification due to flow through these passes. The input is given sequentially at 3-hour intervals along with the wind-field input in Appendix D.

2. Further Adjustments and Results.

a. Adjustments. In the series of runs for the Hurricane Carla simulation, it was necessary to make some adjustments in the block topography, particularly in the upper reaches of the Neches River, in order to provide more ponding area at the levels of flooding encountered. These changes, which are reflected in the final topography (App. D), do not change the results of the astronomical tide calibration because the changes were at levels well above those encountered with the astrotide runs.

A further modification was the reduction of the wind-stress values to 80 percent of those shown in the listings and in the vector plots for the upper left-hand region of the grid. Specifically for I.LE.3 and J.GE.4, the wind-stress components were so reduced in the final runs for Hurricane Carla. This reduction was also used in the later application for Standard Project Hurricane (SPH) simulations. The rationale for this adjustment is based on the greater sheltering in this region due to both topography and vegetation. The initial H for all locations in the bay was taken as 3.2 feet.

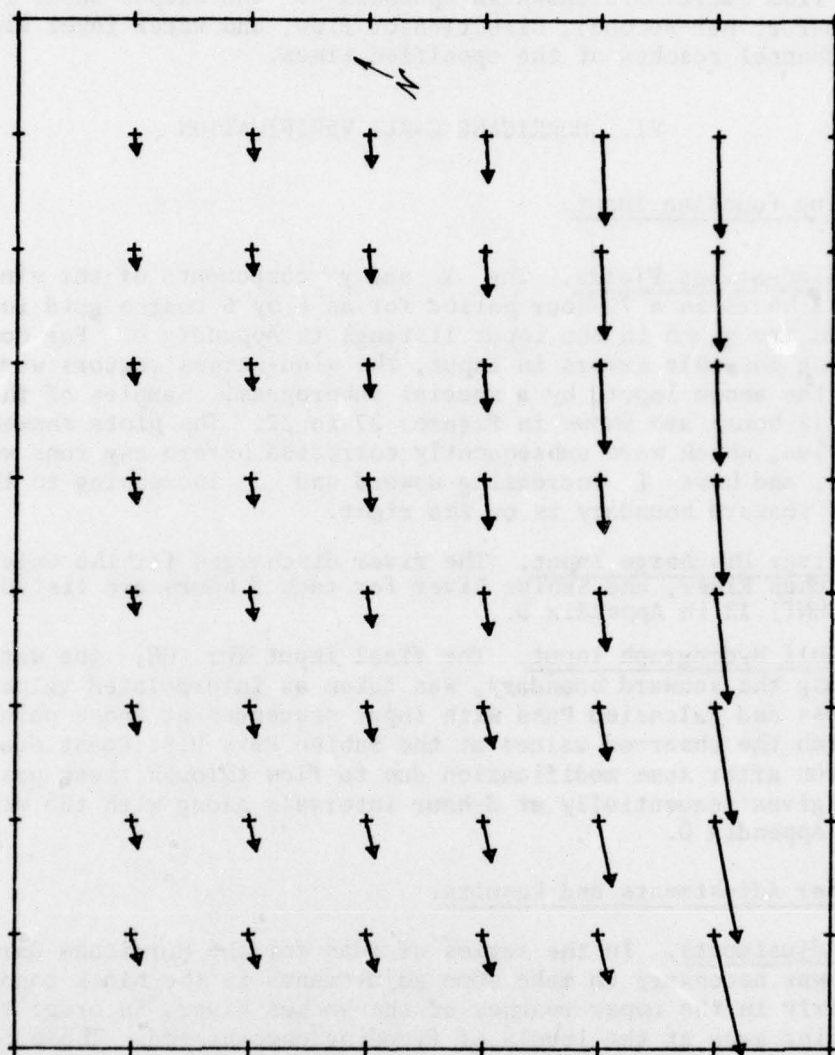


Figure 27. Wind-stress vectors for Hurricane Carla, over Sabine-Calcasieu region on an 8-nautical mile grid; time = 12 hours.

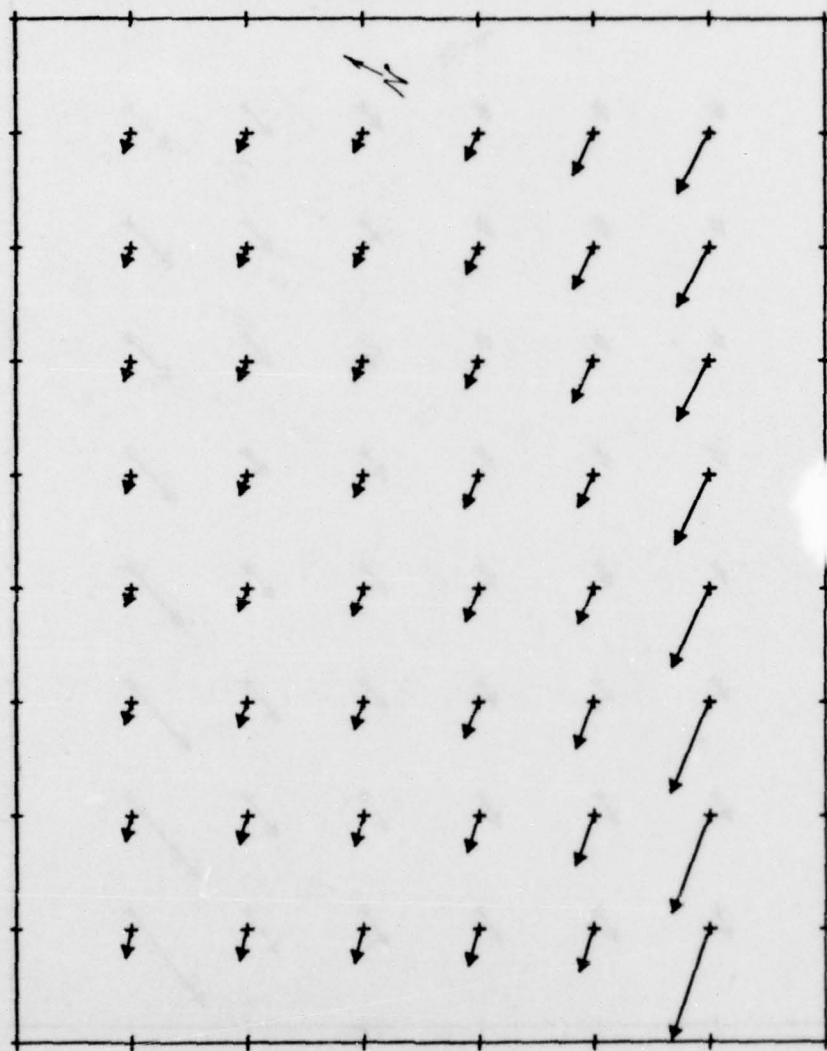


Figure 28. Wind-stress vectors for Hurricane Carla, over Sabine-Calcasieu region on an 8-nautical mile grid; time = 24 hours.

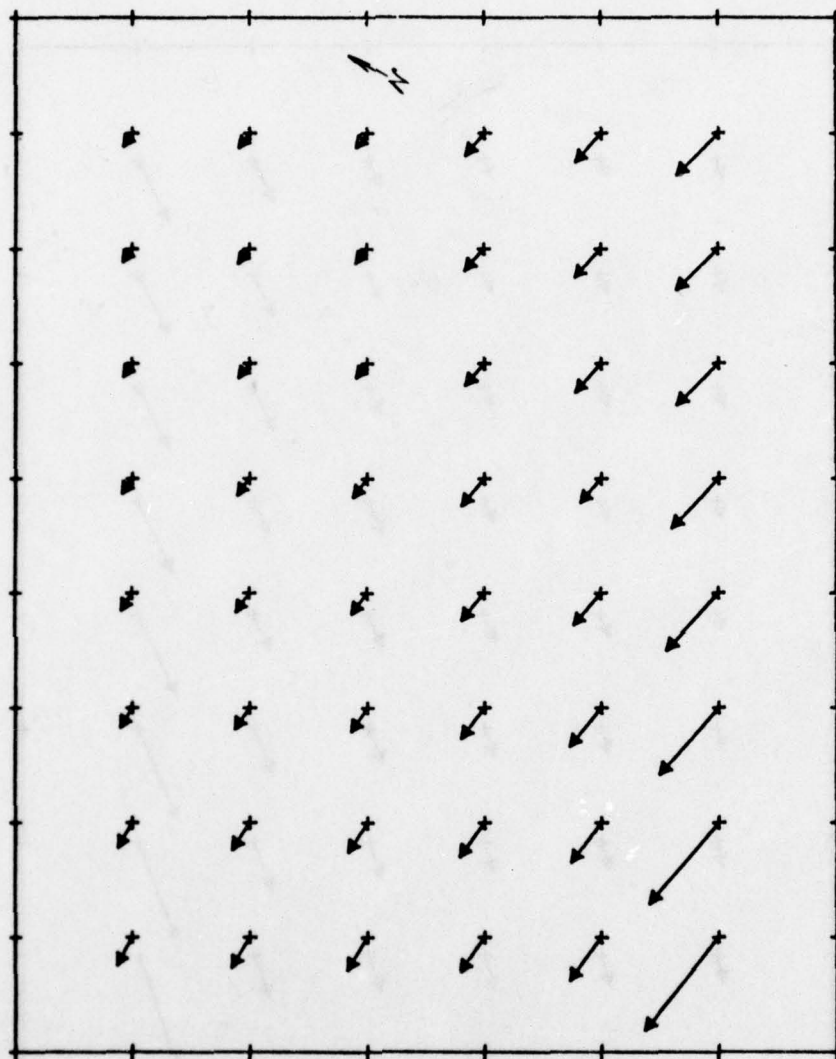


Figure 29. Wind-stress vectors for Hurricane Carla, over Sabine-Calcasieu region on an 8-nautical mile grid; time = 36 hours.

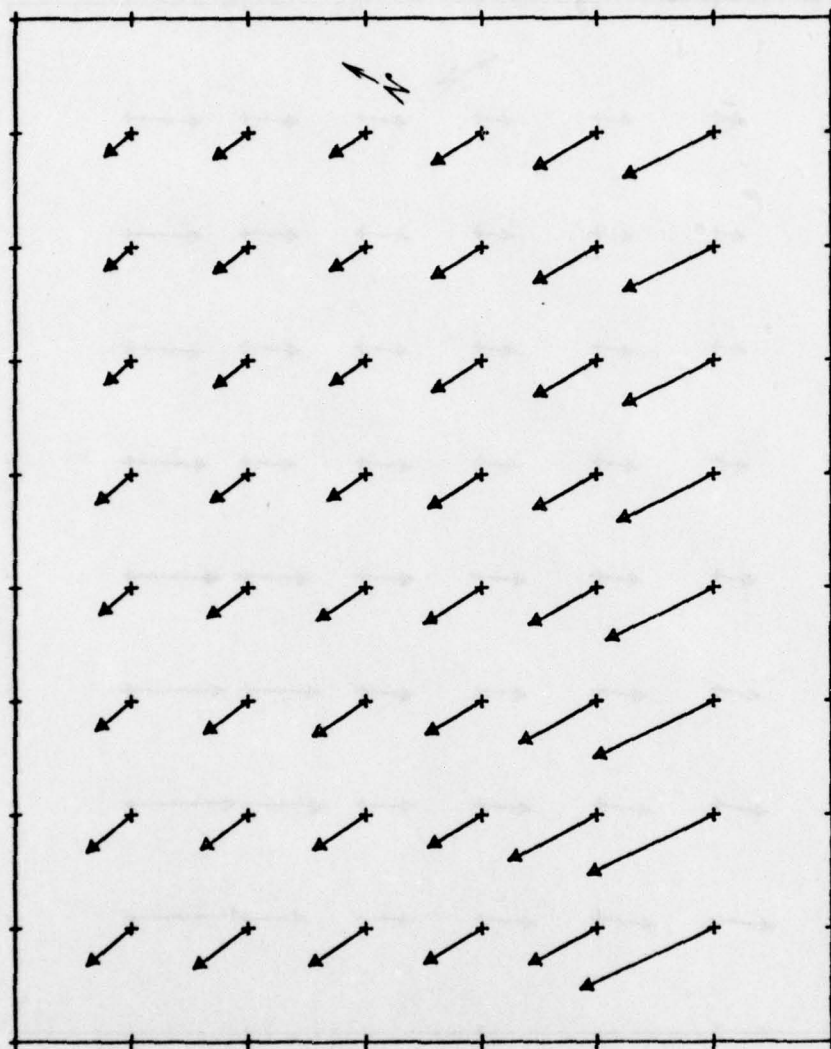


Figure 30. Wind-stress vectors for Hurricane Carla, over Sabine-Calcasieu region on an 8-nautical mile grid; time = 48 hours.

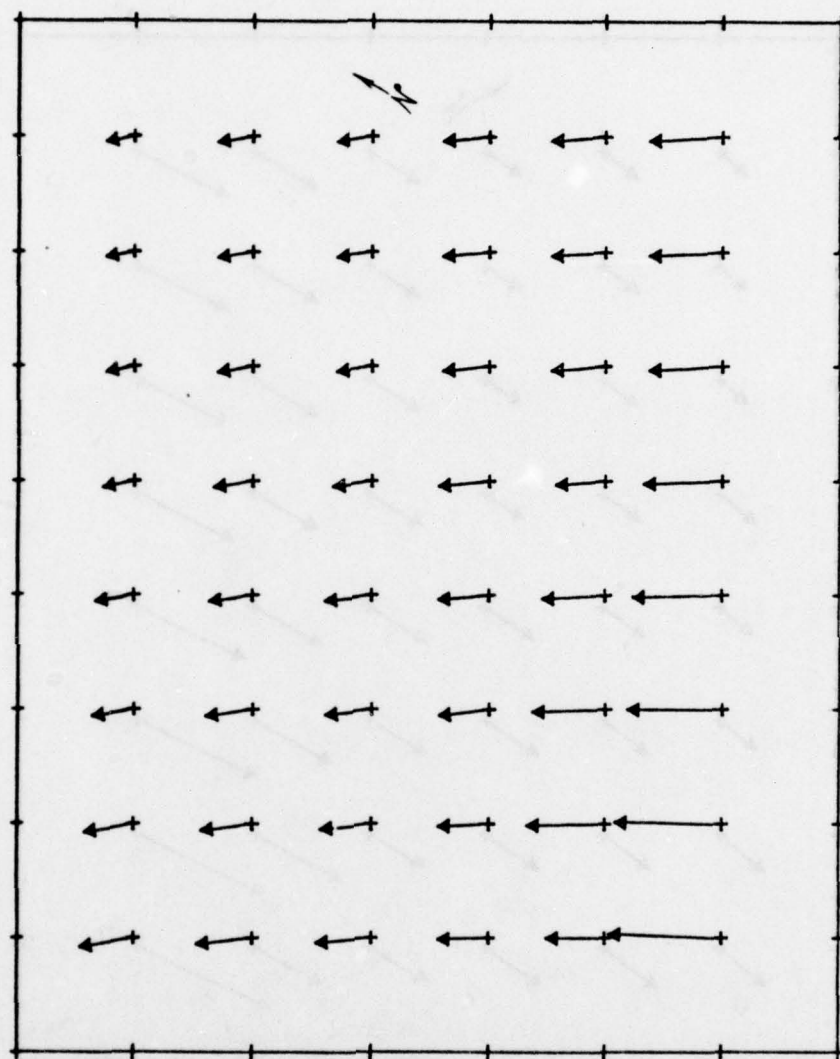


Figure 31. Wind-stress vectors for Hurricane Carla, over Sabine-Calcasieu region on an 8-nautical mile grid; time \approx 54 hours.

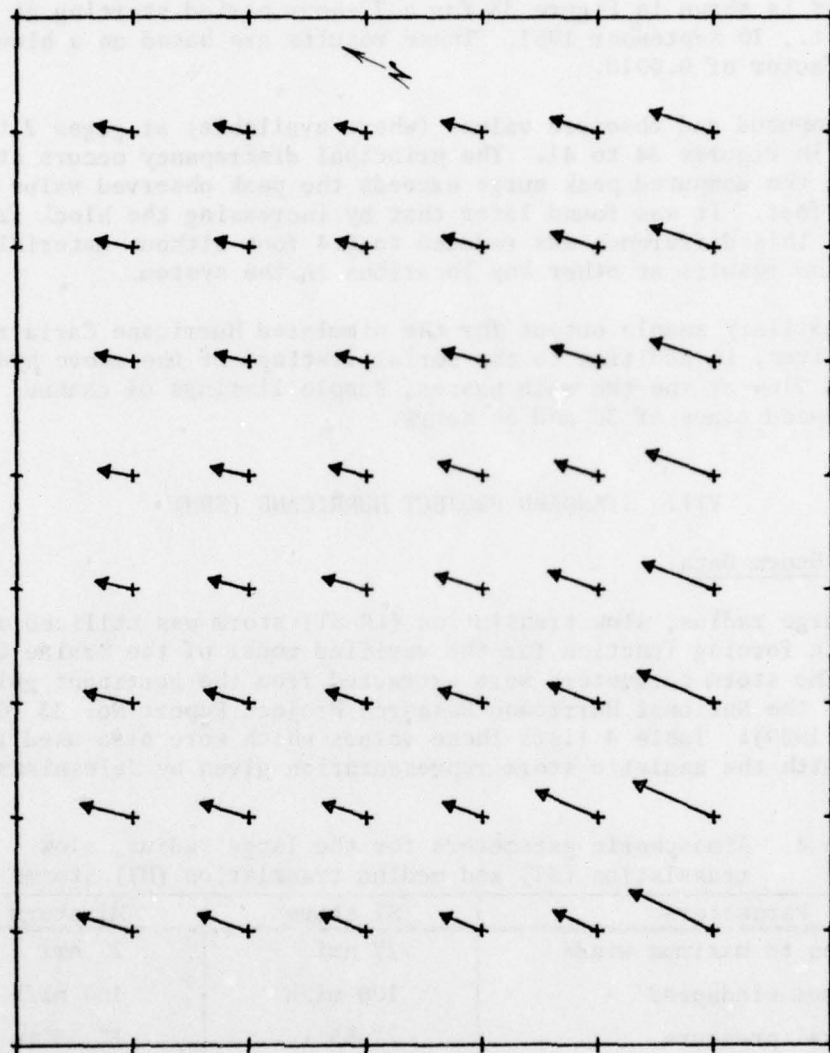


Figure 32. Wind-stress vectors for Hurricane Carla, over Sabine-Calcasieu region on an 8-nautical mile grid; time = 60 hours.

b. Results. The results of the Hurricane Carla simulation are given in Figures 33 to 41, and Appendix G. The input (observed) hydrograph for Sabine Pass is shown in Figure 33 for a 72-hour period starting at 0000 hours c.s.t., 10 September 1961. These results are based on a block friction factor of 0.0010.

The computed and observed values (where available) at gages 2 to 9 are shown in Figures 34 to 41. The principal discrepancy occurs at Beaumont where the computed peak surge exceeds the peak observed value by about 0.8 foot. It was found later that by increasing the block friction to 0.0025, this difference was reduced to 0.4 foot without materially changing the results at other key locations in the system.

The auxiliary sample output for the simulated Hurricane Carla run (App. G) gives, in addition to the serial listings of the above hydrographs and flow at the two main passes, sample listings of channel output at elapsed times of 30 and 60 hours.

VII. STANDARD PROJECT HURRICANE (SPH)

1. LR-ST Storm Data.

The large radius, slow translation (LR-ST) storm was utilized as an atmospheric forcing function for the verified model of the Sabine-Calcasieu system. The storm parameters were extracted from the pertinent gulf coast section of the National Hurricane Research Project Report No. 33 (Graham and Nunn, 1959). Table 4 lists these values which were also used in conjunction with the analytic storm representation given by Jelesnianski (1965).

Table 4. Atmospheric parameters for the large radius, slow translation (ST) and medium translation (MT) storms.

Parameters	ST storm	MT storm
Radius to maximum winds	27 nmi	27 nmi
Maximum windspeed	100 mi/h	100 mi/h
Central pressure	27.55 in	27.55 in
Translation speed	4 kn	11 kn

Wind-stress vector plots have been prepared beginning at $t = 30$ hours and at 10-hour increments to $t = 80$ hours (Figs. 42 to 47). The storm track, which is taken normal to the general shoreline, has the Sabine-Calcasieu system on the right-hand side of the storm approaching the coastline. Landfall of the storm center is close to grid block 1,1. The orientation of these plots relative to the topography is similar to the wind fields shown for the Hurricane Carla verification. The gulf hydrographic input, provided by the Galveston District, was developed by an application of a one-dimensional bathystrophic model (Marinos and Woodward, 1968; Bodine, 1971). A tidal component has been added to this

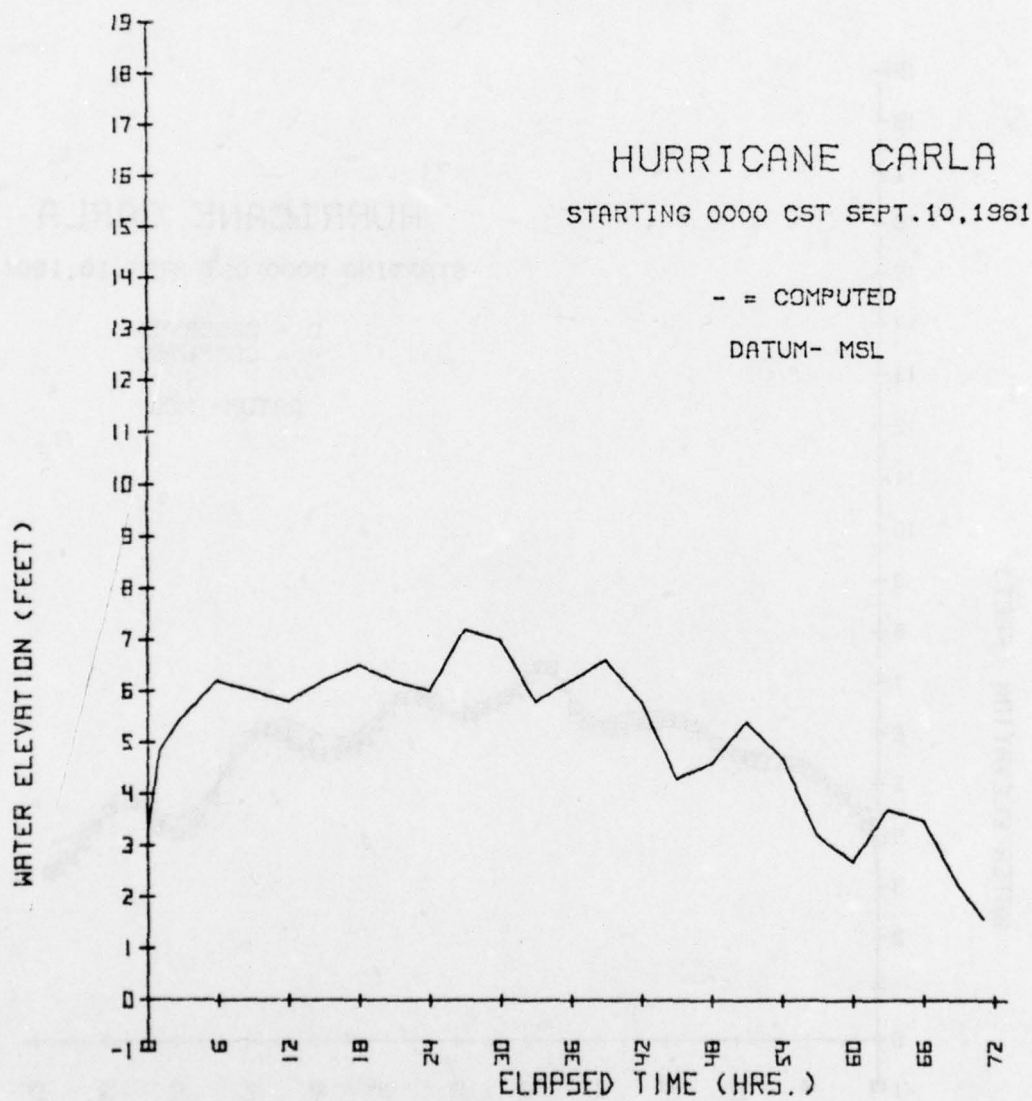


Figure 33. Hydrograph at Sabine Pass, southwest jetty for Hurricane Carla.

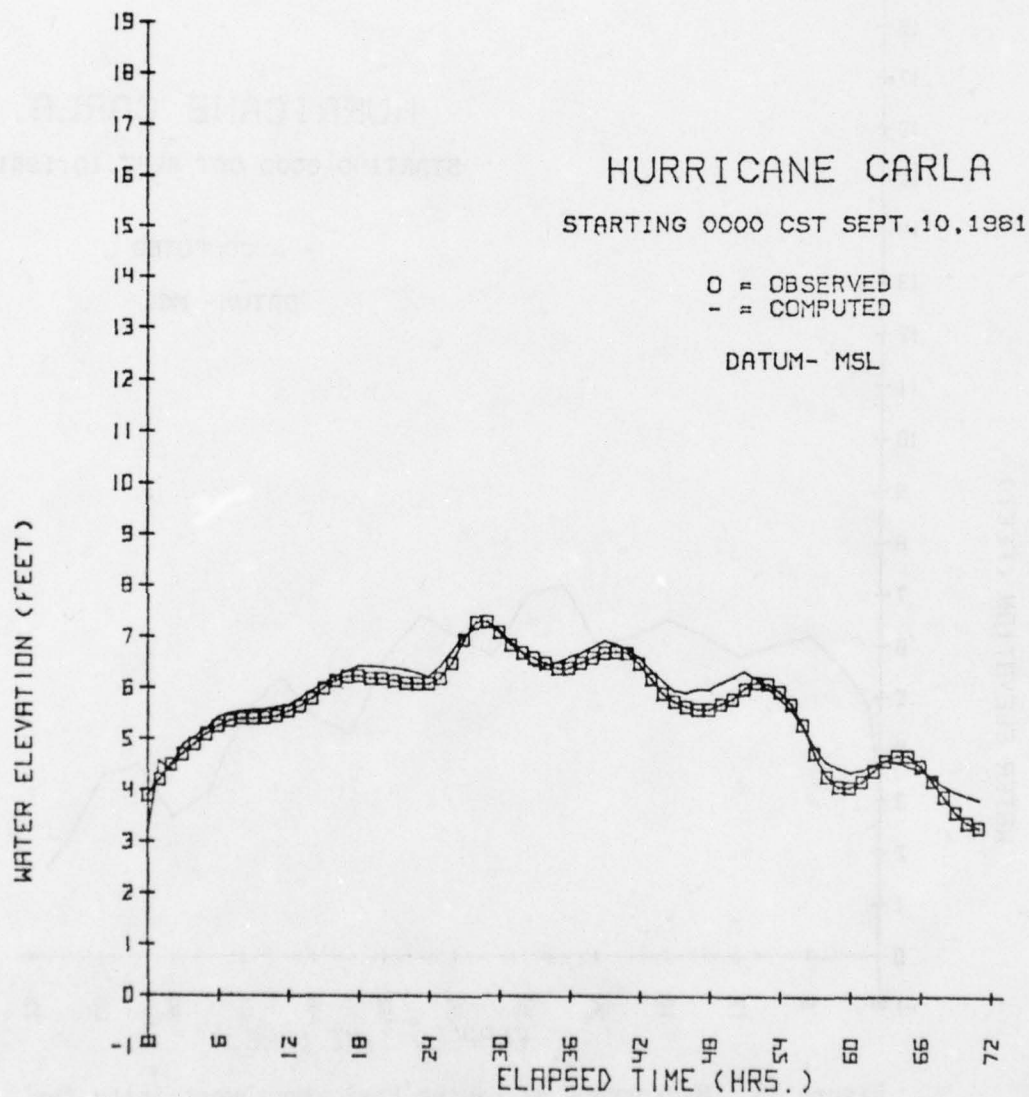


Figure 34. Hydrographs at Sabine Pass, U.S. Coast Guard Station for Hurricane Carla (FK = 0.0010).

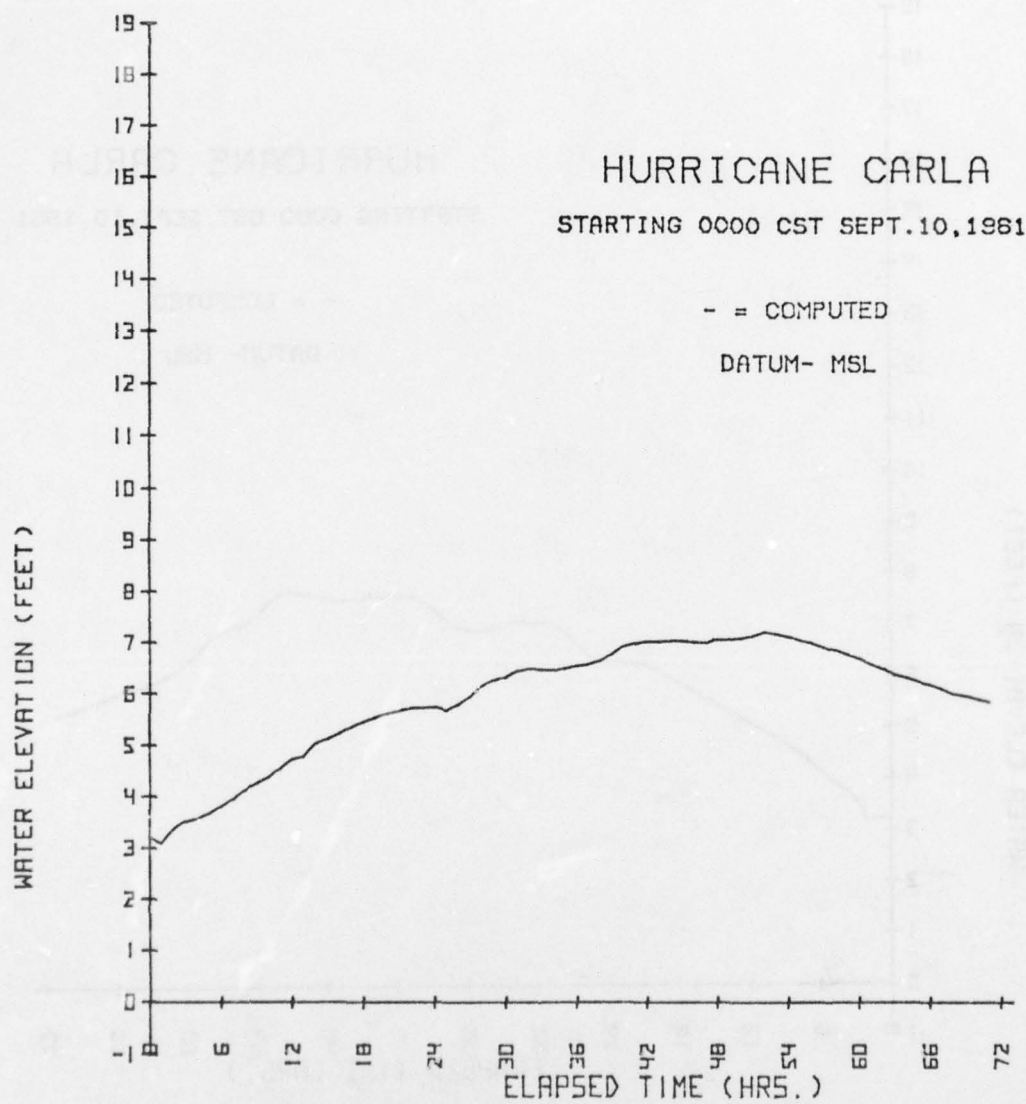


Figure 35. Hydrograph at Port Arthur for Hurricane Carla (FK = 0.0010).

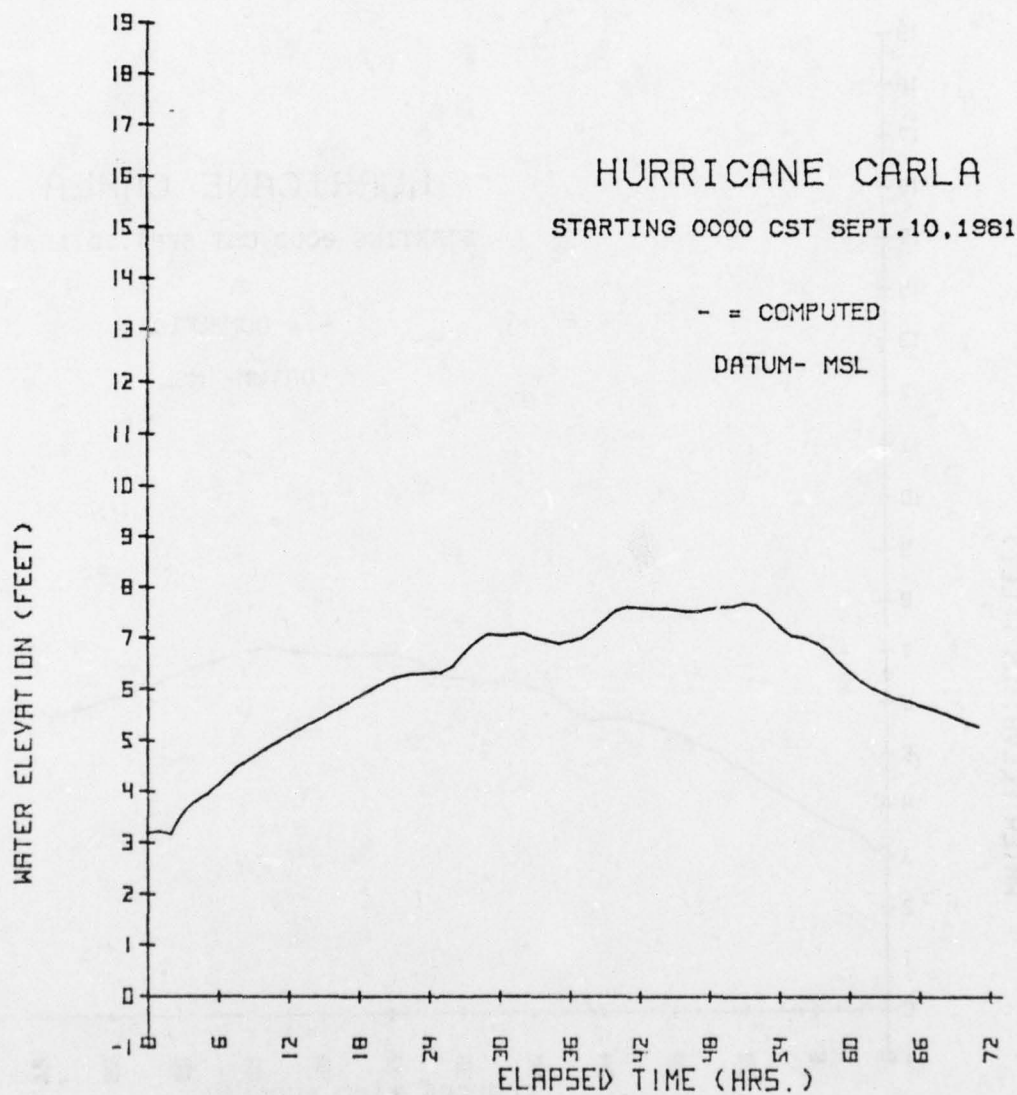


Figure 36. Hydrograph at north Sabine Lake for Hurricane Carla (FK = 0.0010).

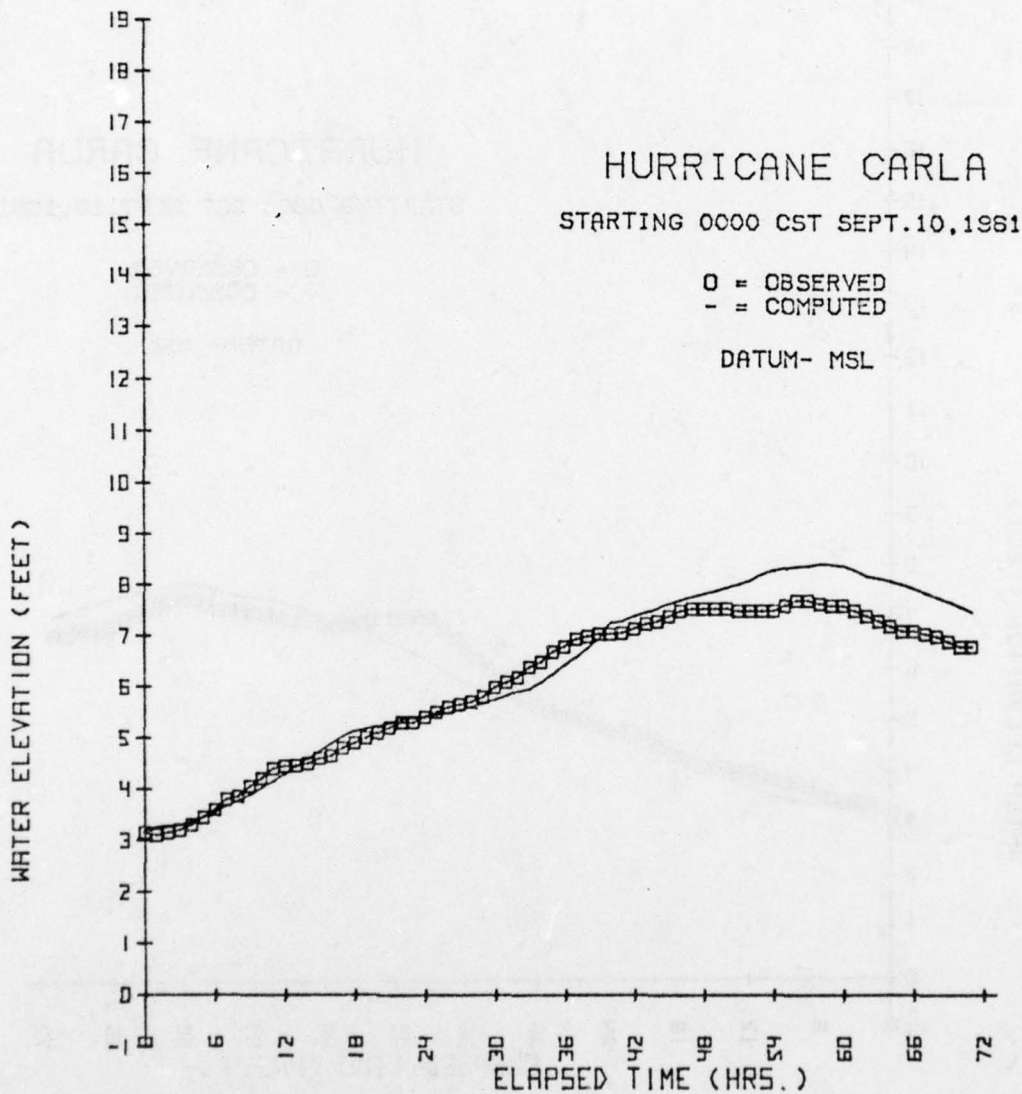


Figure 37. Hydrographs at Beaumont, Neches River, and Brakes Bayou for Hurricane Carla (FK = 0.0010).

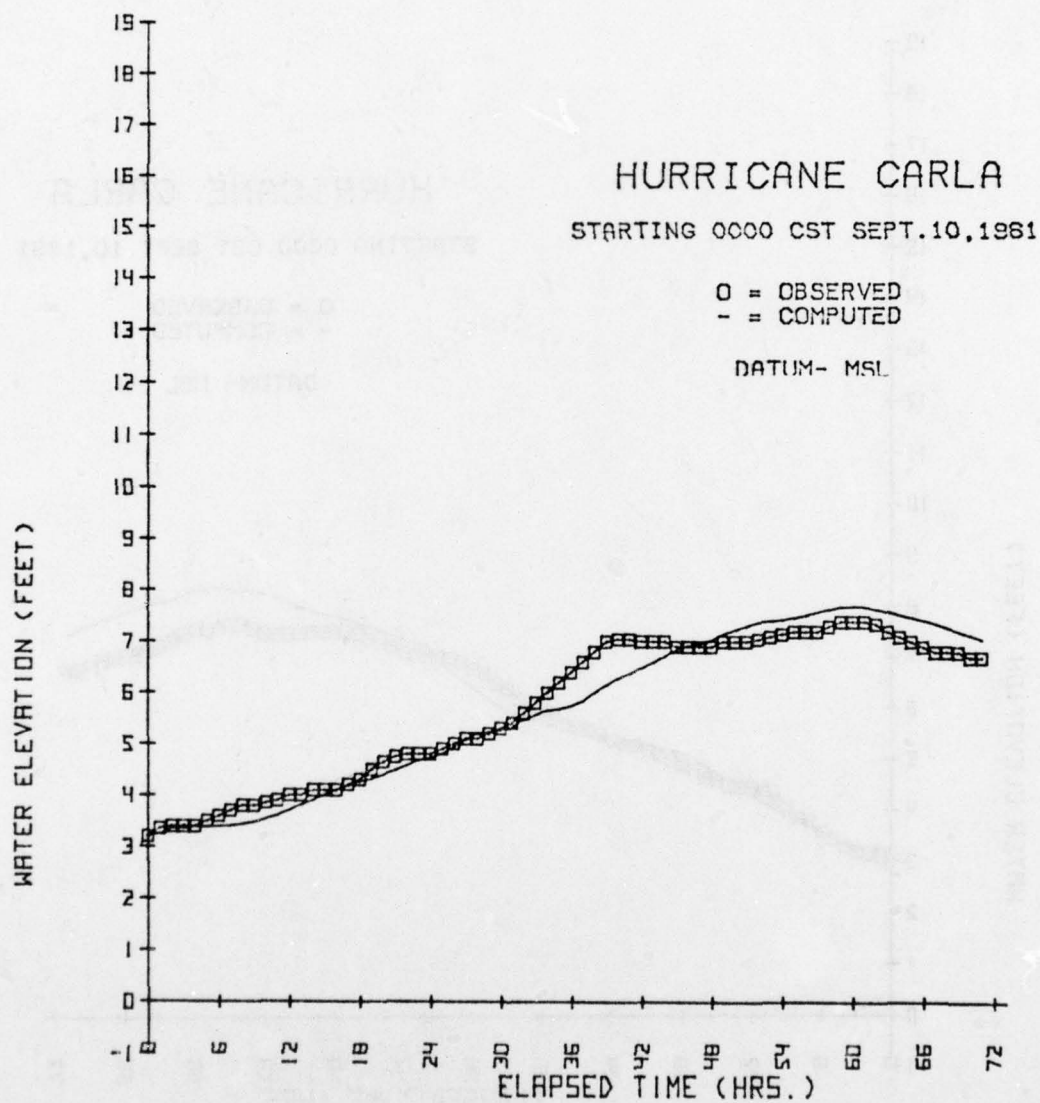


Figure 38. Hydrographs at Orange Naval Station, Sabine River for Hurricane Carla (FK = 0.0010).

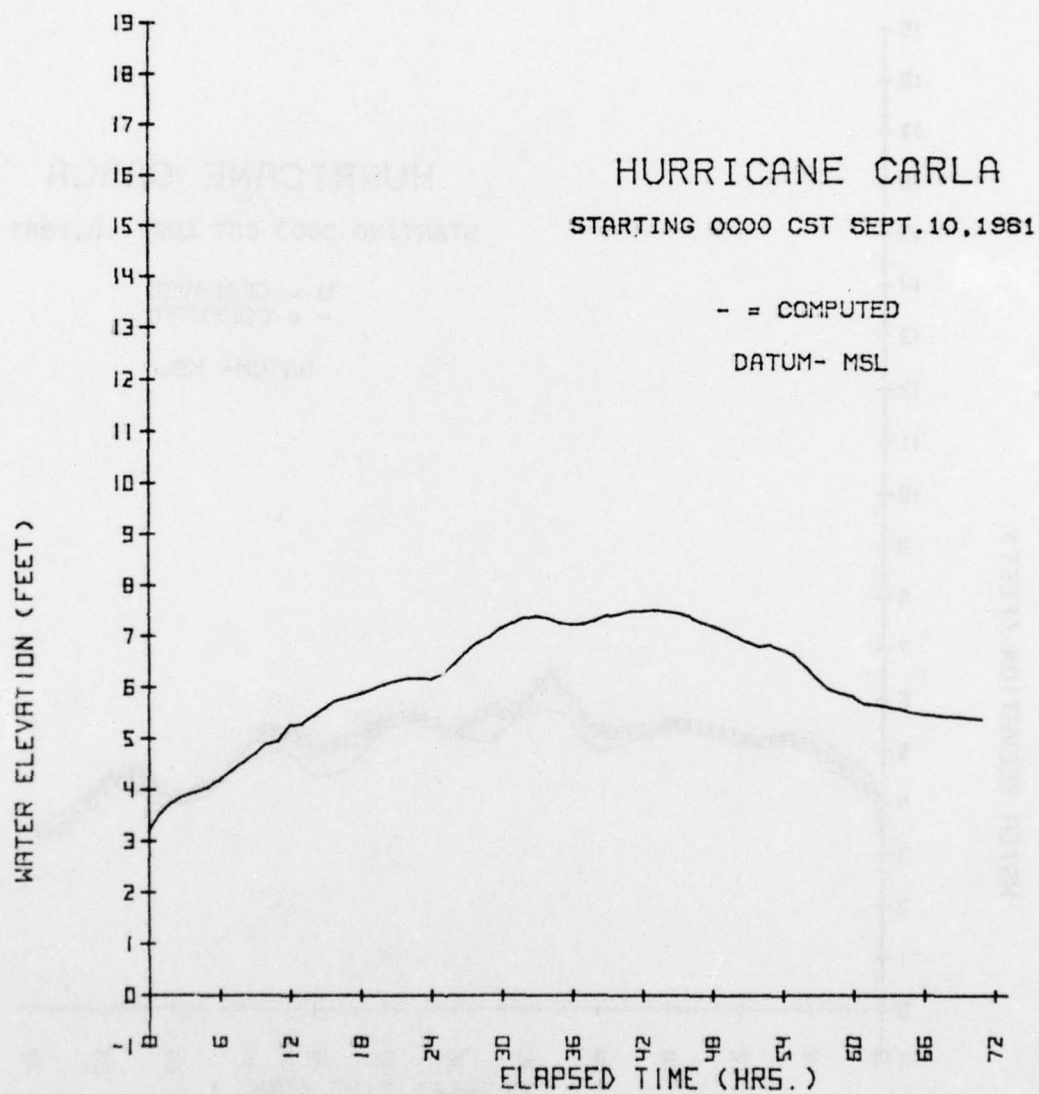


Figure 39. Hydrograph at west end of Intracoastal Waterway for Hurricane Carla (FK = 0.0010).

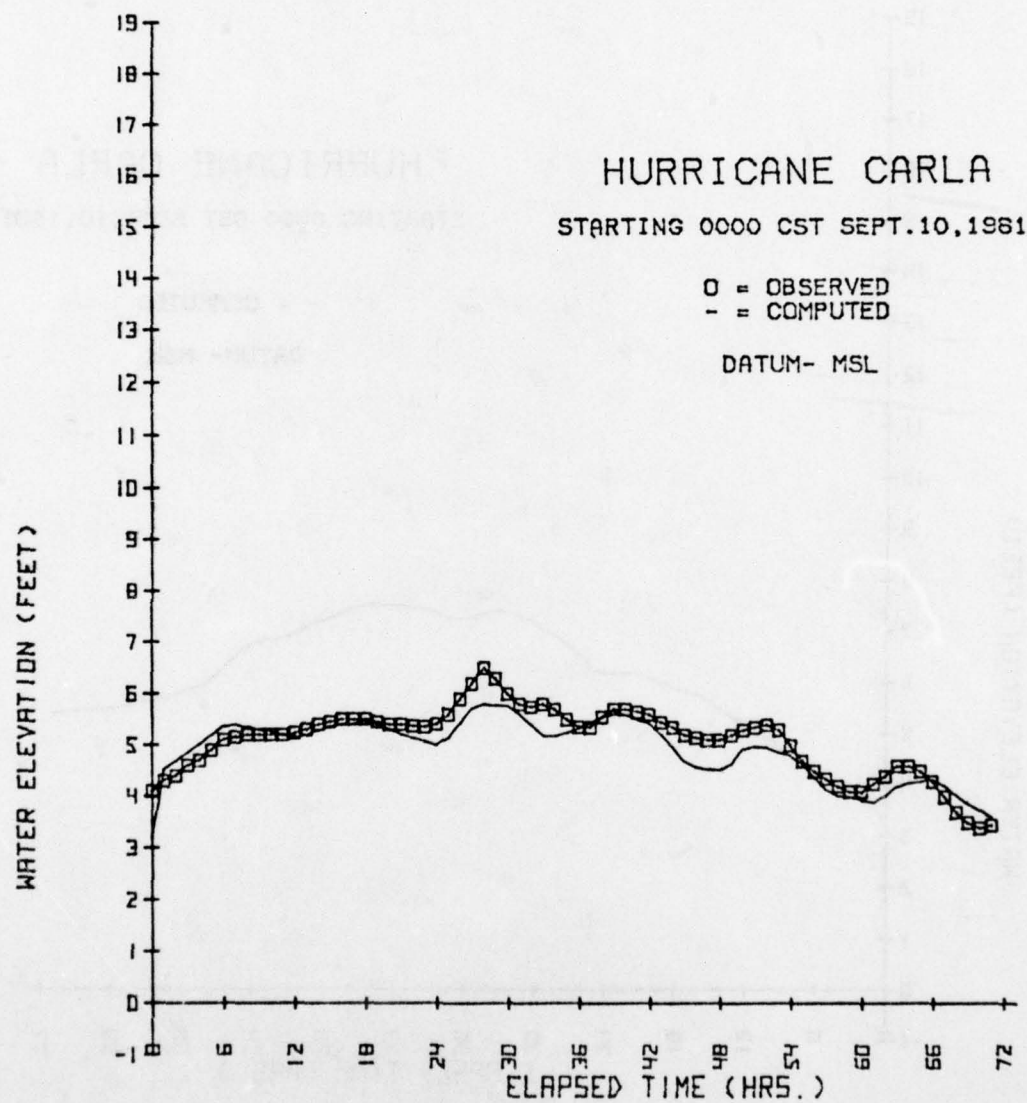


Figure 40. Hydrographs at Cameron, Calcasieu Pass for Hurricane Carla (FK = 0.0010).

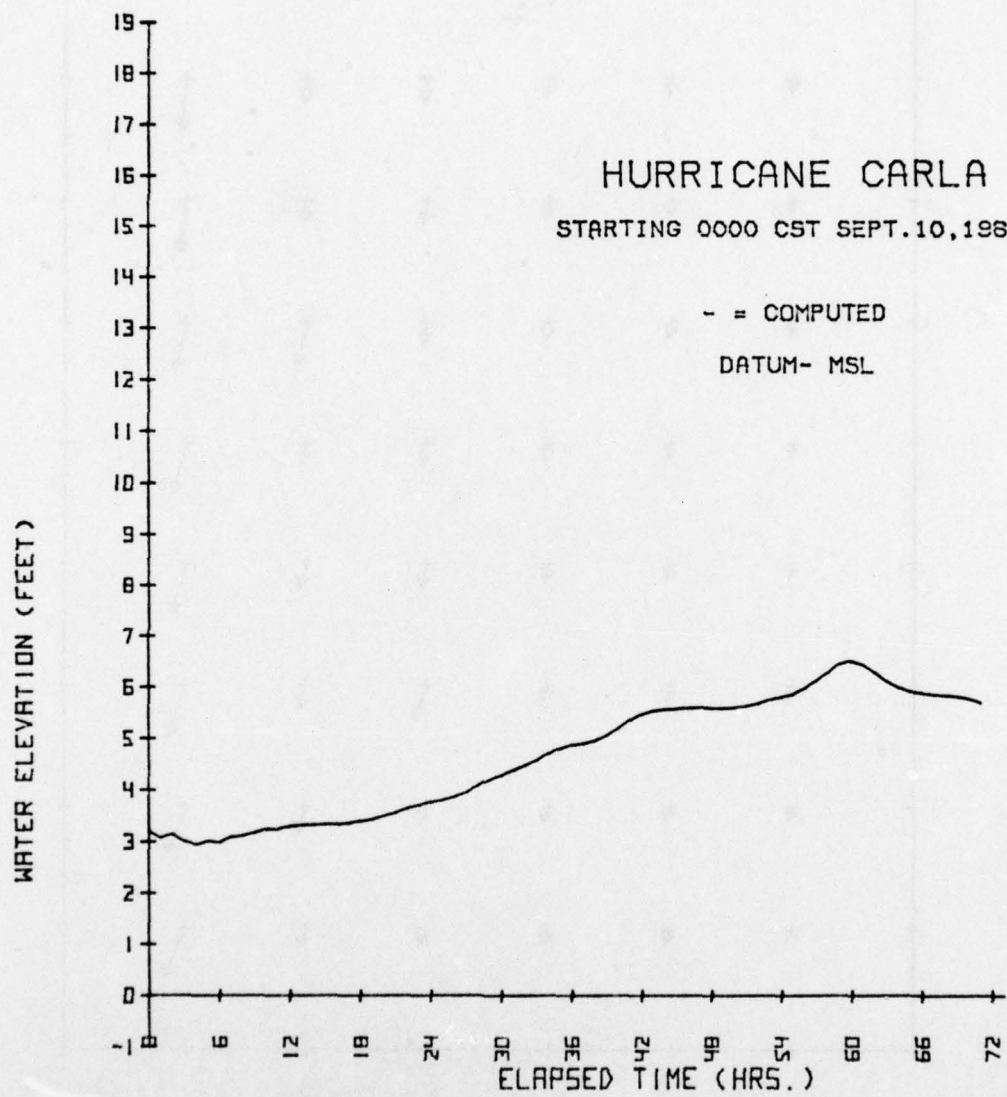


Figure 41. Hydrograph at Lake Charles for Hurricane Carla
(FK = 0.0010).

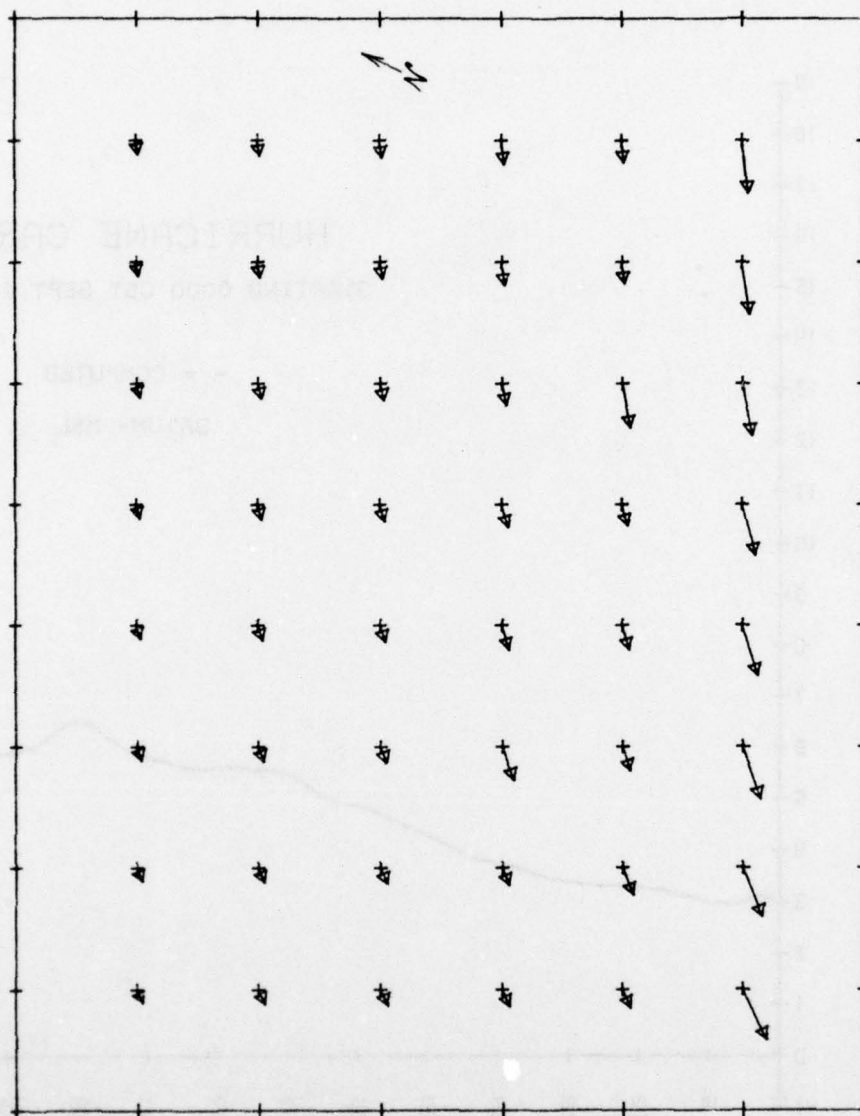


Figure 42. Wind-stress vectors for SPH large radius, slow translation (LR-ST) on an 8-nautical mile grid; time = 30 hours.

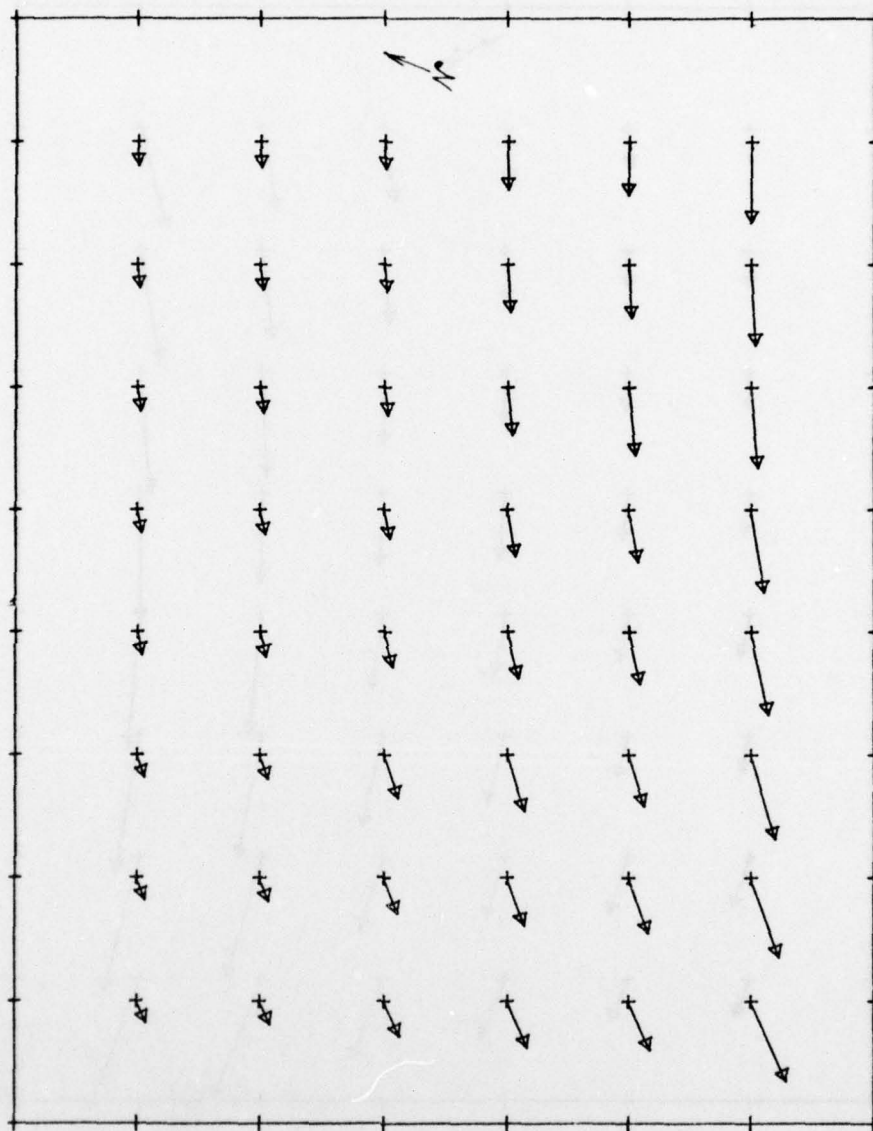


Figure 43. Wind-stress vectors for SPH large radius, slow translation (LR-ST) on an 8-nautical mile grid; time = 40 hours.

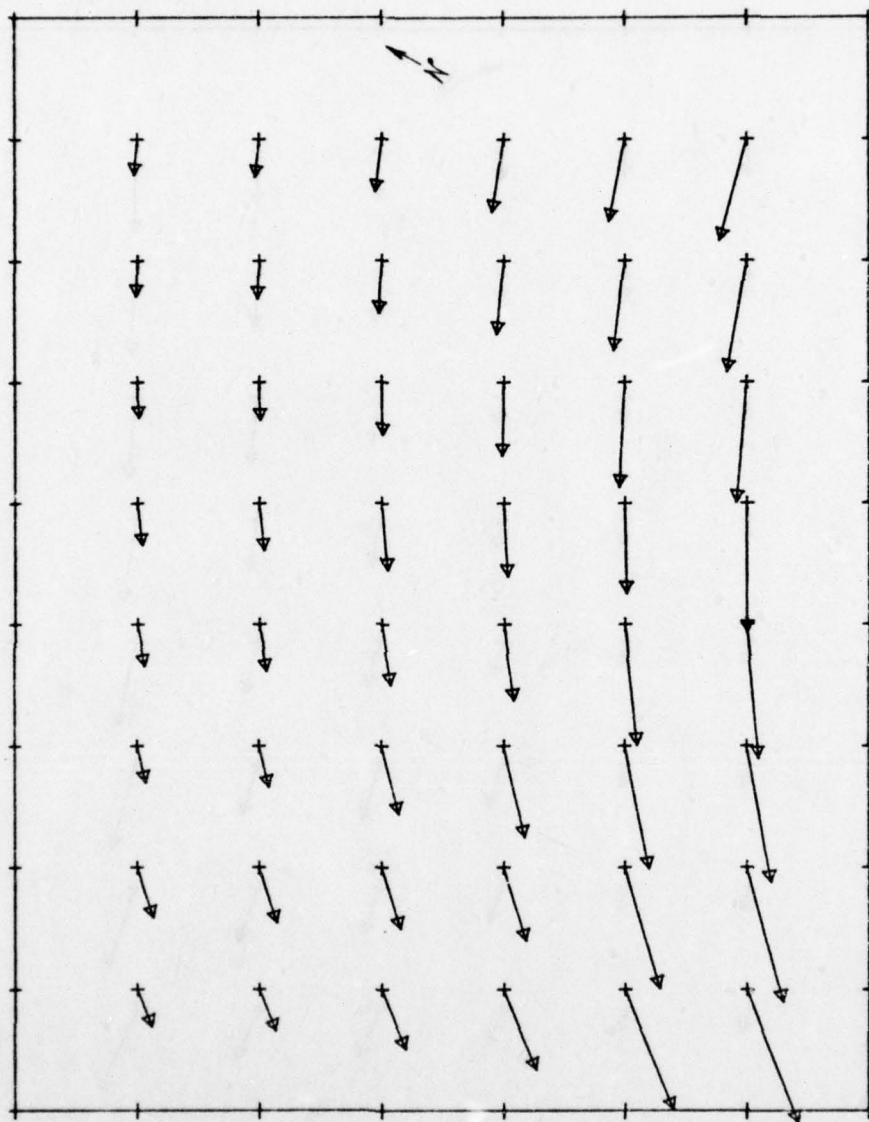


Figure 44. Wind-stress vectors for SPH large radius, slow translation (LR-ST) on an 8-nautical mile grid; time = 50 hours.

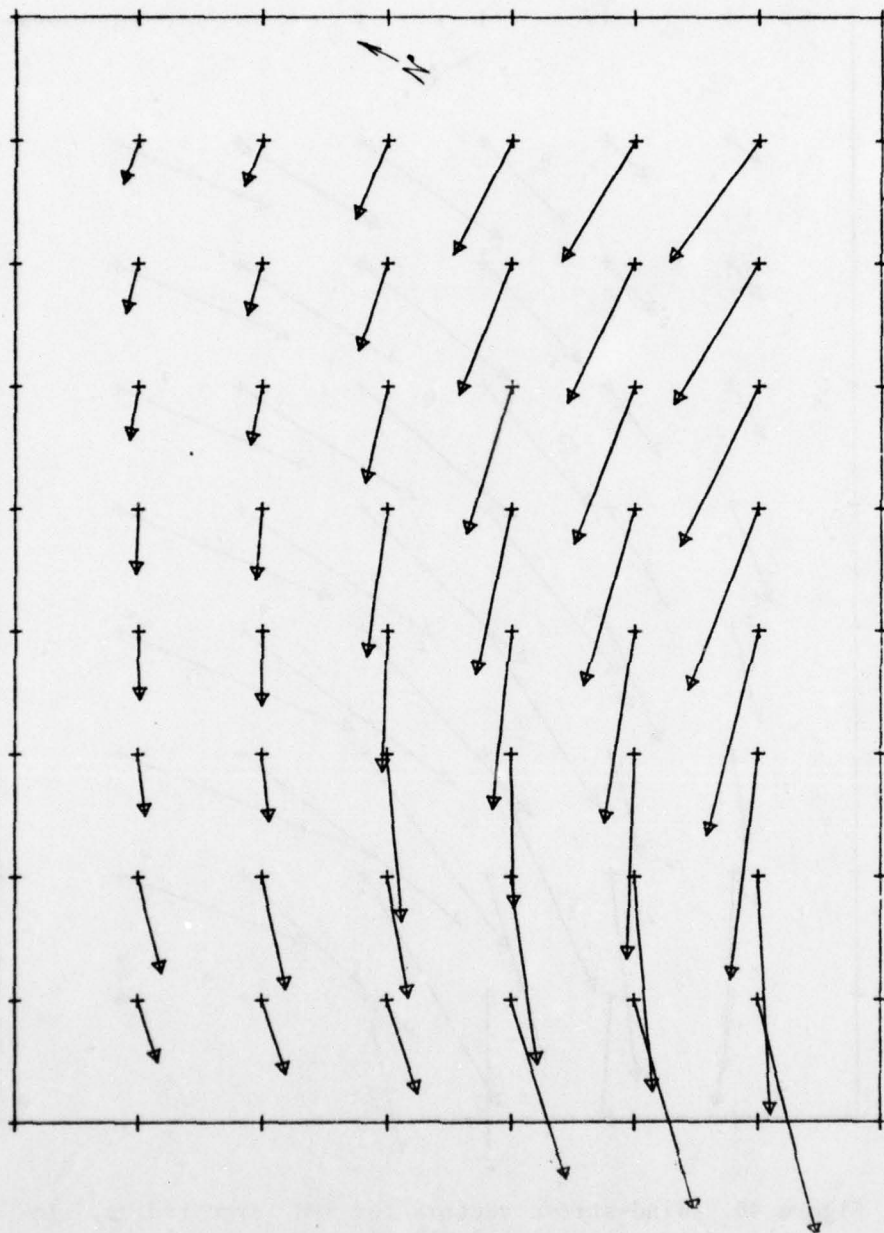


Figure 45. Wind-stress vectors for SPH large radius, slow translation (LR-ST) on an 8-nautical mile grid; time = 60 hours.

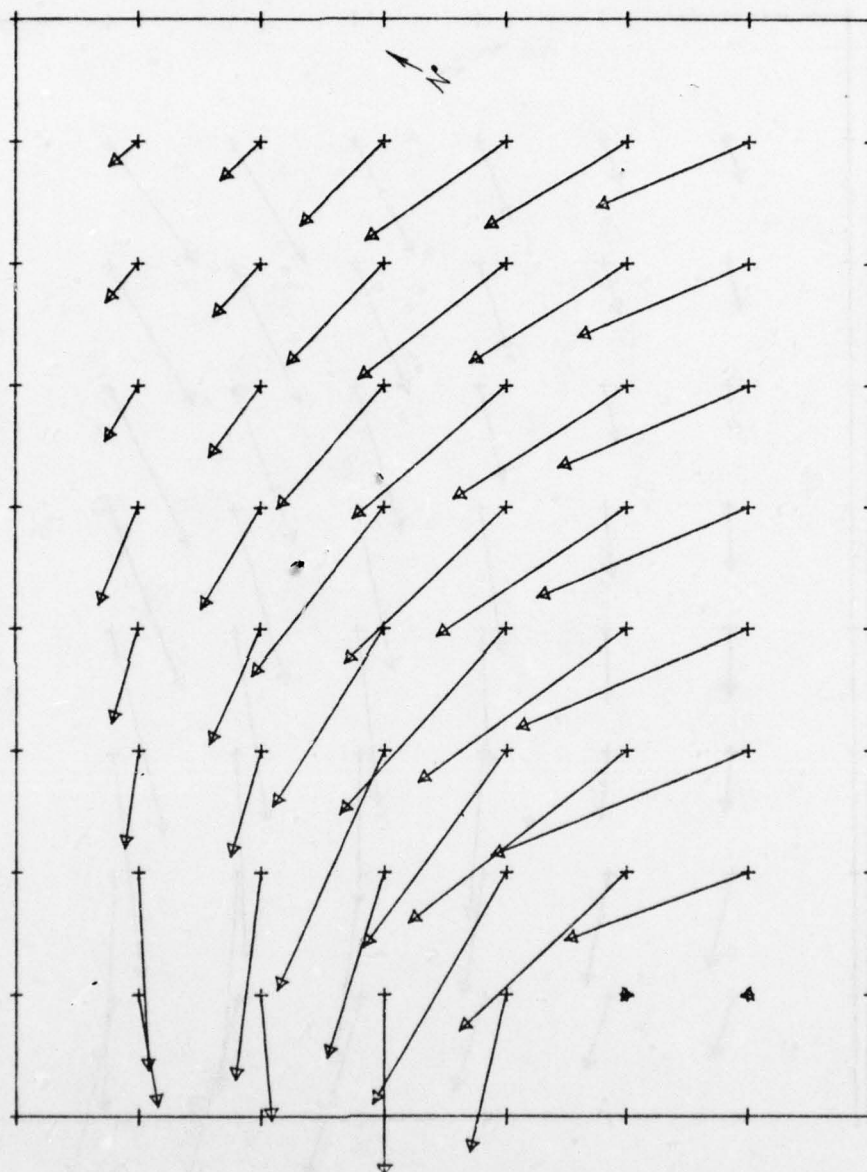


Figure 46. Wind-stress vectors for SPH large radius, slow translation (LR-ST) on an 8-nautical mile grid; time = 70 hours.

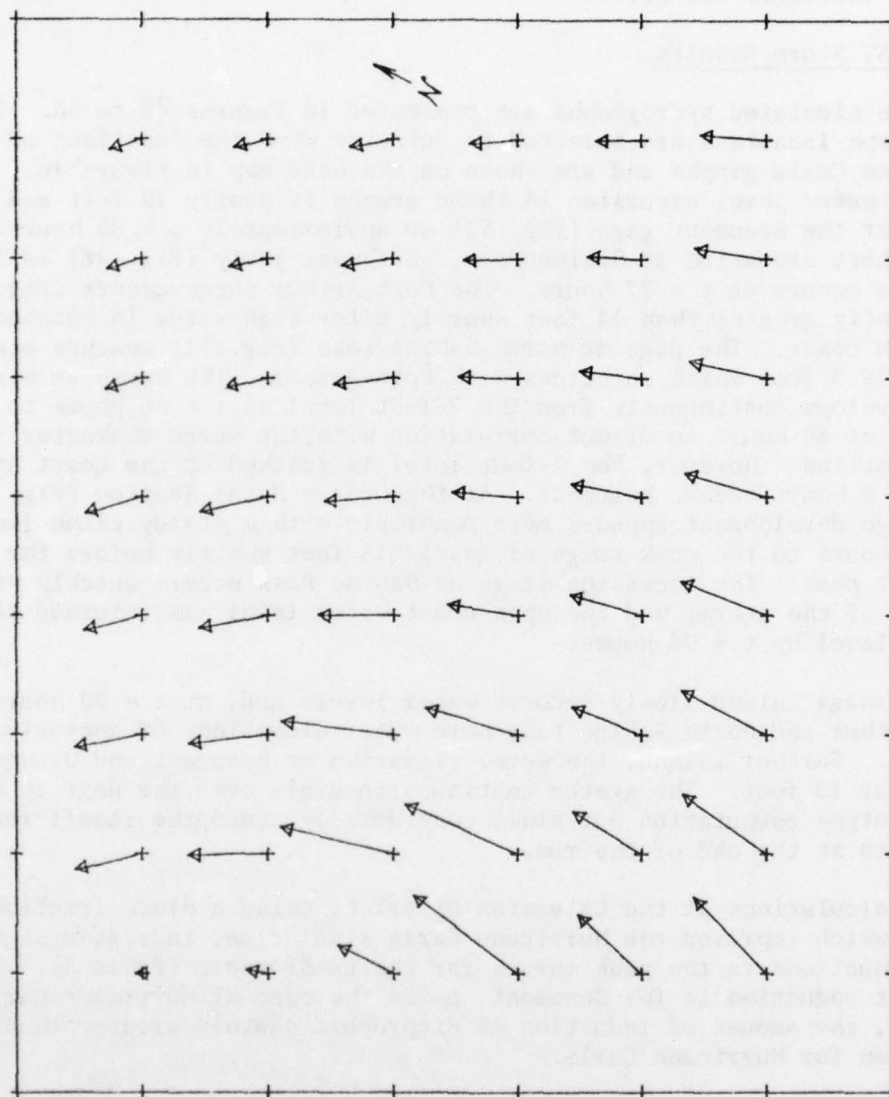


Figure 47. Wind-stress vectors for SPH large radius, slow translation (LR-ST) on an 8-nautical mile grid; time = 80 hours.

open-coast surge. A total rainfall of 16 inches in 24 hours is included as input. The results are based on a block friction factor of 0.0010 and are therefore tentative.

2. LR-ST Storm Results.

Nine simulated hydrographs are presented in Figures 48 to 56. The hydrograph locations are selected to coincide with the locations of the Hurricane Carla graphs and are shown on the base map in Figure 16. The maximum water level excursion in these graphs is nearly 19 feet and occurs at the Beaumont gage (Fig. 52) at approximately $t = 80$ hours. The highest elevation at Sabine Pass, southwest jetty (Fig. 48) is 13 feet and occurs at $t = 77$ hours. The Port Arthur surge crests (Fig. 52) at slightly greater than 14 feet shortly after high water is reached on the open coast. The gage at north Sabine Lake (Fig. 51) reaches a maximum of 15.3 feet which coincides with Port Arthur. The surge at Beaumont develops continuously from the 7-foot level at $t = 66$ hours to a maximum at 80 hours in direct correlation with the surge character at the coastline. However, the 7-foot level is reached at the coast approximately 6 hours before Beaumont. At the Orange Naval Station (Fig. 53) the surge development appears more monotonic with a steady climb from $t = 54$ hours to the peak surge of nearly 16 feet shortly before the Beaumont peak. The recession stage at Sabine Pass occurs quickly with passage of the storm, and the open-coast water level has returned to normal level by $t = 90$ hours.

Drainage inland slowly reduces water levels and, at $t = 90$ hours, Port Arthur and north Sabine Lake have water elevations of approximately 10 feet. Farther inland, the water elevation at Beaumont and Orange stands at 13 feet. The system continues to drain over the next 10 hours of prototype computation but slows considerably since the runoff reaches peak rate at the end of the run.

Recalculations at the Galveston District, using a block friction of 0.0025 which improved the Hurricane Carla simulation, indicated significant reductions in the peak surges for the LR-ST storm (Table 5). The greatest reduction is for Beaumont, as in the case of Hurricane Carla; however, the amount of reduction is disproportionately greater than that seen for Hurricane Carla.

Table 5. Comparison of peak surges for the LR-ST storm, using two different block friction factors.

Location	Surges (ft above MSL)	
	FK = 0.0010	FK = 0.0025
Port Arthur	14.3	14.1
Beaumont	18.7	17.1
Orange Naval Station	15.9	15.2
Lake Charles	14.1	13.9

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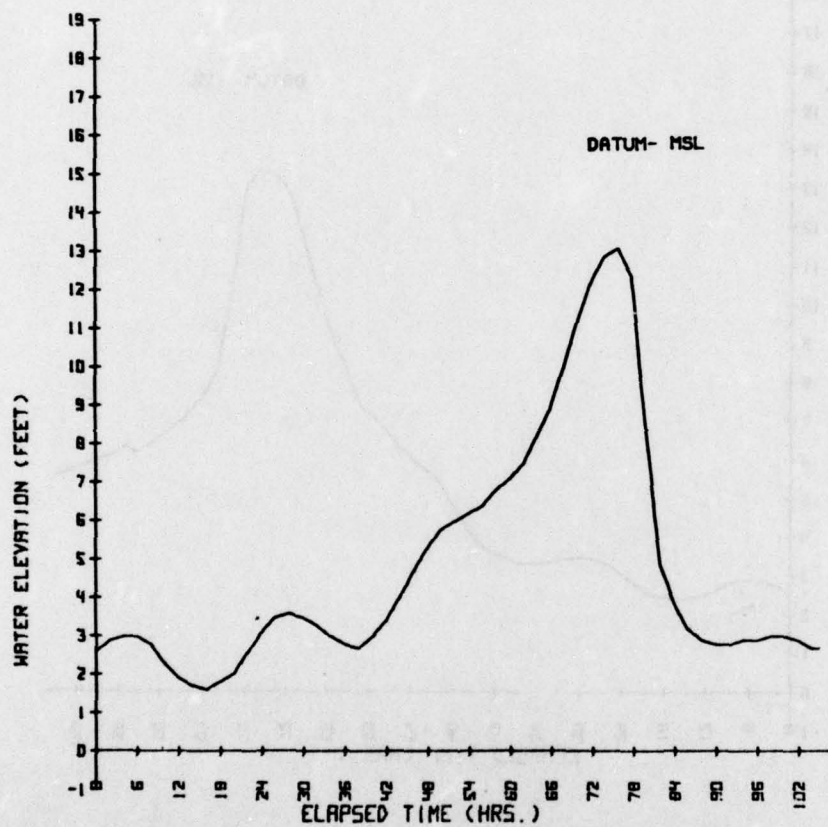


Figure 48. Hydrograph for SPH, LR-ST at Sabine Pass, southwest jetty (FK = 0.0010).

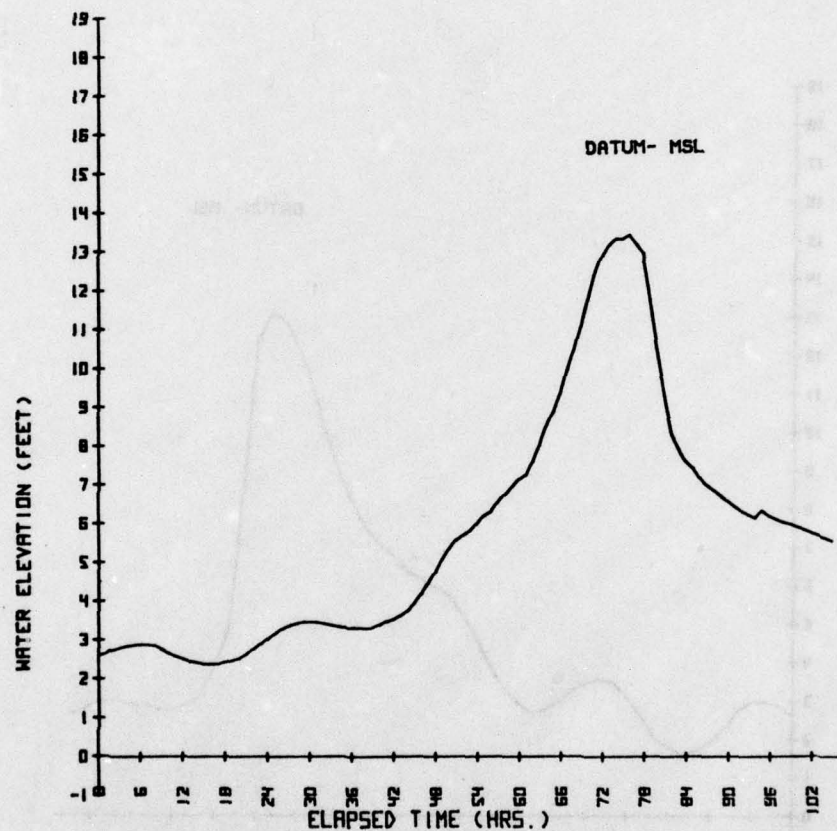


Figure 49. Hydrograph for SPH, LR-ST at Sabine Pass, U.S. Coast Guard Station (FK = 0.0010).

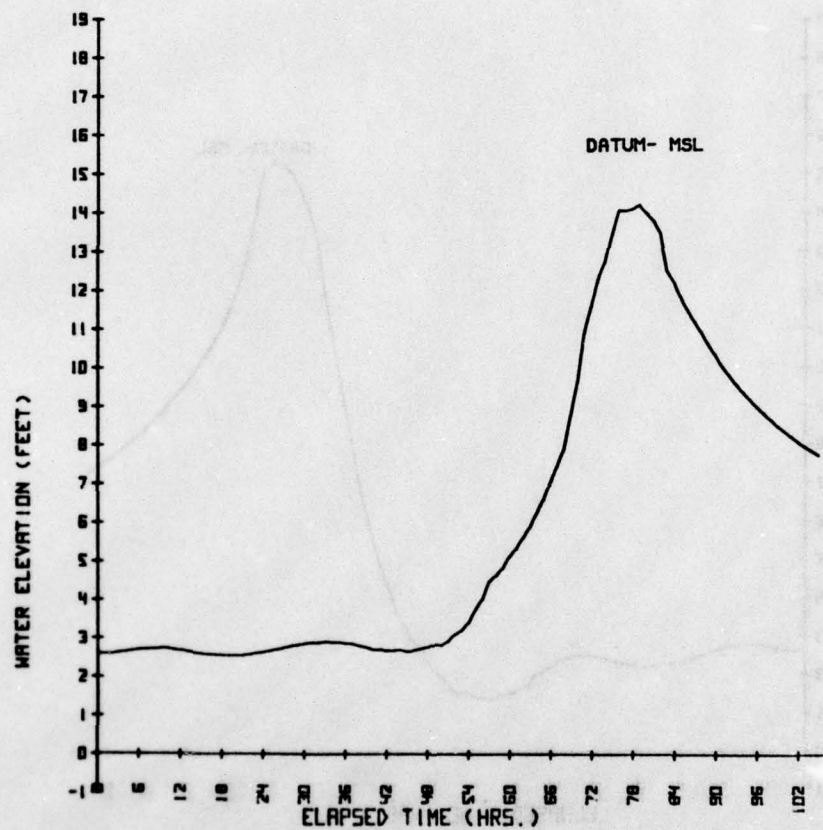


Figure 50. Hydrograph for SPH, LR-ST at Port Arthur (FK = 0.0010).

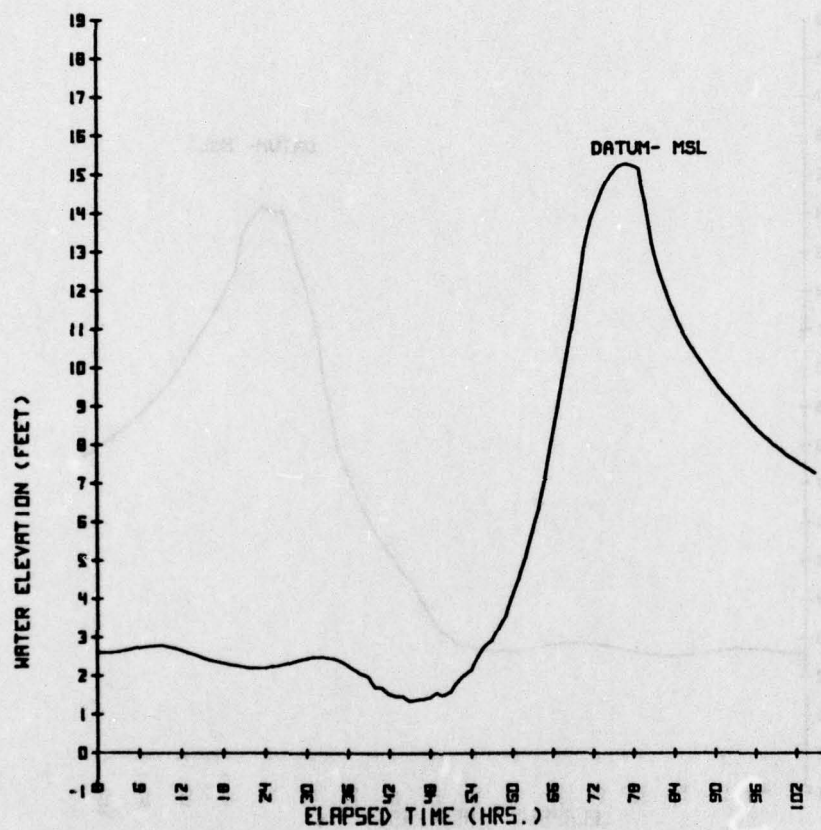


Figure 51. Hydrograph for SPH, LR-ST at north Sabine Lake (FK = 0.0010).

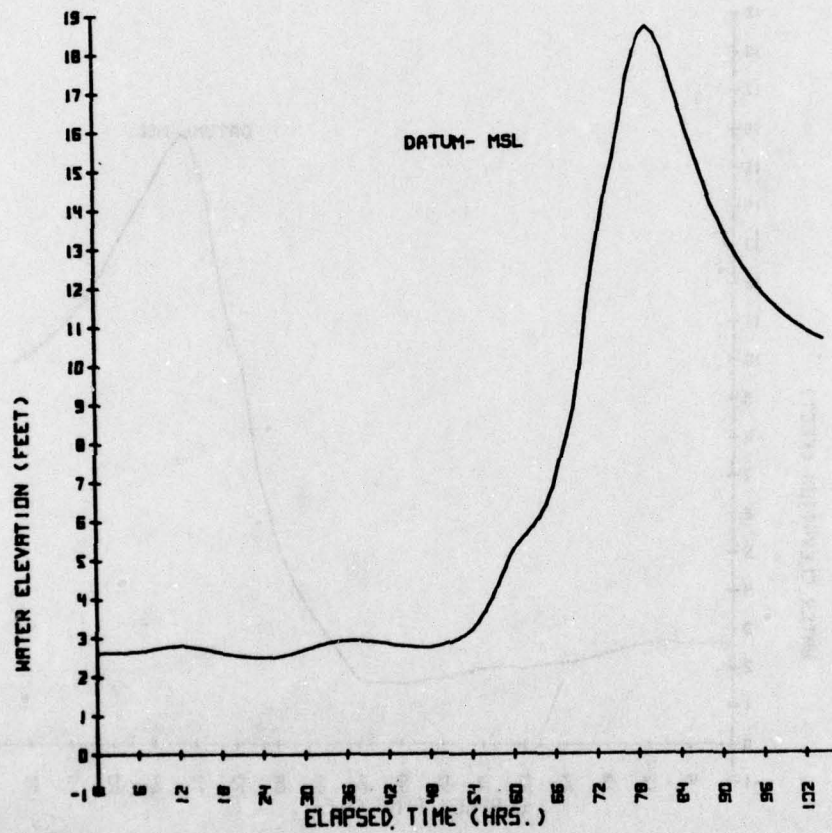


Figure 52. Hydrograph for SPH, LR-ST at Beaumont, Neches River, and Brakes Bayou.

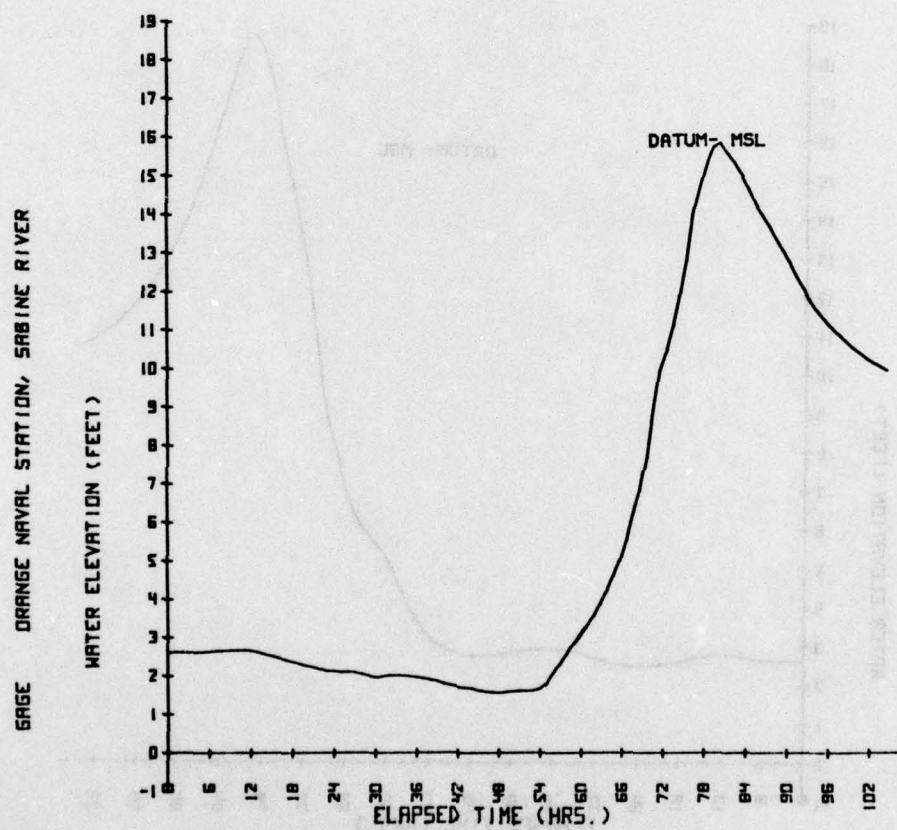


Figure 53. Hydrograph for SPH, LR-ST at Orange Naval Station, Sabine River (FK = 0.0010).

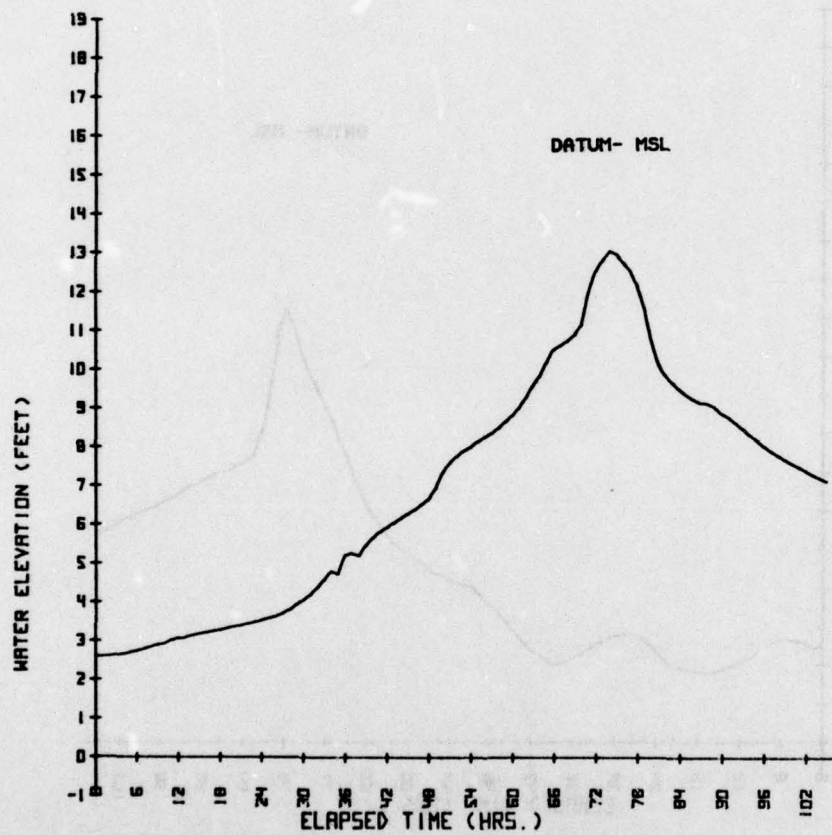


Figure 54. Hydrograph for SPH, LR-ST at west end of Intra-coastal Waterway (FK = 0.0010).

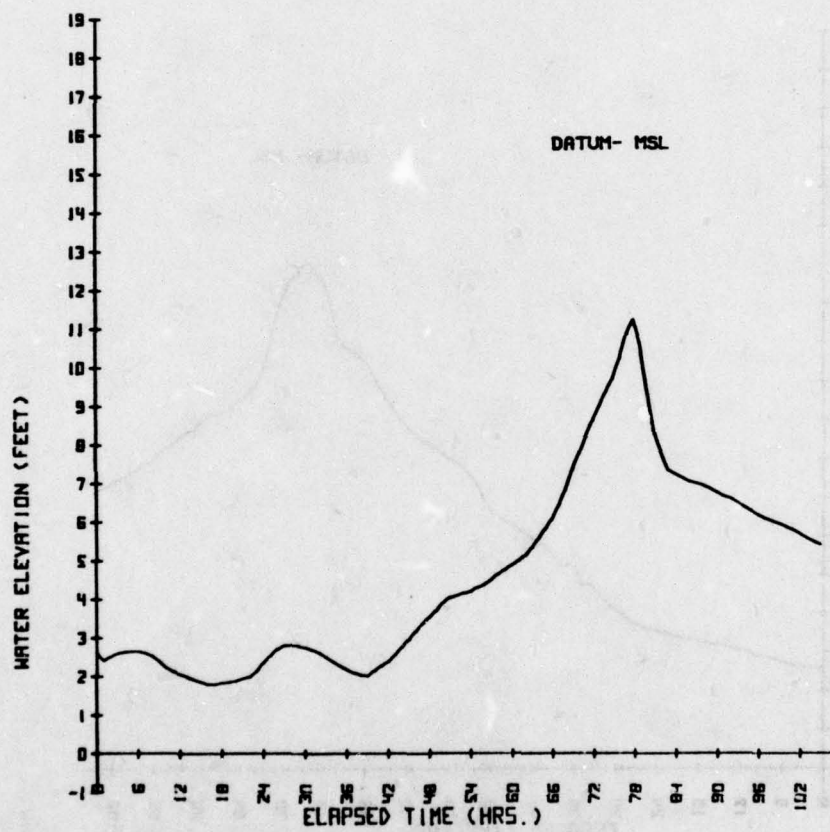


Figure 55. Hydrograph for SPH, LR-ST at Cameron, Calcasieu Pass (FK = 0.0010).

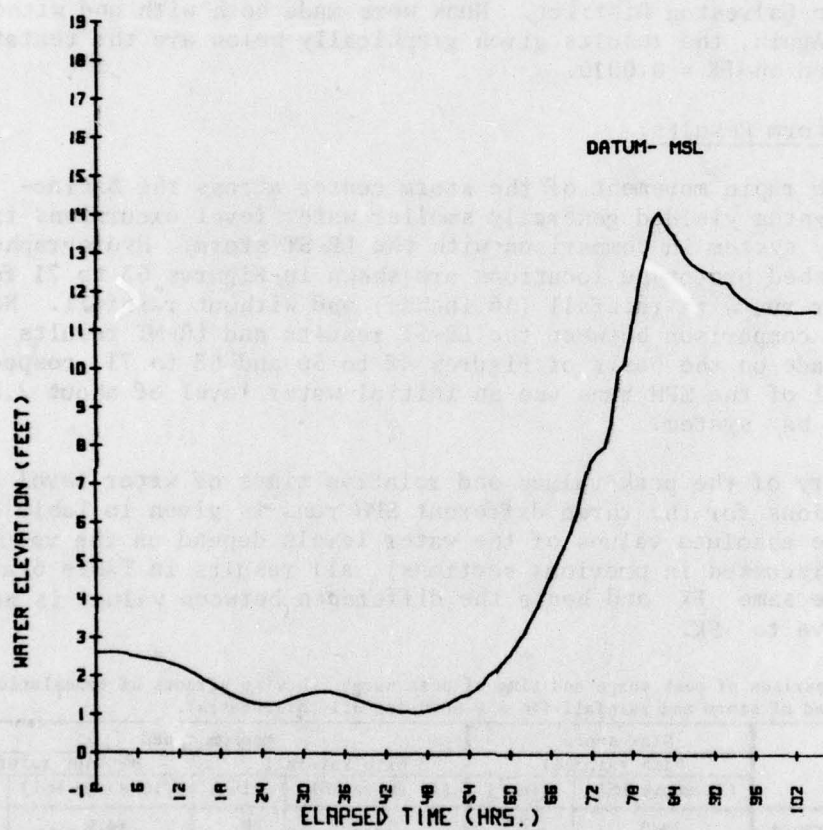


Figure 56. Hydrograph for SPH, LR-ST at Lake Charles, Calcasieu River (FK = 0.0010).

3. LR-MT Storm Data.

The large radius, medium translation (LR-MT) storm has identical characteristics to the LR-ST storm with the exception of a higher translation speed of 11 knots. Wind vector plots from $t = 15$ hours to $t = 40$ hours are shown at 5-hour increments in Figures 57 to 62. The storm track is identical to that of the LR-ST storm. The gulf hydrographic input was derived by one-dimensional, bathystrophic analysis and provided by the Galveston District. Runs were made both with and without rainfall. Again, the results given graphically below are the tentative results based on $FK = 0.0010$.

4. LR-MT Storm Results.

The more rapid movement of the storm center across the Sabine-Calcasieu system yielded generally smaller water level excursions inside the bay system in comparison with the LR-ST storm. Hydrographs at the established prototype locations are shown in Figures 63 to 71 for the computer run with rainfall (16 inches) and without rainfall. Note that direct comparison between the LR-ST results and LR-MT results should be made on the basis of Figures 48 to 56 and 63 to 71, respectively. All of the SPH runs use an initial water level of about 2.5 feet in the bay system.

A summary of the peak values and relative times of water level at seven locations for the three different SPH runs is given in Table 6. Although the absolute values of the water levels depend on the value of FK (as discussed in previous sections), all results in Table 6 are based on the same FK and hence the difference between values is not too sensitive to FK .

Table 6. Comparison of peak surge and time of peak surge, showing effects of translational speed of storm and rainfall ($FK = 0.0010$ for all three cases).

Location	Slow speed		Medium speed			
	With rainfall		With rainfall		Without rainfall	
	(ft above MSL)	(time) ¹	(ft above MSL)	(time)	(ft above MSL)	(time)
Sabine Pass entrance	13.0	0	14.9	0	14.9	0
Port Arthur	14.3	2	13.2	2	12.5	2
North Sabine Lake	15.3	1	15.3	1	14.7	1
Beaumont	18.7	4	15.1	5	11.5	6
Orange Naval Station	15.9	4	14.5	5	11.7	6
Cameron	11.3	1	11.0	1	10.8	1
Lake Charles	14.1	6	14.2	6	13.2	6

¹Nearest hour after that of Sabine Pass entrance.

Comparison of the first and second sets of peak levels in Table 6 indicates a reduced response at nearly all stations within the Sabine-Calcasieu system with an increase in the translational speed of the storm, in spite of the increased surge at the shoreline (Sabine Pass entrance). A reduction in volume response within the system is expected

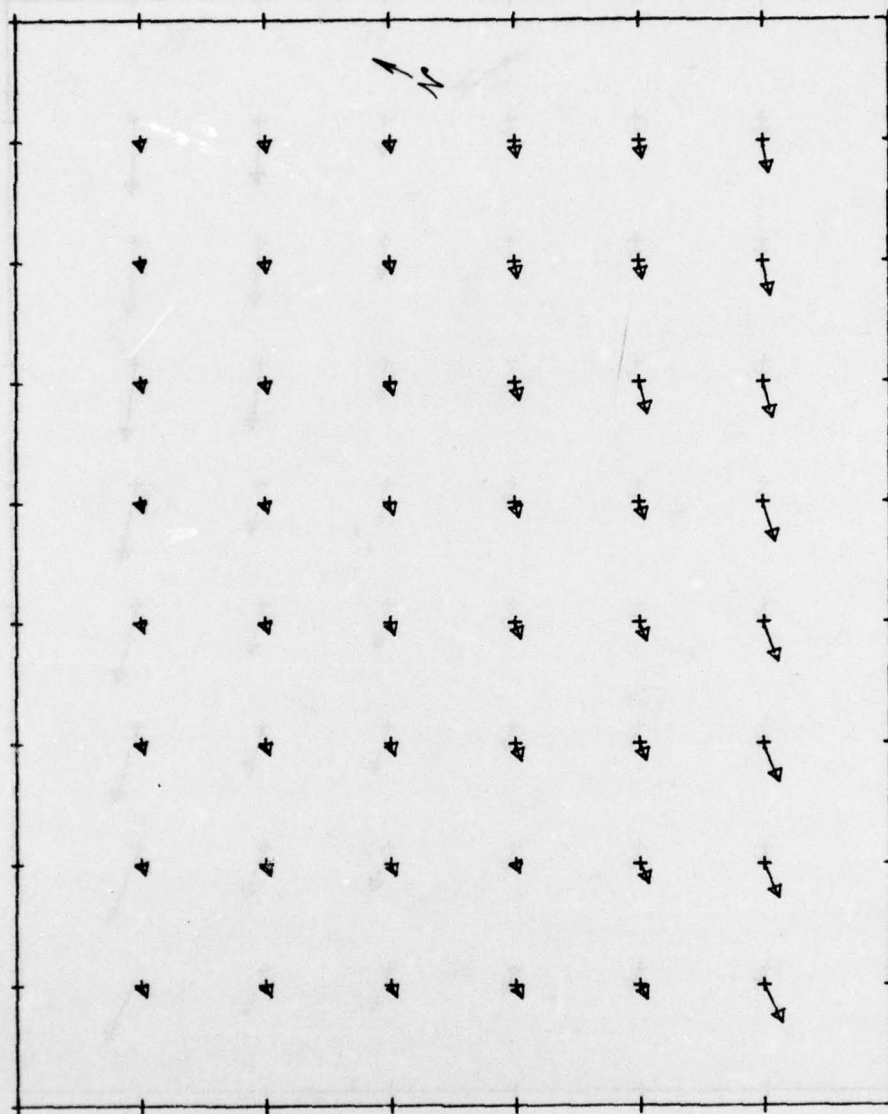


Figure 57. Wind-stress vectors for SPH large radius, medium translation (LR-MT) on an 8-nautical mile grid; time = 15 hours.

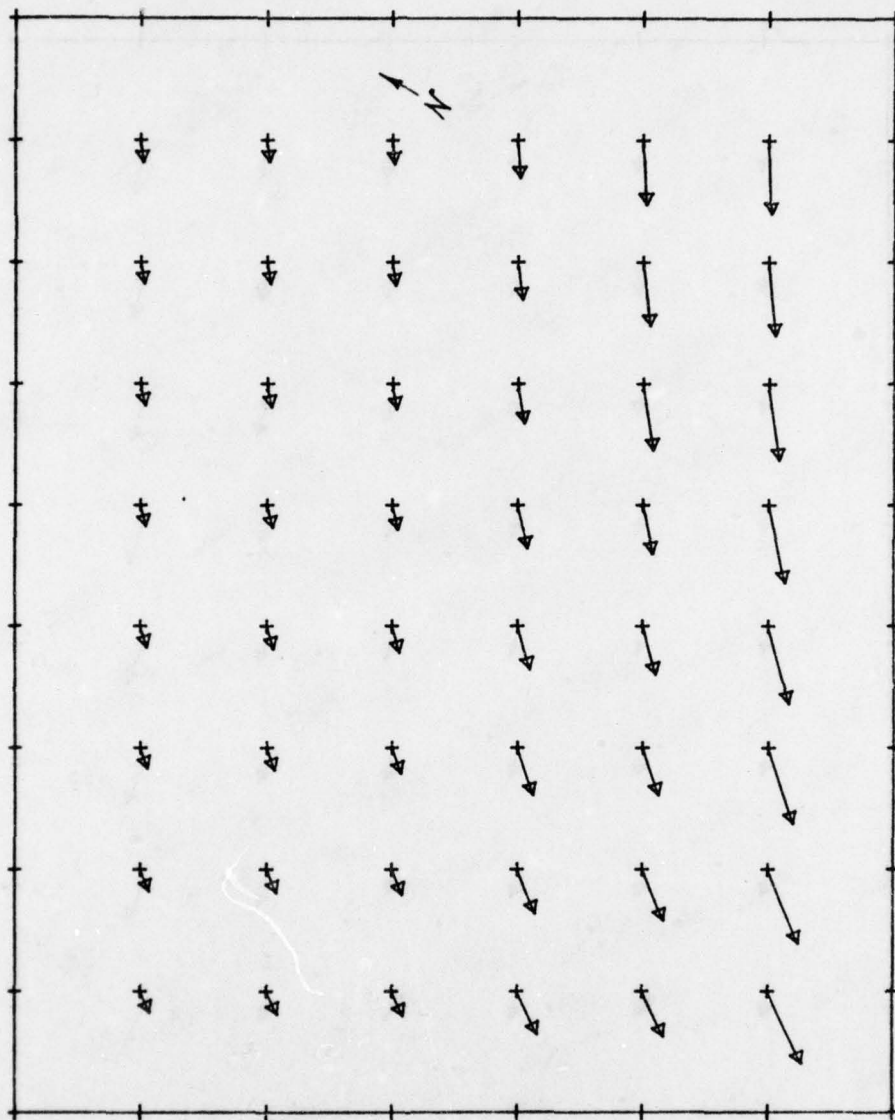


Figure 58. Wind-stress vectors for SPH large radius, medium translation (LR-MT) on an 8-nautical mile grid; time = 20 hours.

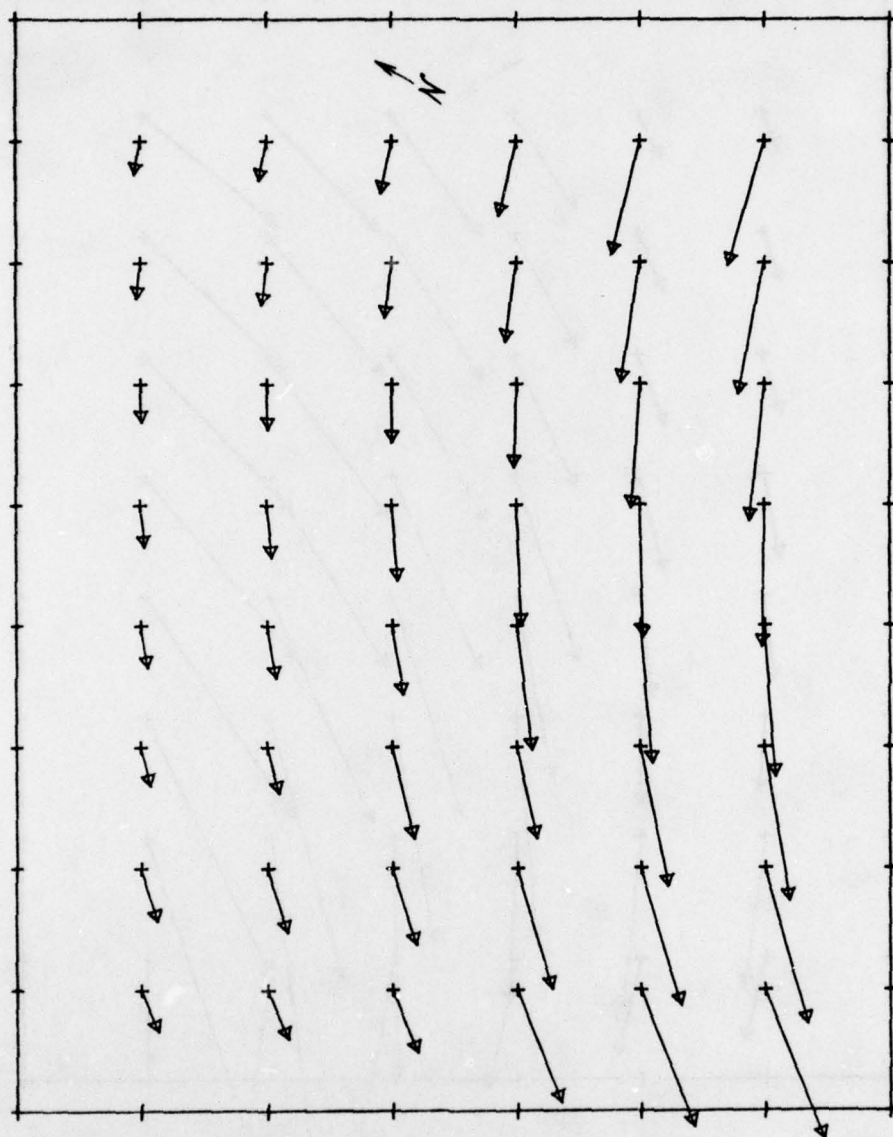


Figure 59. Wind-stress vectors for SPH large radius, medium translation (LR-MT) on an 8-nautical mile grid; time = 25 hours.

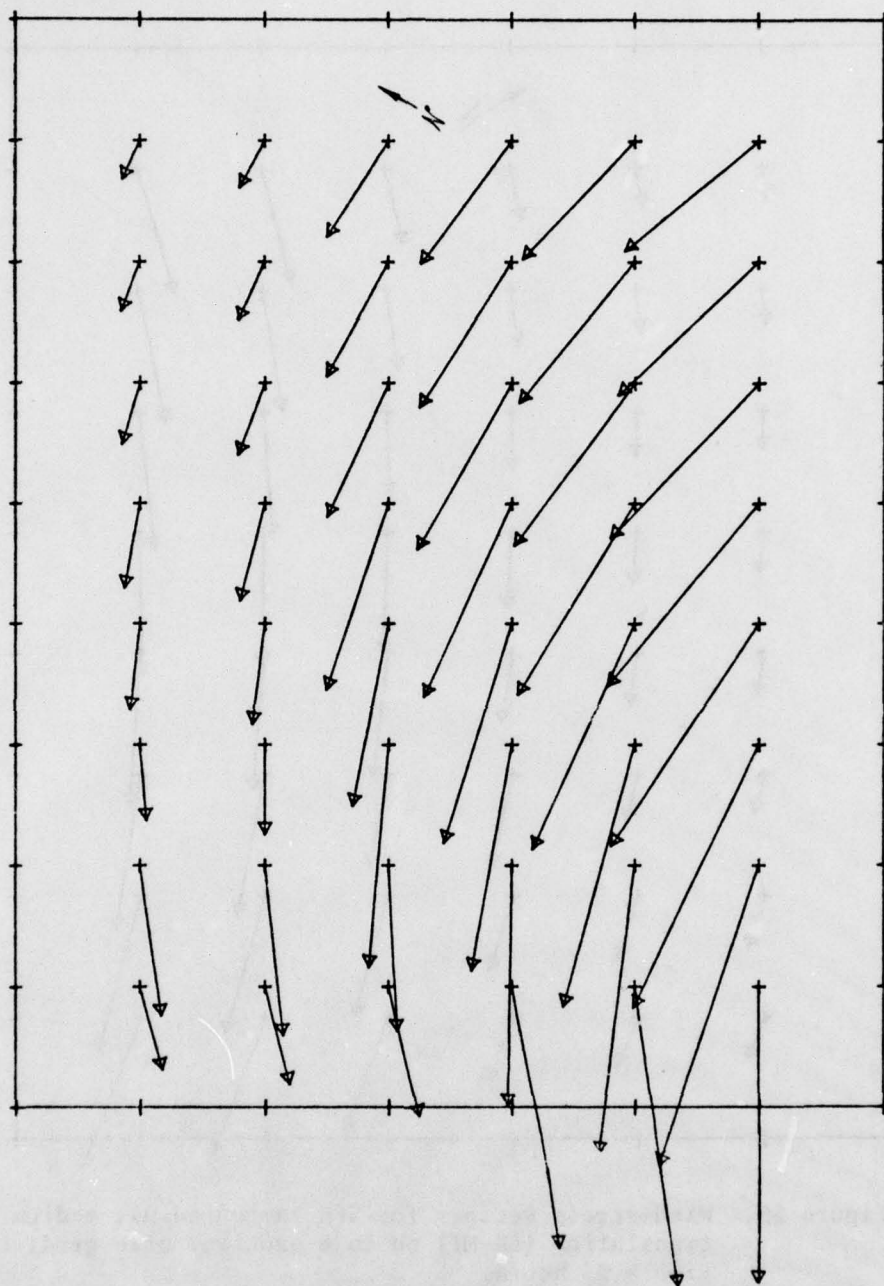


Figure 60. Wind-stress vectors for SPH large radius, medium translation (LR-MT) on an 8-nautical mile grid; time = 30 hours.

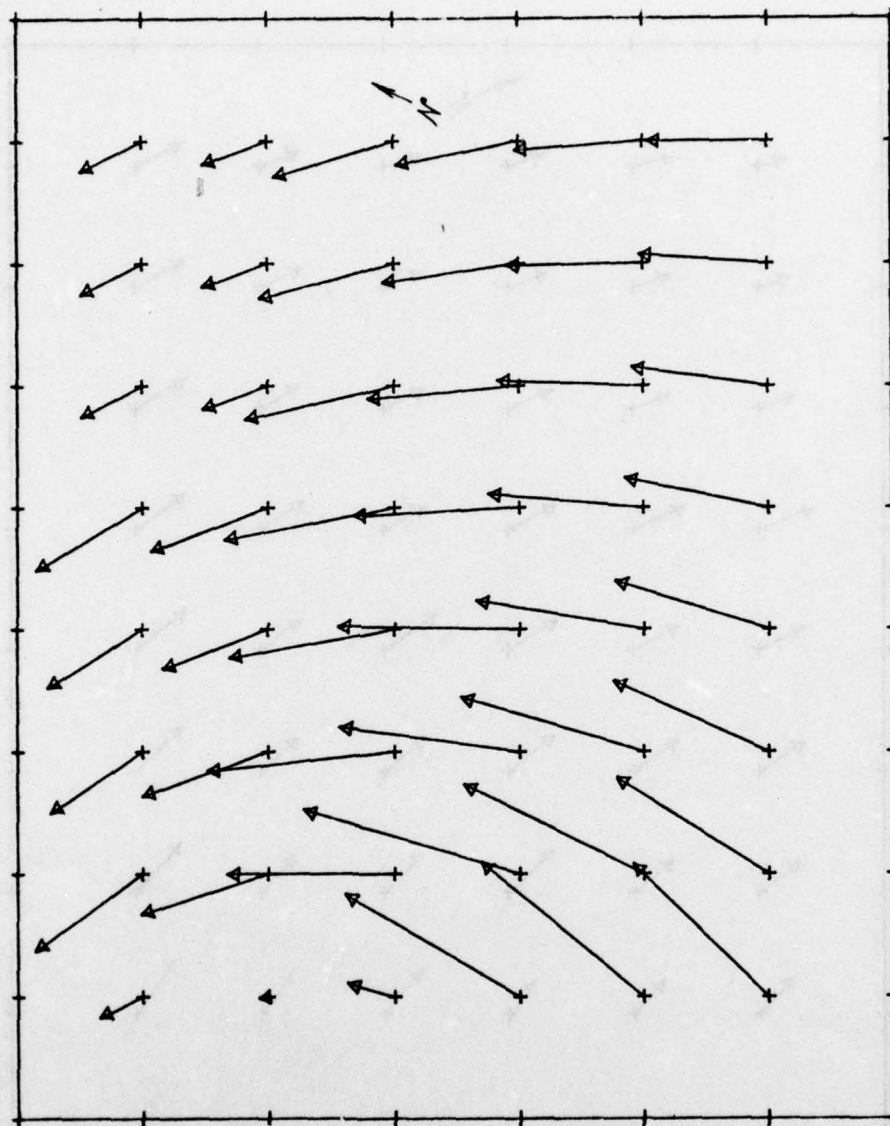


Figure 61. Wind-stress vectors for SPH large radius, medium translation (LR-MT) on an 8-nautical mile grid; time = 35 hours.

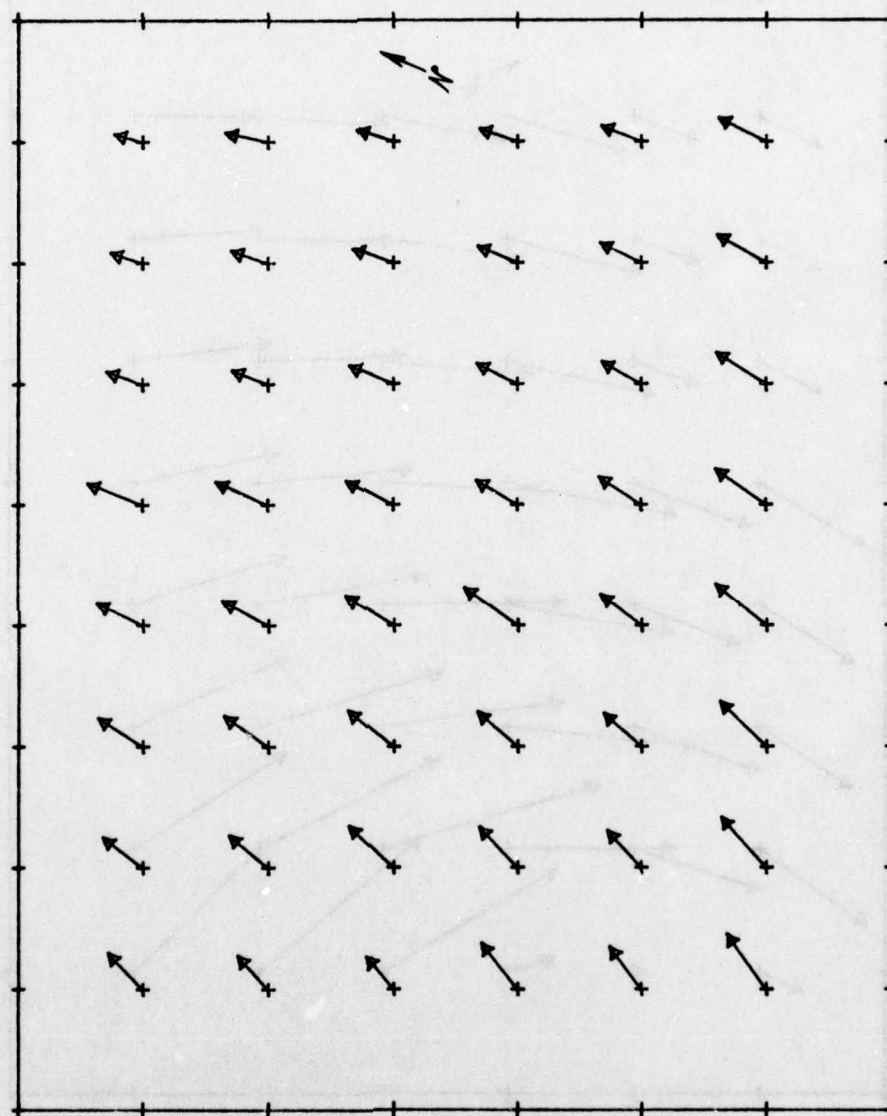


Figure 62. Wind-stress vectors for SPH large radius, medium translation (LR-MT) on an 8-nautical mile grid; time = 40 hours.

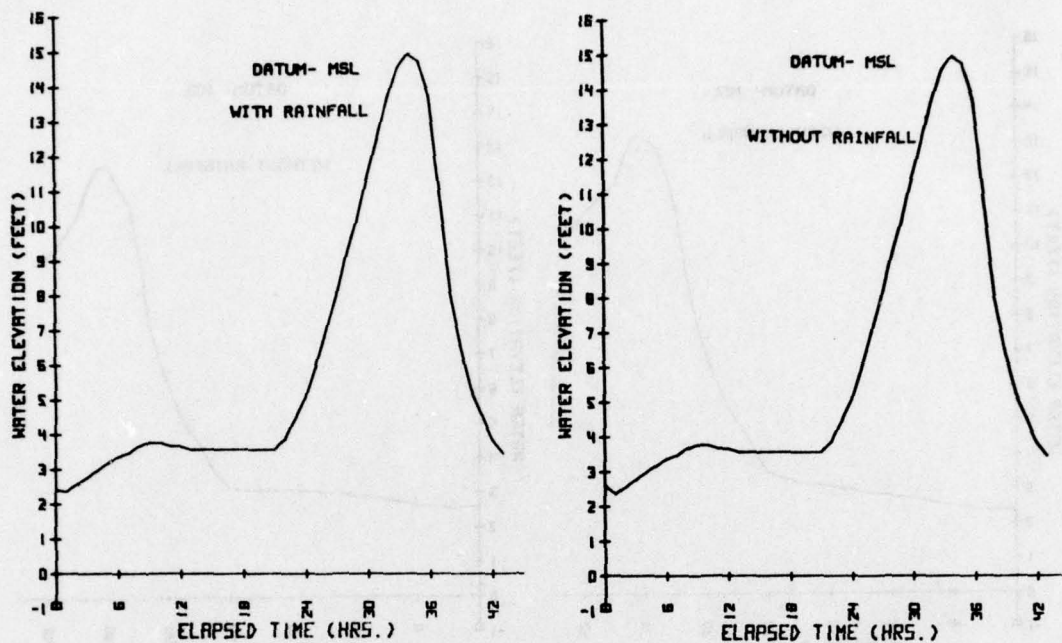


Figure 63. Hydrographs for SPH, LR-MT (with and without rainfall) at Sabine Pass, southwest jetty.

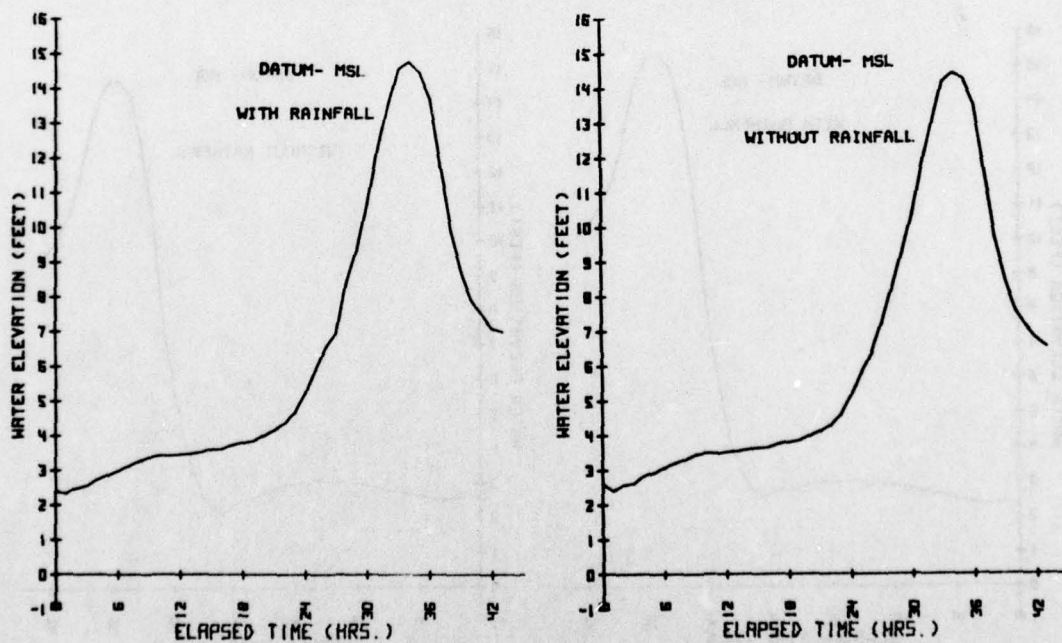


Figure 64. Hydrographs for SPH, LR-MT (with and without rainfall) at Sabine Pass, U.S. Coast Guard Station (FK = 0.0010).

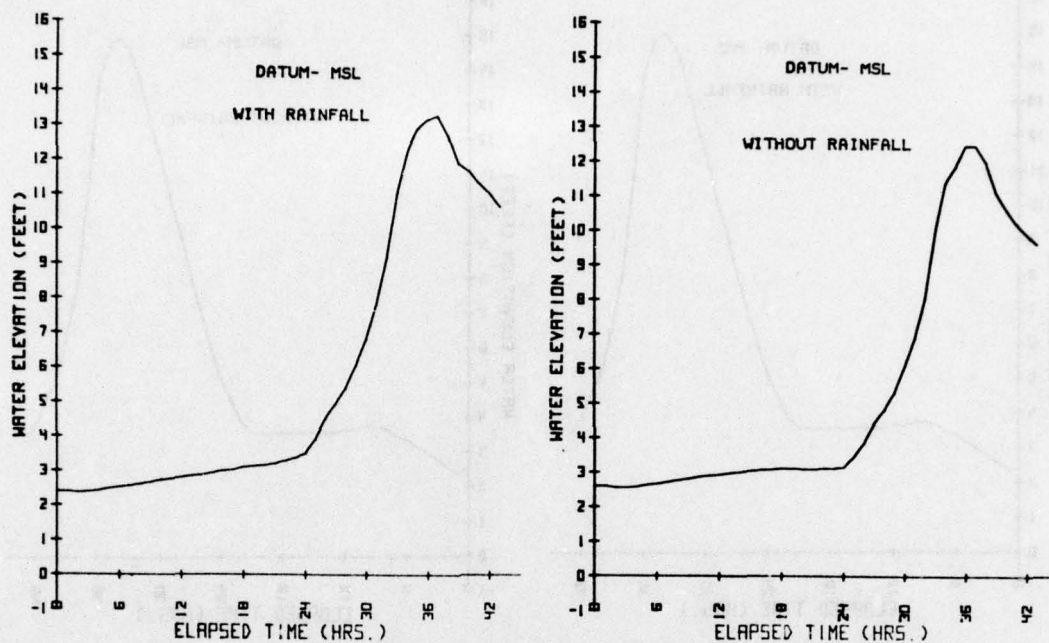


Figure 65. Hydrographs for SPH, LR-MT (with and without rainfall) at Port Arthur (FK = 0.0010).

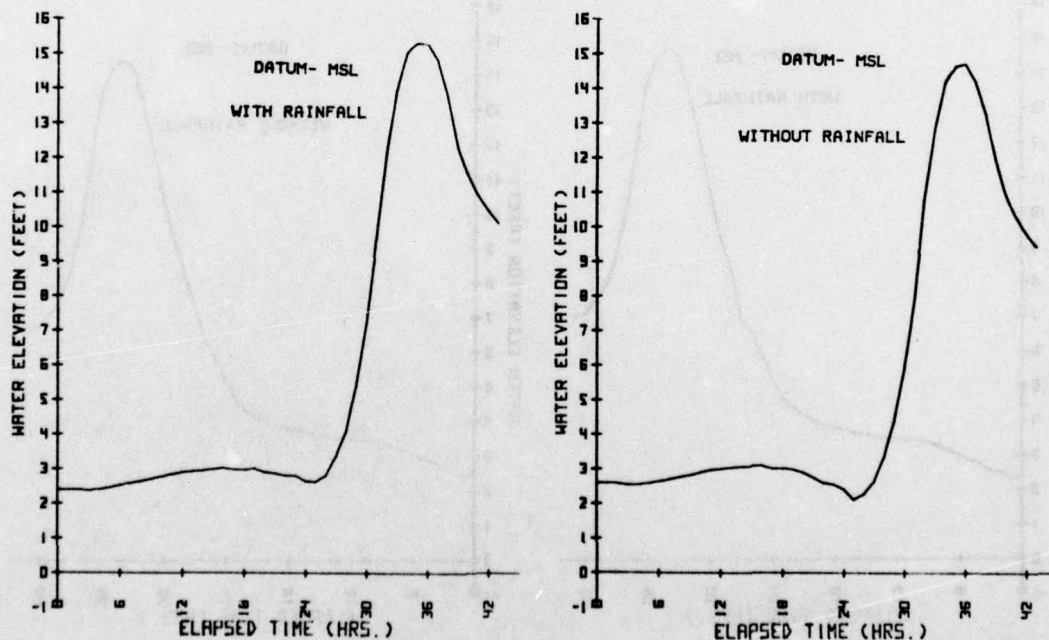


Figure 66. Hydrographs for SPH, LR-MT (with and without rainfall) at north Sabine Lake (FK = 0.0010).

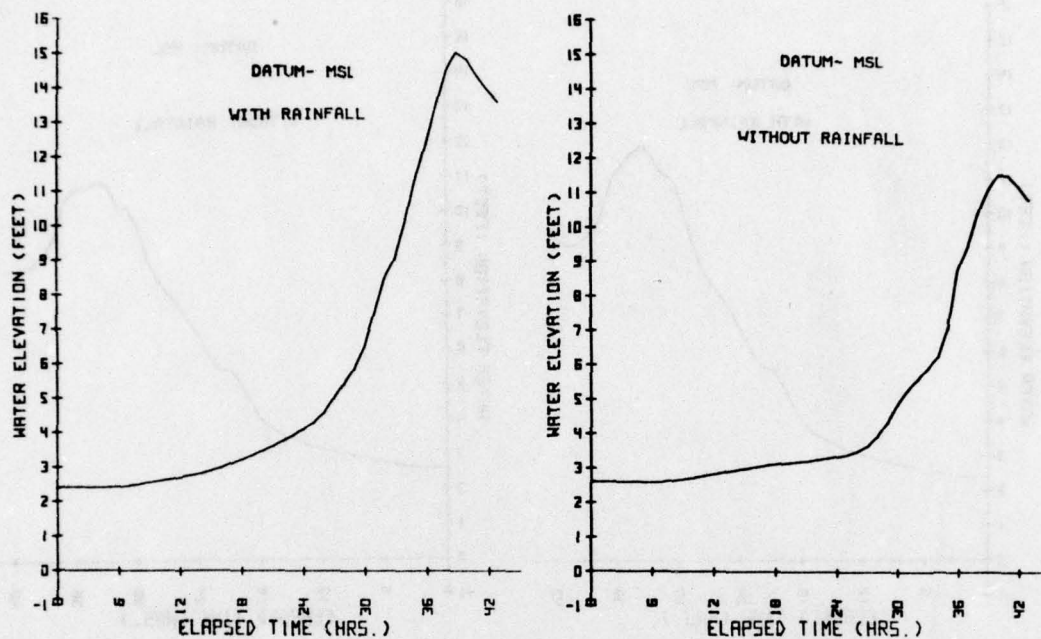


Figure 67. Hydrographs for SPH, LR-MT (with and without rainfall) at Beaumont, Neches River, and Brakes Bayou (FK = 0.0010).

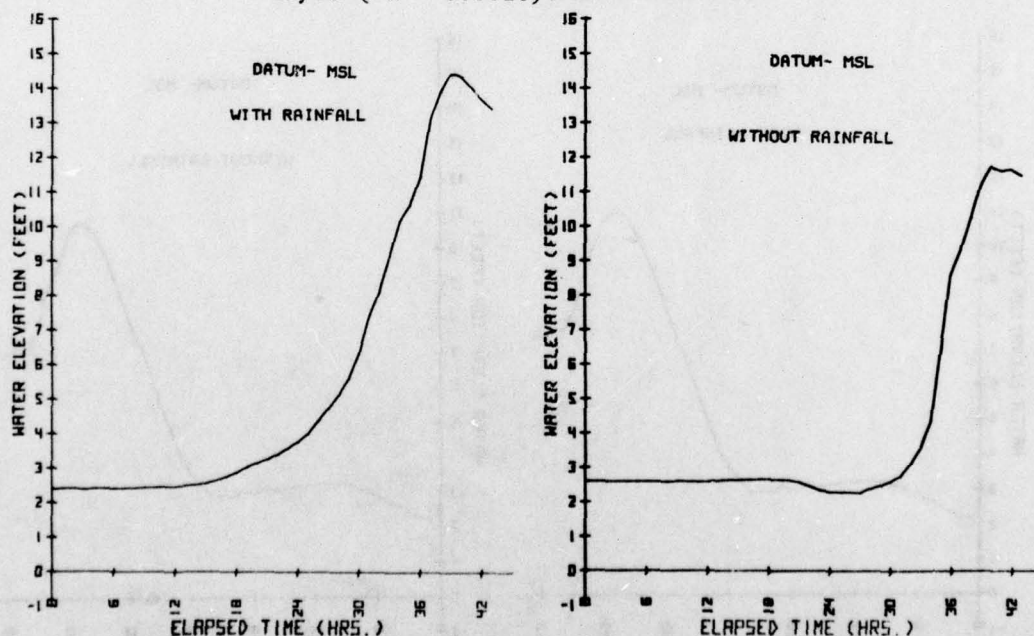


Figure 68. Hydrographs for SPH, LR-MT (with and without rainfall) at Orange Naval Station, Sabine River (FK = 0.0010).

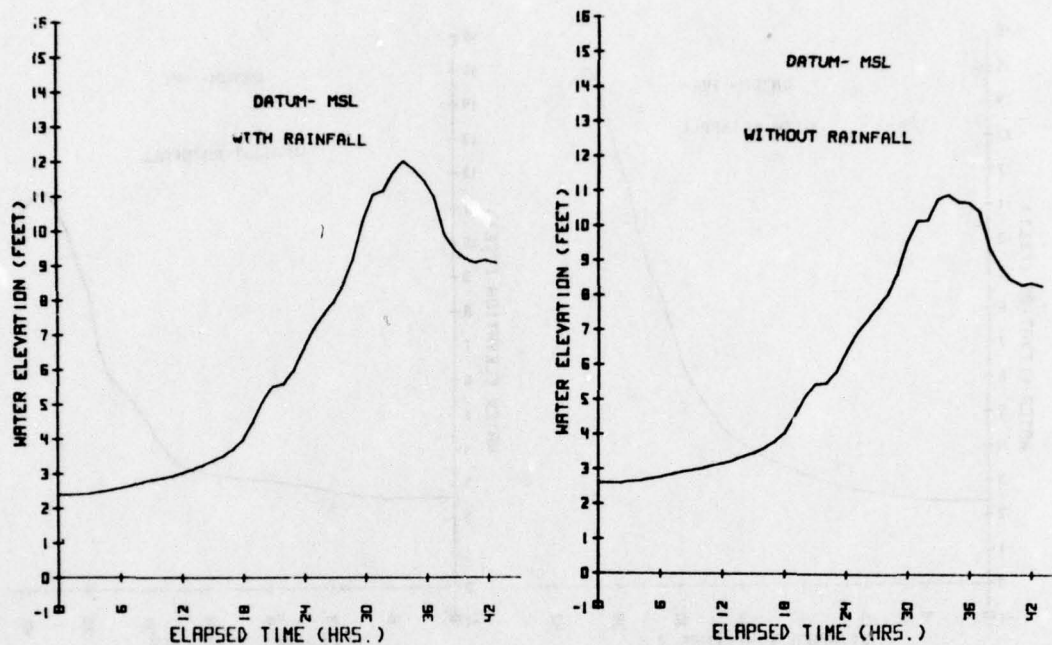


Figure 69. Hydrographs for SPH, LR-MT (with and without rainfall) at west end of Intracoastal Waterway (FK = 0.0010).

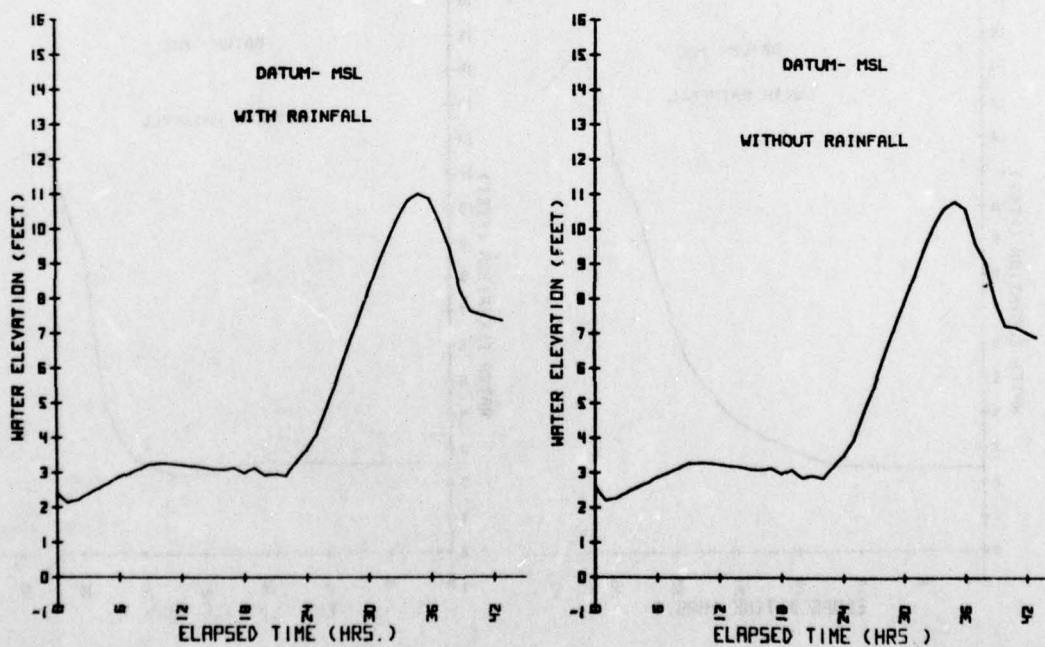


Figure 70. Hydrographs for SPH, LR-MT (with and without rainfall) at Cameron, Calcasieu Pass (FK = 0.0010).

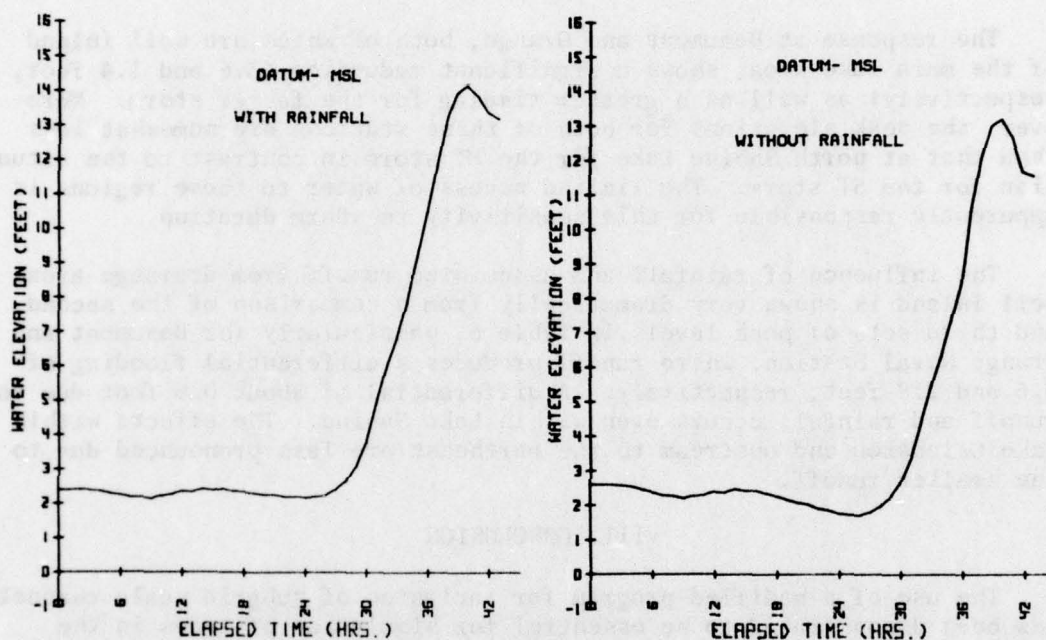


Figure 71. Hydrographs for SPH, LR-MT (with and without rainfall) at Lake Charles, Calcasieu River (FK = 0.0010).

for the greater speed (shorter duration) storm because of the constricted connection to the sea. Port Arthur shows a reduction of 1.1 feet for the MT storm relative to the ST storm; north Sabine Lake appears to show no change. An examination of the wind fields close to the time of the peak surges (Figs. 46 and 61) indicates that a greater wind-induced setup within the lake occurs between Port Arthur and the north Sabine Lake station for the medium speed storm, due to the favorable orientation of the winds near the time of peak surge at the lake entrance.

The response at Beaumont and Orange, both of which are well inland of the main lake area, shows a significant reduction (3.6 and 1.4 feet, respectively) as well as a greater timelag for the faster storm. Moreover, the peak elevations for both of these stations are somewhat less than that at north Sabine Lake for the MT storm in contrast to the situation for the ST storm. The limited access of water to these regions is apparently responsible for this sensitivity to storm duration.

The influence of rainfall and associated runoff from drainage areas well inland is shown very dramatically from a comparison of the second and third sets of peak levels in Table 6, particularly for Beaumont and Orange Naval Station, where runoff produces a differential flooding of 3.6 and 2.8 feet, respectively. A differential of about 0.6 foot due to runoff and rainfall occurs even within Lake Sabine. The effects within Lake Calcasieu and upstream to the northeast are less pronounced due to the smaller runoff.

VIII. CONCLUSION

The use of a modified program for inclusion of subgrid scale channels has been demonstrated to be essential for simulation of tides in the upper reaches of a system like the Sabine-Calcasieu region, where the primary connection to locations such as Beaumont, Orange, and Lake Charles is via river channels which would not otherwise be resolved by a grid scheme of the order of a 1-nautical mile scale. Even for conditions of extreme flooding, as occur during hurricanes, the incorporation of the subgrid scale channels provides a degree of freedom for return flow in the presence of water level gradient, which would otherwise not exist in models which exclude subgrid scale channels. The simulation of Hurricane Carla in particular is improved over that attainable with the SURGE I program which did not allow for the subgrid scale channel subroutine.

While programs such as SURGE I can, in principle, simulate the effects of channels, provided the grid scale is of the order of the channel width, the required computer time is usually prohibitive at least for explicit numerical models. Some advantage can be gained in respect to economy by the use of implicit numerical models such as that of Leendertse (1967); however, the accuracy of such schemes when used on a competitive basis, from the standpoint of economy (large time steps) can suffer relative to that which can be achieved with the subgrid scale channel routine. However, the best procedure for such numerical simulation remains to be determined.

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APPENDIX A

SURGE II PROGRAM

This appendix includes a complete listing of the SURGE II program. Except for SUBROUTINE CHANL, the program is much the same as that used in Reid and Bodine (1968). It should be emphasized that the coding of calculations of flow and water level for blocks does not include the effect of Coriolis force. Moreover, no attempt has been made to optimize the coding since the original version. The actual new part of the program is embodied in SUBROUTINE CHANL and the way in which the channel computations mesh with the block calculations. Thus, while many users may prefer their own version for calculations over the main grid, it should be possible to incorporate SUBROUTINE CHANL with their own program when applied to systems like the Sabine-Calcasieu region in which allowance for channels is essential.

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PROGRAM SURGE

7/2/74 OPT22

FTN 4.6+420

08/22/77 15.51.0

```
1      PROGRAM SURGE(INPUT,OUTPUT,TAPES=INPUT,TAPES=OUTPUT)
C
C
C
5      .....
C
C      SURGE II PROGRAM
C      FOR SIMULATION OF TIDES AND WIND INDUCED SURGES IN BAYS WITH
C      ALLOWANCE FOR SUBGRID-SCALE CHANNELS AND HARBORS
10     DEVELOPED FOR THE U. S. ARMY CORPS OF ENGINEERS, GALVESTON DISTRICT
C      BY R. O. REIN, A. C. VASTANO AND T. J. REID OF
C      COASTAL STUDIES, INC., P.O. BOX 9064, COLLEGE STATION, TX 77843
C      DECEMBER, 1974
C
15     .....
C
C
C      COMMON/BLK1/ IB(100),JB(100),IZX(100),IZY(100),ICDIX(100)
20     1,ICDIX(100),ICDSX(100),ICDSY(100),LRO(10),LROJ(10),DIST(24)
C      2,CHST(30),RO(10,30),HGR(3),XR(8,6),YP(8,6),HRR(6)
C
C      COMMON/BLK2/ IZ(28,20),U(28,20),V(28,20),H(28,20),NTIME
25     .....
C
C      COMMON/BLK3/ NM,MMIN,MMAX,VFU,INFLO,IM,JM,KM,KMAX,LMAX,DELT,DELT
C      1,CDQ,FK,HGI,INUT,KI,LJ,KII,LJJ,JBL,IR,KMM,LMM,RF,CONST,S
C      2,IMRO,JMRO,KR,ITR,IND,NOM,KIM,NORT,NTIME,INTIME,NOM,IND,GRAV
30     3,KCMP,DFU,INTER
C
C      COMMON/BLK4/ HRO(8),CRO(8),KO(24),X1(28,21),Y1(28,21),X2(28,21)
C      1,Y2(28,21),X(28),Y(28),HG1(28),HG2(28),H(1(8),H(9),VN(28)
C      2,HG(28),HR2(8)
C
C      COMMON/BLK5/ ICG(130),JCG(130),ICX(130),ICY(130),ICX(130)
35     1,IZCY(130),ICXP(130),ICXN(130),ICYP(130),ICYN(130),IC(130),IP(130)
C      2,KCM,KCX(130),KCY(130),KCH(130),UCY(130),JCF(130),KRI(130),IBN
C      3,KEN(2,130),VCT(130),VCF(130),ADGX(130),ADGY(130),KCPX(130)
C      4,KCYP(130),KLR(50),KLM,IFC(130),FC
C
C      COMMON/BLK6/ HGRN(8,25), HBRN(8,25), XRM(5,6,25), YRM(5,6,25)
40     .....
C
C      COMMON/BLK7/ IEND,NF,IBL,NJ,ALPHA(40)
C
C      COMMON/BLK8/ HS(4,72), GS(6,72), TIME(72)
45     .....
C
C      COMMON/BLK9/ KZ,LZ,NOMRO,C1,C2,C3,IMH,JMH,NY,KM,NEXT1,IT,KC,IFIRST
C      1,JAIND,NE1,XND,NE2,XNDRT, C4,HA1N,AJ,AI,LJ,KIK
C
C      COMMON/BLK10/ AGAGE,VFLO,IGAGE(12),JGAGE(12),AFLO(6),XMIN,XMAX
50     .....
C
C      CODE WORD IICARD=ON CARD ONE OF INPUT, CONTROLS INITIAL
C      COMPUTATIONS AND READ ACTIONS AS FOLLOWS
C
```

BEST AVAILABLE COPY

PROGRAM SURGE

74/74 OPT#2

FTN 4.6+420

08/22/77 10.51.06

```

55      C      ICARD#0 THIS IS FOR STARTING A PROBLEM
      C      CARD INPUT IS ICARD FOLLOWED BY THE FULL GALV
      C      DATA DECK LESS THE BLANK CARD AT THE END
      C      PLUS THE GALV LIST DECK
      C      PLUS THE CHANNEL DATA DECK (IF KCH.GT.ZERO)

60      C      ICARD#1 THIS IS FOR CONTINUING A PROBLEM
      C      CARD INPUT IS ICARD FOLLOWED BY THE CONTIN DECK
      C      THEN FOLLOWED BY THE FIRST FOUR DATA CARDS OF THE
      C      GALV DATA DECK. (THE CONTIN DECK IS OUTPUTED FROM
      C      A PREVIOUS RUN)

65      C      CODE WORD (ITAL) IS THE STARTING COLUMN IN THE LISTING OF H
      C      CODE WORD (KCH) IS THE NUMBER OF BLOCKS WITH CHANNELS
      C      CODE WORD (NOIND#) IS NEGATIVE FOR NO HIND FIELD (OMITS CARD INPUT
      C      OF H,V FIELDS AND USES FORMAT #10 FOR H,V INPUT)
70      C      INTER IS THE INTERVAL USED IN THE SAVE H OPERATION
      C      NGAGE IS THE NUMBER OF HYDROGRAPHS TO BE SAVED
      C      NFLOW IS THE NUMBER OF FLOW GRAPHS TO BE SAVED
      C      IMIN AND IMAX ARE THE DESIRED LOWER AND UPPER LIMITS OF H FOR

75      C      GRAPHICAL OUTPUT
      C      PRINT 1A
      C      10 FORMAT (14I)
      C      READ 1, ICARD
      C      1 FORMAT (11,13,10I4)
      C      IF (ICARD.EQ.0) GO TO 2
      C      HEADS PREVIOUS RESULTS FOR CONTINUATION OF PROBLEM
      C      CALL CONTIN(1)
      C      INTIME=TIME
      C      2 CONTINUE
85      C      READ 1, INENT, IRL, KCH, NOIND, INTER, NGAGE, NFLOW, IMIN, IMAX
      C      XMAX = IMAX
      C      XMIN = IMIN
      C
      C      50 FORMAT (3X, ICARD#, I2, I, IRL#, I3, I, KCH#, I4, I, NOIND#, I3
      C      1, I, INTER#, I3, I, NGAGE#, I3, I, NFLOW#, I3, I, IMIN#, I3, I, IMAX#, I3, I)
      C      51 FORMAT (3X, IONT#, I12, I, INTIME#, I3, I, XMAX#, I4, I, XMIN#, I4
      C      1, I, XMAX#, I4, I, NFLOW#, I4, I, IONT#, I10, I, IRL#, I4, I)
      C      52 FORMAT (3X, IONT#, I12, I, IRL#, I3, I, J#, I3, I, KCH#, I4, I, XMAX#, I3
      C      1, I, XMIN#, I3, I)
      C      53 FORMAT (3X, IONT#, I12, I, DELTA#, I4, I, N H1, DELTA#, I4, I,
      C      1, I, SEC, COG#, I4, I, F#, I7, I, F#, I7, I, WGT#, I4, I, FT, I)
      C      54 FORMAT (3X, IONT#, I12, I, KIR#, I3, I, LU#, I3, I, KIR#,
      C      1, I3, I, LUJ#, I4, I, J#, I3, I, J#, I3, I, J#, I3, I, J#, I3, I)
      C      55 FORMAT (3X, IONT#, I12, I, IRL#, I3, I, J#, I3, I, ZK#, I5, I, ZV#, I5
      C      1, I, COG#, I4, I, COG#, I4, I, COS#, I4, I, COS#, I4, I)
      C      57 FORMAT (3X, IONT#, I12, I, IRL#, I3, I, IRL#, I3, I, KCH#, I3, I, KCH#, I3
      C      1, I, IRL#, I3, I, IRL#, I3, I, KCH#, I3, I, KCH#, I3, I)
      C      58 FORMAT (3X, IONT#, I12, I, IRL#, I3, I, IRL#, I3, I, KCH#, I3, I, KCH#, I3
      C      1, I, IRL#, I3, I, IRL#, I3, I, KCH#, I3, I, KCH#, I3, I)
      C      75 FORMAT (3X, IONT#, I12, I, IRL#, I3, I, IRL#, I3, I, KCH#, I3, I, KCH#, I3
      C      1, I, IRL#, I3, I, IRL#, I3, I, KCH#, I3, I, KCH#, I3, I)
      C      76 FORMAT (3X, IONT#, I12, I, IRL#, I3, I, IRL#, I3, I, KCH#, I3, I, KCH#, I3
      C      1, I, IRL#, I3, I, IRL#, I3, I, KCH#, I3, I, KCH#, I3, I)

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PROGRAM SURGE

74/74 OPT=2

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```
79 FORMAT(1, /3X, (IDNT= 9(1,3X,13, ( PATHS OF I,J FOR RUNOFF LOCATIONS MAIN0054
1( ) MAIN0055
110 80 FORMAT(1, /3X, (IDNT= 10(1,3X, (PERCENT RAINFALL EACH MARTIME() MAIN0056
81 FORMAT(1, /3X, (IDNT= 11(1,3X, (CHANNEL STRESS VALUES AT MARTIMES() MAIN0057
82 FORMAT(1, /3X, (IDNT= 12(1,3X,13, ( SETS OF RUNOFF VALUES IN CFS FOR MAIN0058
1(,13, ( MARTIMES() MAIN0059
83 FORMAT(1, /3X, (MGR FOLLOWED BY HRR ARRAY (IN FEET) AT MTIME=(1,14) MAIN0060
84 FORMAT(1, /3X, (XR VALUES (IDNT=6) AND YR VALUES (IDNT=7) AT MARTIM MAIN0061
115 1E=1,14) MAIN0062
C
PRINT 50, ICARD, IBL, KCM, NCM, IND, INTER, NGAGE, NFLOW, IMIN, IMAX MAIN0063
READ 100, IDNT1, NTIME, NM, MMIN, MMAX, NFM, IOUT, INFLO MAIN0064
PRINT 51, IDNT1, NTIME, NM, MMIN, MMAX, NFM, IOUT, INFLO MAIN0065
120 100 FORMAT (11, 2X, 15, 9(3X, 15)) MAIN0066
101 FORMAT(1X, 12, 15, 9(3X, 15)) MAIN0067
IF (IDNT1=1) 1000, 150, 1000 MAIN0068
150 READ 100, IDNT1, I, J, K, KMAX, LMAX MAIN0069
PRINT 52, IDNT1, I, J, K, KMAX, LMAX MAIN0070
125 KMM=KMAX-1 MAIN0071
LMM=LMAX-1 MAIN0072
IF (IDNT1=2) 1000, 200, 1000 MAIN0073
200 READ 250, IDNT1, DELX, DELT, COU, FK, FC, HGI MAIN0074
PRINT 53, IDNT1, DELX, DELT, COU, FK, FC, HGI MAIN0075
130 DELX=DELX*6080. MAIN0076
250 FORMAT (11, F7.0, 9F8.0) MAIN0077
260 FORMAT (1X, 11, F7.3, 9F8.4) MAIN0078
IF (IDNT1=3) 1000, 295, 1000 MAIN0079
295 IF (ICARD, FC, 0) GO TO 3 MAIN0080
135 NTIME=INTIME MAIN0081
PRINT 10 MAIN0082
GO TO 745
C
3 CONTINUE MAIN0085
INTIME=NTIME MAIN0086
140 300 READ 100, IDNT1, KI, LJ, KII, LJJ, JRL, JRR MAIN0087
PRINT 54, IDNT1, KI, LJ, KII, LJJ, JRL, JRR MAIN0088
IF (IDNT1=4) 1000, 350, 1000 MAIN0089
350 IF (NFM=MMAX=NM) 365, 300, 365 MAIN0090
145 360 IF (NFM=NTIME=NTIME) 365, 368, 365 MAIN0091
365 PRINT 366 MAIN0092
366 FORMAT (21, MMIN OR MMAX IN ERROR ) MAIN0093
368 CONTINUE MAIN0094
IF ((KI=KMM)=IM) 440, 380, 370 MAIN0095
150 370 IF ((KII=KMM)=IM) 380, 440, 440 MAIN0096
380 IF ((LJ=LMM)=JM) 440, 400, 390 MAIN0097
390 IF ((LJJ=LMM)=JM) 400, 440, 440 MAIN0098
400 IF (KMM = KII) 410, 410, 460 MAIN0099
410 IF (LMM = LJJ) 460, 460, 460 MAIN0100
155 440 PRINT 450 MAIN0101
450 FORMAT (1H, 42H K=L RANGE NOT CONSISTENT WITH I,J RANGE////) MAIN0102
GO TO 490 MAIN0103
C
460 PRINT 470 MAIN0104
```

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PROGRAM SURGE

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```
160      470 FORMAT (1H 'RANGE OF K OR L TOO LARGE FOR PROGRAM'////)      MAIN0105
      480 STOP                                                                MAIN0106
      490 CONTINUE                                                            MAIN0107
      IF (KM.EQ.0) GO TO 501                                                  MAIN0108
      PRINT 74                                                                MAIN0109
      DO 500 K=1,KM                                                            MAIN0110
165      READ 100, IDNT1,IB(K),JB(K),IZ(K),IZY(K),ICDOK(K),ICDOY(K),ICDSX(K),ICDSY(K)      MAIN0111
      2K),ICDSY(K)                                                            MAIN0112
      PRINT 55, K,IB(K),JB(K),IZ(K),IZY(K),ICDOK(K),ICDOY(K),ICDSX(K),ICDSY(K)      MAIN0113
      1K),ICDSY(K)                                                            MAIN0114
170      IF (IDNT1-5)1000,500,1000                                           MAIN0115
      500 CONTINUE                                                            MAIN0116
      501 CONTINUE                                                            MAIN0117
      PRINT 74                                                                MAIN0118
      DO 550 I=1,IM                                                            MAIN0119
175      READ 100, IDNT1,(IZ(I,J),J=1,10)                                    MAIN0120
      PRINT 101, I,(IZ(I,J),J=1,10)                                          MAIN0121
      IF (IDNT1-6)1000,540,1000                                              MAIN0122
      540 READ 100, IDNT1,(IZ(I,J),J=1,10)                                    MAIN0123
      PRINT 101, I,(IZ(I,J),J=1,10)                                          MAIN0124
180      IF (IDNT1-6)1000,550,1000                                           MAIN0125
      550 CONTINUE                                                            MAIN0126
      READ 100, IDNT1,IMRO,J=RO,KR,ISTR,IND,NO,AT,NORT                      MAIN0127
      PRINT 47,IDNT1,IMRO,J=RO,KR,ISTR,IND,NO,AT,NORT                      MAIN0128
      IF (IDNT1-7)1000,560,1000                                              MAIN0129
185      560 READ 250, IDNT1,WF,CONST,S                                     MAIN0130
      PRINT 58,IDNT1,WF,CONST,S                                              MAIN0131
      IF (IDNT1-9)1000,570,1000                                              MAIN0132
      570 PRINT 70, I=RO                                                    MAIN0133
      READ 100, IDNT1,(LHOI(JJJ),LHOJ(JJJ),JJJ=1,5)                        MAIN0134
      PRINT 101,IDNT1,(LHOI(JJJ),LHOJ(JJJ),JJJ=1,5)                        MAIN0135
190      IF (IMRO.LT.0) GO TO 575                                             MAIN0136
      READ 100, IDNT1,(LHOI(JJJ),LHOJ(JJJ),JJJ=6,T=RO)                    MAIN0137
      PRINT 101,IDNT1,(LHOI(JJJ),LHOJ(JJJ),JJJ=6,T=RO)                    MAIN0138
      575 CONTINUE                                                            MAIN0139
195      IF (IDNT1-9)1000,580,1000                                           MAIN0140
      580 IF (NORT.NE.0)590,590,600                                          MAIN0141
      590 IF (K1=NO-JAP0,020,000)                                           MAIN0142
      600 PRINT 610                                                            MAIN0143
      610 FORMAT (40H K1 OR NORT EXCEEDS NO+OR BOTH EXCEED NO+ )          MAIN0144
200      C                                                                    MAIN0145
      620 IF (NO+IND.GE.0) GO TO 625                                          MAIN0146
      JH00=1                                                                    MAIN0147
      GO TO 690                                                                MAIN0148
205      625 PRINT 80                                                         MAIN0149
      DO 630 K1=1,2                                                            MAIN0150
      K22=100*(K12=1)+1                                                        MAIN0151
      K23=100*(K12=1)                                                        MAIN0152
      READ 250, IDNT1,(OIST(K),K=22,K23)                                    MAIN0153
      PRINT 240,IDNT1,(OIST(K),K=22,K23)                                    MAIN0154
210      IF (IDNT1-1)999,630,999                                           MAIN0155
      630 CONTINUE                                                            MAIN0156
      READ 250, IDNT1,(OIST(K),K=21,24)
```

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PROGRAM SURGE

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```
      PRINT 260, IDNT1, (DIST(K), K=21, 24)
      IF (IDNT1=0) 900, 600, 999
215      600 K33=0/10
      IF (K4, EQ, 0) GO TO 690
      IF (K33 1 671, 670, 671)
      670 K32 = 0
      GO TO 672
220      671 PRINT 81
      DO 680 K=1, K33
      K31=10*(K-1)+1
      K32=10*K
      READ 250, IDNT1, (CHST(K4), K=K31, K32)
225      PRINT 240, IDNT1, (CHST(K4), K=K31, K32)
      IF (IDNT1=1) 900, 600, 999
      680 CONTINUE
      672 K31=K32+1
      READ 250, IDNT1, (CHST(K4), K=K31, K32)
230      PRINT 240, IDNT1, (CHST(K4), K=K31, K32)
      IF (IDNT1=1) 900, 600, 999
      C
      690 PRINT 82, I=0, J=0
      DO 700 J=1, J=0
235      READ 250, IDNT1, (RO(I, J), I=1, I=0)
      PRINT 241, IDNT1, (RO(I, J), I=1, I=0)
      261 FORMAT (1X, I1, F7.0, 9PB, 0)
      IF (IDNT1=2) 900, 700, 999
      700 CONTINUE
240      WRITE
      I=0
      705 CONTINUE
      IF (NO-ING, LT, 0) GO TO 900
      C
245      710 READ 100, IDNT1, MTIME
      720 IF (IDNT1=1) 900, 730, 999
      730 IF (MTIME=0) 700, 700, 740
      740 PRINT 700, MTIME
      750 FORMAT (1X, MTIME ERROR AT ,15)
250      760 PRINT 83, MTIME
      READ 250, IDNT1, (HOR(K), K=1, KMAX)
      PRINT 260, IDNT1, (HOR(K), K=1, KMAX)
      IF (IDNT1=4) 900, 770, 999
      770 READ 250, IDNT1, (HOR(J), J=2, 8)
255      PRINT 240, IDNT1, (HOR(J), J=2, 8)
      IF (IDNT1=5) 900, 780, 999
      780 PRINT 84, MTIME
      DO 790 K=1, KMAX
      READ 250, IDNT1, (K(K, L), L=1, LMAX)
260      PRINT 240, IDNT1, (K(K, L), L=1, LMAX)
      IF (IDNT1=6) 900, 790, 999
      790 CONTINUE
      DO 800 K=1, KMAX
      READ 250, IDNT1, (Y(K, L), L=1, LMAX)
265      PRINT 260, IDNT1, (Y(K, L), L=1, LMAX)
```

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      IF (IDNT1-7)000,800,999
      800 CONTINUE
      C*****ADJUST AINO FIELD*****
      DO 802 K=1,3
      270 DO 802 L=1,LMAX
      XR(K,L)=0.3*XR(K,L)
      YR(K,L)=0.3*YR(K,L)
      802 CONTINUE
      C*****
      275 801 IT=11+1
      DO 810 I=1,KMAX
      HGR(I,IT)=HGR(I)
      DO 820 J=1,LMAX
      XRM(I,J,IT)=XR(I,J)
      280 YRM(I,J,IT)=YR(I,J)
      810 CONTINUE
      DO 830 J=2,8
      830 HRM(J,IT)=HR(J)
      IF(4TIME-HMAX) 710,1015,1015
      285 C
      900 MU=1
      DO 901 K=1,1-RO
      901 HRO(K)=RO(K+1)
      PRINT 914
      290 914 FORMAT(1)
      PRINT 915
      905 READ 910, IGA, 4TIME, (M(1+J),J=1,12)
      910 FORMAT(I2,I4,12F6.2)
      915 FORMAT(1) THE FOLLOWING ARE HOURLY TIDE LEVELS OUTSIDE MAIN(,
      295 1) PASS FOR TIDE CALIBRATION WITH NO AINO(,/)
      PRINT 910, IGA, 4TIME, (M(1+J),J=1,12)
      IDNT1=IGA
      IF(IGA .NE.1) GO TO 999
      HLEMTIME=1
      MU=MTIME+12
      300 DO 940 J=1,12
      H=ML+J=1
      DO 920 I=1,KMAX
      920 HGR(I,M)=H(1,J)
      DO 930 I=2,8
      305 930 HBR(I,M)=H(1,J)
      940 CONTINUE
      IF(MU,LT,HMAX) GO TO 905
      C
      310 DO 950 K=1,KMAX
      DO 950 L=1,LMAX
      XR(K,L)=0.0
      YR(K,L)=0.0
      950 CONTINUE
      315 DO TO 1015
      C
      999 IDNT1=IDNT1+10
      1000 PRINT 1010,IDNT1
      1010 FORMAT(1) ERROR IN DATA - MISSING (1,14,2X)MCARD)
      320 STOP
      C
      1015 CALL PART 2
      C
      1020 STOP
      325 END

```

MAIN0205

MAIN0206

MAIN0207

MAIN0208

MAIN0209

MAIN0210

MAIN0211

MAIN0212

MAIN0213

MAIN0214

000001IN0215

MAIN0216

000001IN0217

MAIN0218

MAIN0219

MAIN0220

MAIN0221

MAIN0222

MAIN0223

MAIN0224

MAIN0225

MAIN0226

MAIN0227

MAIN0228

MAIN0229

MAIN0230

MAIN0231

MAIN0232

MAIN0233

000001IN0234

000001IN0235

MAIN0236

MAIN0237

MAIN0238

MAIN0239

MAIN0240

MAIN0241

MAIN0242

MAIN0243

MAIN0244

MAIN0245

MAIN0246

MAIN0247

MAIN0248

MAIN0249

MAIN0250

MAIN0251

MAIN0252

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SUBROUTINE PART2

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```
1      C
      C
      SUBROUTINE PART 2                                PT2-0001
5      C
      COMMON/RLK1/ IR(100),JB(100),IZX(100),IZY(100),ICDCK(100)
      1:ICDGY(100),ICD5A(100),ICD5Y(100),LQOI(A),LQOJ(A),DIST(24)
      2:CHST(30),RD(A,30),HGR(5),XM(A,6),YR(A,6),HRR(6)
      COMMON/RLK2/ IZ(24,20),UC(24,20),V(24,20),M(24,20),PTIME
      COMMON/RLK3/ IM,IMIN,MNAX,NFU,INFLO,IN,J,K,M,KMAX,LMAX,DELX,DELT
      1:COD,FK,HGI,IOHT,KI,LJ,KII,LJJ,JHL,JEP,KVM,LUM,RF,CONST,S
      2:IMRO,JMRO,KR,ISTR,IND,NQ,KIN,NORT,PTIME,INTIME,NC,IND,GRAV
      3:KCHP,DFI,INTER
      COMMON/RLK4/ HRO(8),CRO(6),KH(24),X1(24,21),Y1(24,21),X2(24,21)
      1:Y2(24,21),X(24),Y(24),HG1(24),HG2(24),HJ1(8),HJ2(9),VN(24)
      2:HG(24),HJ(8)
      15      COMMON/RLK5/ ICG(130),JCG(130),ICFX(130),ICFY(130),IZCX(130)
      1:IZCY(130),QCV(130),QCVN(130),QCV(130),QCV(130),HC(130),HP(130)
      2:KCM,KCX(130),KCY(130),KCH(130),UCY(130),UCF(130),KRI(130),IMH
      3:KEN(2,130),VCT(130),VCF(130),ADGX(130),ADGY(130),KEXP(130)
      20      4:KCV(130),KLR(50),KLM,IFC(130),FC
      COMMON/RLK6/ MGRM(8,25),HSH(8,25),XK(8,6,25),YK(8,6,25)
      COMMON/RLK7/ TEND,NF,IBL,NJ,ALPHA(40)
      COMMON/RLK8/ HS(9,72),OS(6,72),TIME(72)
      COMMON/RLK9/ KZ,LZ,NUMRO,C1,C2,C3,IMH,JH,NT,NN,EXT1,IF,KC,IFIRST
      25      1:JHIND,NE-1,XNO,NE-3,XNORT, Cu,HAIN,AJ,ILJJ,KIK
      COMMON/RLK10/ NGAGE,NFLOW,IGAGE(12),JGAGE(12),XFLOW(6),XMIN,XMAX
      C
      ARSF(X)=ARSF(X)                                PT2-0026
      SGRTF(X)=SGRTF(X)                              PT2-0027
30      C
      NUMRO=IMRO
      KZ=K1
      LZ=LJ
      GRAV=32.1454355
      C1=FX*DELT
      35      C2=(GRAV*DELT)/(2.*DELT)
      C3=DELT/DELT
      IMH=1
      JH=JH
      40      NT=0
      NJ=0
      NEXTI=1
      IT=2
      KCR=1
      45      IFIRST=1
      J=INJHFI=1
      DO 130 I=1,IM
      VN(I)=0.
      DO 130 J=1,JM
      50      M(1,J)=I2(I,J)
      Z=IZ(I,J)
      IF(Z,LT,HGI) M(I,J)=HGI
      U(I,J)=0.
      PT2-0050
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SUBROUTINE PART2

74/74 OPT=2

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```

95      130 V(I,J)=0.
          MTIME=0
          C
          C READ CHANNEL DATA AND ESTABLISH KEY ARRAYS
          IF(KCH.GT.0) CALL CHANL(1)
          C READ GAGE LOCATIONS FOR SAVING KEY M AND Q VALUES AS TIME SEQUENCE
          60 CALL SAVE(1)
          C READ LIST DATA AND PRINT PROBLEM IDENTIFICATION AND Z FIELD
          READ 15, IDEFAT, IEND, NF, IBEGLN, NJ, NCARD
          15 FORMAT(I1,I4,4I5)
          140 FORMAT( /1H,15A2,15A2,10A2)
          65 220 FORMAT(1H1)
          230 FORMAT(15A2,15A2,10A2)
          PRINT 220
          DO 250 J=1,NCARD
          READ 230, (ALPHA(I),I=1,40)
          70 240 PRINT 140, (ALPHA(I),I=1,40)
          PRINT 230
          C
          C
          148 CONTINUE
          75 RF=(RF/12.)*CONST
          NEW=NDW
          XNO=NDW
          NE=3*NDW
          XNORT=NDW
          80 ISTR=ISTR*NFU
          IND=IND*NFU
          AJ=JZ
          AI=KZ
          JMM=JW-1
          85 LJ=JLZ-1
          KIK=KZ-1
          C
          ENTRY PART 2B
          NU = (NM-TIME)/INTER
          90 CALL PLOT
          C PLOT CHANNELS AND BARRIERS
          CALL SAVE(2)
          IF(KCH.GT.0) CALL CHANL(4)
          C START OF TIME INCREMENTING LOOP
          95 200 CONTINUE
          IF(NOWIND.LT.0) GO TO 430
          300 IF (NE=1+NDW) 330+310+310
          310 C=(CHST(NEXT1+1)-CHST(NEXT1))/XNO
          BIND=CHST(NEXT1)
          100 NE=1
          DO 320 NE=2,1+NDW
          320 CHD(NE)=RO(NF=2,NEXT1+1)-RO(NE=2,NEXT1)/XNO
          NEXT1=NEXT1+1
          GO TO 340
          105 330 NE=1+NDW+1
          340 AN=NE-1
          PT2-0051
          PT2-0052
          PT2-0053
          PT2-0054
          PT2-0055
          PT2-0056
          PT2-0057
          PT2-0058
          PT2-0059
          PT2-0060
          PT2-0061
          PT2-0062
          PT2-0063
          PT2-0064
          PT2-0065
          PT2-0066
          PT2-0067
          PT2-0068
          PT2-0069
          PT2-0070
          PT2-0071
          PT2-0072
          PT2-0073
          PT2-0074
          PT2-0075
          PT2-0076
          PT2-0077
          PT2-0078
          PT2-0079
          PT2-0082
          PT2-0083
          PT2-0084
          PT2-0085
          PT2-0086
          PT2-0087
          PT2-0088
          PT2-0089
          PT2-0090
          PT2-0091
          PT2-0092
          PT2-0093
          PT2-0094
          PT2-0095
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	CMSTH=BTNP*(AN1-1.)*C-	PT2=0096
	VC=CMSTP	PT2=0097
	DO 350 K=1,IMRO	PT2=0098
110	NEXNEXT1	PT2=0099
	HRO(KA)=HRO(KA,NA=1)*(AN1-1.)*CRO(KA)	PT2=0100
	IMOUNHNTIME/NFUEANFU	PT2=0101
	360 IF(NTIME-TSTR)430,380,370	PT2=0102
	370 IF(NTIME-TND)380,410,430	PT2=0103
115	380 IF(NE=3-NORT)400,390,390	PT2=0104
	390 NEAUE(DIST(KC+1)-DIST(KC))/XNORT	PT2=0105
	KC=KC+1	PT2=0106
	NEA3=1	PT2=0107
	GO TO 420	PT2=0108
120	400 NEA3=NEA3+1	PT2=0109
	GO TO 420	PT2=0110
	410 NEAUE=0	PT2=0111
	KC=KC+1	PT2=0112
	420 CONTINUE	PT2=0113
125	AN3=NE-1	PT2=0114
	AN4=NE-1	PT2=0115
	R=(R+(DIST(KC+1)-(AN3-1.)*AN4))/XNORT	PT2=0116
	RAIN=0	PT2=0117
	GO TO 440	PT2=0118
130	430 RAIN=0,0	PT2=0119
	440 CONTINUE	PT2=0120
	END OF RAIN AND RO VALUES	
	C	
	C	
	START OF WIND COMPUTATIONS	
135	500 IF (JWIND=NFU)800,800,510	PT2=0121
	510 CONTINUE	PT2=0122
	560 IF (IFIRST)600,570,570	PT2=0123
	570 DO 580 I=1,IM	PT2=0124
	MG1(I)=MG2(I)	PT2=0125
140	DO 590 J=1,JM	PT2=0126
	X1(I,J)=XP(I,J)	PT2=0127
	Y1(I,J)=YP(I,J)	PT2=0128
	DO 590 I=1,2,4	PT2=0129
	MB1(I)=MB2(I)	PT2=0130
145	NTIME=NTIME+1	PT2=0131
	ITIME=ITIME+1	PT2=0132
	DO 610 I=1,IMAX	PT2=0133
	MGR(I)=MGR(I,IT)	PT2=0134
	IF(MGR(I,LY,0)) GO TO 610	PT2=0135
150	DO 620 J=1,LMAX	PT2=0136
	XW(I,J)=XW(I,J,IT)	PT2=0137
	YW(I,J)=YW(I,J,IT)	PT2=0138
	620 CONTINUE	PT2=0139
	DO 630 J=1,4	PT2=0140
155	MRR(J)=MRR(J,IT)	PT2=0141
	J=IND=1	PT2=0142
	IS=1	PT2=0143
	DO 710 L=1,LMAX	PT2=0144
	JC=1+(L-1)	PT2=0145

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160      DO 660 K=1,MM
          I1=1+(K7*(K-1))
          I2=I1+K7K
          DXR=(XR(K+1+L)-XR(K+L))/AI
          DYR=(YR(K+1+L)-YR(K+L))/AJ
          GO TO (A50,A60),IS
165      650 DMH=(HGR(K+1)-HGR(K))/AI
          660 DO 680 IC=1,I2
              DFU=IC-I1
              Y2(IC+JC)=YR(K+L)+(DYR*(DFU+.5))
              X2(IC+JC)=XR(K+L)+(DXR*(DFU))
              GO TO (A70,A80),IS
          670 HG2(IC)=HGR(K)+DMH*(DFU+.5)
          680 CONTINUE
          DO 690 IRT=1,IR
175      690 HB2(IRT)=HBR(IRT)
          GO TO (700,710),IS
          700 IS=2
          710 CONTINUE
          DO 740 I=1,IM
              DO 730 L=1,LM
180              J1=1+(L7*(L-1))
              J2=J1+L7K
              J1K=J1+KZ
              J1L=J1+L7
              DXR=(X2(I,J1K)-X2(I,J1))/AI
              DYR=(Y2(I,J1L)-Y2(I,J1))/AJ
              DO 720 J=J1,J2
                  DFU=J-J1
                  X2(I,J)=X2(I,J1)+DXR*(DFU+.5)
                  Y2(I,J)=Y2(I,J1)+DYR*(DFU)
190          720 CONTINUE
              730 CONTINUE
              740 CONTINUE
              IF (IFIRST)750,800,800
195      750 IFIRST=1
              GO TO 570
              800 CONTINUE
              810 ANUP=NFU
                  =IND=J-IND
                  DFU=(IND-1)/ANUP
                  DFU=DFU*(1./ANUP)
                  DO 820 K=1,IR
200          820 HB(K)=H1(K)+DFU*(HB2(K)-H1(K))
                  HG(IM)=HG1(IM)+DFU*(HG2(IM)-HG1(IM))
205      C
          C      S-EEP WHOLE FIELD FOR FLOW FROM BLOCKS
          C
          830 DO 2010 J=1,JM
              THIS BRANCH SKIPS THE INVESTIGATION OF POSSIBLE BARRIERS FOR THE
              ROW J=1. FOR J=1 THE INDICATOR LG=3 IS SET. IF J IS GREATER THAN
              1 A SEARCH FOR BARRIERS IN THE ROW WILL TAKE PLACE.
210      C
          840 KJ = 0

```

PT2=0146
PT2=0147
PT2=0148
PT2=0149
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PT2=0189
PT2=0190

PT2=0191

PT2=0192

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      L=0
      C A NORMAL COMPUTATION SEQUENCE WILL OCCUR. THE FIRST X-DIR FLUX
215 C TEMPORARY STORAGE IS SET AS THAT OF THE FIRST COLUMN.
      C THE NUMBER AND LOCATIONS OF THE BARRIERS PRESENT IN THE ROW ARE
      C FOUND AND PLACED IN TEMPORARY STORAGE. IF NO BARRIERS ARE PRESENT
      C THE INDICATOR KJ REMAINS ZERO.
      IF (KM.EQ.0) GO TO 870
220 C LO 800 KM1,KM
      IF (J=JR(K)) 860,850,860
      850 KJ = KJ + 1
      LEL+1
      KM(L)=K
225 C
      860 CONTINUE
      C BASED ON KJ, THE INDEX LJ IS SET TO INDICATE THE BARRIER SITUATION
      C IN THE MATN COMPUTATION, LJ=1 FOR NO BARRIERS.
      IF (KJ) 870,870,880
230 C 870 LJ=1
      GO TO 890
      880 LJ=2
      C
      C THIS IS THE PRIMARY LOOP FOR STEPPING THRU THE IM GRID COLUMNS.
235 C 890 DO 2000 J=1,IMM
      BEGIN THE EXAMINATION OF THE BASIC TRIAD OF GRID SQUARES. THE
      C DUMMY VARIABLES M1,D1,M2,D2 AND Q ARE USED TO ALLOW ONE ROUTINE TO
      C BE EMPLOYED FOR BOTH SETS OF SQUARES. SQUARES ONE AND TWO ARE
      C TAKEN FIRST.
240 C 900 IF (J=1) 910,910,920
      910 MG(I)=MG(I)+DFU*(MG2(I)-MG1(I))
      920 X(I)=S*(X1(I,J)+DFU*(X2(I,J)-X1(I,J)))
      Y(I)=S*(Y1(I,J)+DFU*(Y2(I,J)-Y1(I,J)))
      M1 = M(I,J)
      Z = IZ(I,J)
      D1 = M1-Z
245 C
      C THIS BRANCH WILL SET UP A SEARCH FOR A BARRIER IN THE SQUARES
      C BEING CONSIDERED IF LJ=2. IF LJ=1 OR THE BARRIER EXISTS BETWEEN
      C THE OTHER PAIR OF SQUARES AN INDEX IS SET, LI=1, FOR A BARRIER.
250 C LI=2.
      1000 GO TO (1040,1010),LJ
      C THIS X LOOP SEARCHES FOR A BARRIER IN THE PAIR OF SQUARES.
      1010 DO 1030 K=1,KJ
      K=K(K)
255 C IF (I=IR(K)) 1030,1020,1030
      1020 LI=2
      GO TO 1050
      C
      1030 CONTINUE
      1040 LI=1
260 C
      C THE DUMMY VARIABLES M2 AND D2 ARE SET FOR THE SQUARE ONE AND TWO
      C CALCULATION. THIS IS INDICATED BY LI=1.
      1050 CONTINUE
      1053 M2 = M(I+1,J)
265 C 1054 Z = IZ(I+1,J)

```

PT2-0193

PT2-0194

PT2-0195

PT2-0196

PT2-0197

PT2-0198

PT2-0199

PT2-0200

PT2-0201

PT2-0202

PT2-0203

PT2-0204

PT2-0205

PT2-0206

PT2-0207

PT2-0208

PT2-0209

PT2-0210

PT2-0211

PT2-0212

PT2-0213

PT2-0214

PT2-0215

PT2-0216

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PT2-0218

PT2-0219

PT2-0220

PT2-0221

PT2-0222

PT2-0223

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      D2 = M2 -7
      LG = 1
      PT2=0224
      PT2=0225
270  C
      C THE INVESTIGATION OF THE RELATION BETWEEN DATUMS OF BOTH PAIRS OF
      C SQUARES BEGINS HERE. THIS BRANCH TESTS LI FOR A BARRIER.
      1060 GO TO (1110,1070),LI
      C A BARRIER EXISTS AND ON THE BASIS OF LG THE DATUM IS ASSIGNED THE
      C PROPER BARRIER HEIGHT.
      1070 GO TO (1080,1090),LG
275  1080 ZB = IZ(XI)
      CDOI = ICDOX(XI)
      CDSI = ICOSX(XI)
      GO TO 1100
      1090 ZB = IZ(YI)
      CDOI = ICDOY(XI)
      CDSI = ICOSY(XI)
280  1100 ZR = ZB * .001
      CDOI = CDOI * .001
      CDSI = CDSI * .001
      GO TO 1140
285  C NO BARRIER EXISTS. THE RELATIVE DATUM HEIGHTS OF THE SQUARES ARE
      C TESTED AND THE HIGHER DATUM SET EQUAL TO ZB.
      1110 CDOI = CDSI
      IF (M1-D1-M2+D2)1120,1130,1130
290  1120 ZR=(M2-D2)
      GO TO 1140
      1130 ZR = M1 - D1
      C THE INVESTIGATION OF THE DEPTH SIGNATURES BEGINS AT THIS POINT.
      C THE PROPER ASSIGNMENT IS MADE FOR THE FLUX CALCULATION.
295  C
      1140 IF (D1)1150,1160,1190
      1150 LM=1
      GO TO 1170
      C
300  1160 LM=2
      1170 IF (D2)1160,1160,1180
      1180 IF (M2-ZR)1130,1160,1260
      1190 IF (D2)1200,1210,1230
      1200 LM=1
      GO TO 1220
305  1210 LM=2
      1220 IF (M1-ZR)1160,1160,1270
      1230 IF (M1-ZR)1160,1160,1240
      1240 IF (M2-ZR)1250,1250,1240
310  1250 LM=2
      GO TO 1270
      1260 DM=ZB-M2
      DP=ABS(DM)
      TAD = 4.6 D2
      GO TO (1290,1350),LM
315  1270 DM=M1-ZR
      DP=ABS(DM)
      PT2=0239
      PT2=0240
      PT2=0241
      PT2=0242
      PT2=0243
      PT2=0244
      PT2=0245
      PT2=0246
      PT2=0247
      PT2=0248
      PT2=0249
      PT2=0250
      PT2=0251
      PT2=0252
      PT2=0253
      PT2=0254
      PT2=0255
      PT2=0256
      PT2=0257
      PT2=0258
      PT2=0259
      PT2=0260
      PT2=0261
      PT2=0262
      PT2=0263
      PT2=0264

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```
320      TAD = 4.0 D1
      GO TO (1300,1350)+LM
1280      GO TO (1400,1330)+LI
1290      M(I,J)=M1-D1
      GO TO 1350
325      GO TO (1310,1320)+LQ
1310      M(I+1,J) = M2 = D2
      GO TO 1350
1320      M(I+J+1) = M2 = D2
      GO TO 1350
330      IF (ZB=(M1-D1)) 1400,1400,1340
1340      IF (ZB=(M2-D2)) 1400,1400,1370
1350      IF (CP.LT.C.OOOO01) GO TO 1360
      DRE = (C001 * D2) * (C001 * D1)
      GO TO 1380
335      GO TO 1370
1370      DMEH1=M2
      TAD = D1+D2
      DB = (((M1+M2)/2.) - ZB) * CDSI
      DRE = DB+DB
340      GO TO (1390,1400)+LQ
1390      Q = U(I+1,J)
      PUSH = V(I) * DELT
      GO TO 1450
1400      Q = V(I+J+1)
345      PUSH = V(I) * DELT
      C
      SPECIAL CALCULATION OF Q FOR BARRIERS
      C
1450      GDS=GRAV*PREH
      RG=GDS/(C2*TAD)
350      FORCE=RG*(Q+PUSH)+GDS*DM
      HRG=4G/2.
      G = SORT(ABS(FORCE)+HRG**2) = HRG
      IF (FORCE.LT.0.) G = -G
      GO TO 1570
355      C
1460      GO TO (1470,1490)+LQ
1470      Q = U(I+1,J)
1480      B1 = V(I,J) + V(I+1,J) + V(I+J+1) + V(I+1+J+1)
      PUSH = V(I) * DELT
360      GO TO 1510
1490      Q = V(I+J+1)
1500      B1 = U(I,J) + U(I+1,J) + U(I+J+1) + U(I+1+J+1)
1505      PUSH=V(I)*DELT
1510      A1 = Q * G
365      H = SQRTF ((A1 * A1) + (B1 * B1))
1520      G = 1. + ((C1 * H) / ((D1+D2)*(D1+D2)))
      TAD = D1+D2
      DM = M1-M2
1540      IF (PUSH-1541,1542,1542
1541      IF (D2=0.0) 1560,1560,1545
370      1545      IF (D2=0.0) 1540,1544,1560
      PT2=0265
      PT2=0266
      PT2=0267
      PT2=0268
      PT2=0269
      PT2=0270
      PT2=0271
      PT2=0272
      PT2=0273
      PT2=0274
      PT2=0275
      PT2=0276
      PT2=0277
      PT2=0278
      PT2=0279
      PT2=0280
      PT2=0281
      PT2=0282
      PT2=0283
      PT2=0284
      PT2=0285
      PT2=0286
      PT2=0287
      PT2=0288
      PT2=0289
      PT2=0290
      PT2=0291
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      PT2=0295
      PT2=0296
      PT2=0297
      PT2=0298
      PT2=0299
      PT2=0300
      PT2=0301
      PT2=0302
      PT2=0303
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      PT2=0306
      PT2=0307
      PT2=0308
      PT2=0309
      PT2=0310
      PT2=0311
      PT2=0312
      PT2=0313
      PT2=0314
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1542 IF(01=0.0)1560,1560,1543
1543 IF(01=0.1)1544,1544,1560
1544 PUSH=0.0
375 G=(G-1.1*.07+1.
C
C STANDARD CALCULATION OF G FOR BLOCKS
1560 G=(1.0/G)*(2+(C2*TAD*U)+PUSH)
C
380 C THE H AND D CALCULATIONS ARE MADE ON THE BASIS OF THE INDEX LG. IF
C LG#1 THE CALCULATIONS ARE POSTPONED AND A RETURN TO THE POINT OF
C INVESTIGATION OF THE DATUM RELATIONSHIPS IS MADE (STATEMENT 21)
C AFTER THE DUMMY VARIABLES H2 AND D2 ARE SET UP FOR THE ONE-TREE
C SQUARES.
385 1570 G01=D1/C3
G02=D2/C3
IF(ABS(D1).LT.1.E-10) G=0.0
GO TO (1571,1681),LG
C
390 1571 IF(0)1572,1577,1573
1577 H=H+1
GO TO 1580
1572 H=H+1
IF(H=2F)1576,1575,1575
395 1576 G=0.0
GO TO 1580
1575 IF(G02=0)1574,1574,1580
1574 G=H02
GO TO 1580
400 1573 H=H+1
IF(H=2F)1576,1580,1580
1580 IF(I=1)1590,1590,1630
1590 IF(J=JRL)1610,1610,1620
C LEFT HAND SEAWARD BOUNDARY CONDITION
405 1610 H=(1+J)*H0(1)
1620 D=H0
GO TO 1670
C
1630 IF(J=JRR)1640,1640,1670
410 1640 IF(I=IMM)1670,1650,1670
C RIGHT HAND SEAWARD BOUNDARY CONDITION
1650 H=(IMM,J)*H0(IMM)
GO TO 1680
1670 UNITS
415 1680 H2 = H(I,J+1)
Z = IZ(I,J+1)
D2 = H2 - Z
LG = 2
2892 GO TO 1660
420 C
1681 IF(0)1671,1674,1673
1674 IF(ML=1)1690,1682,1689
1671 IF(H=2F)1672,1682,1682
1672 G=0.0

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PT2-0315
PT2-0316
PT2-0317
PT2-0318

PT2-0319

PT2-0320
PT2-0321
PT2-0322
PT2-0323

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PT2-0329
PT2-0330
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PT2-0332
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PT2-0340

PT2-0341
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PT2-0345
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PT2-0348
PT2-0349
PT2-0350

PT2-0351
PT2-0352
PT2-0353
PT2-0354

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SUBROUTINE PART2

74/74 CPT=2

FTN 4.0+420

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425      1682 IF(ML-1)1684,1684,1685      PT2=0355
      1685 IF(Q01-UN1)1686,1686,1684      PT2=0356
      1686 UN1=Q01                        PT2=0357
      1687 IF(Q02-Q)1687,1687,1690      PT2=0358
      1687 Q=Q02                          PT2=0359
430      GO TO 1690                        PT2=0360
      1673 IF(M1-ZR)1672,1683,1683      PT2=0361
      1683 IF(ML-1)1683,1684,1689      PT2=0362
      1688 IF(Q01-Q)1692,1692,1690      PT2=0363
      1692 Q=Q01                          PT2=0364
435      GO TO 1690                        PT2=0365
      1689 IF(Q+UN1-Q01)1690,1690,1691  PT2=0366
      1691 ADDQ = Q+UN1 + 0.00001        PT2=0367
      Q = (Q/(ADDQ))*Q01                 PT2=0368
      UN1 = (UN1/(ADDQ))*Q01             PT2=0369
440      VN1 = Q                          PT2=0370
      U(I,J)=UN1                         PT2=0371
      UN = UN1                           PT2=0372
      V(I,J) = VN(I)                     PT2=0373
      VN(I) = VN1                         PT2=0374
445      2000 CONTINUE                    PT2=0375
      U(IM,J)=UN1                        PT2=0376
      2010 CONTINUE                       PT2=0377
      IF(KCM,GT,0) CALL CHANL(2)         PT2=0378
C
450      C SHEEP WHOLE FIELD FOR M ON BLOCKS
C
      SUM=0.                             PT2=0379
      COUNT=0.                           PT2=0380
      DO 2020 J=1,JMM                     PT2=0381
      DO 1790 I=1,IMM                     PT2=0382
      Z=IZ(I,J)                           PT2=0383
      D1=M(I,J)-Z                         PT2=0384
      IF(J=1)1700,1700,1710               PT2=0385
460      1700 M(I,1) = MG(I)              PT2=0386
      1710 IF(D1)1740,1720,1720            PT2=0387
      1720 IF(J=1)1790,1790,1721           PT2=0388
      1721 IF(I=1)1722,1722,1723           PT2=0389
      1722 IF(J=JRL)1790,1790,1724         PT2=0390
      1723 IF(I=IMM)1729,1724,1724        PT2=0391
      1724 IF(J=JRL)1790,1790,1729        PT2=0392
465      1729 SETUP=C3*(U(I,J)-U(I+1,J)+V(I,J)-V(I+1,J))
      M(I,J)= M(I,J) + SETUP + RAIN      PT2=0393
      SUM=SUM+ABS(M(I,J))                 PT2=0394
      COUNT=COUNT+1                      PT2=0395
470      IF ( D1 + SETUP + RAIN) 1740,1740,1750
      1740 M(I,J) = TZ(I,J)               PT2=0396
C
      1750 IF(KCM,GT,0) GO TO 1790        PT2=0399
C
475      C ENTER RUNOFF VALUES ON ENTRY BLOCKS ONLY IF CHANNELS NOT PROVIDED
      DO 1770 J=1,JMM                     PT2=0400
      IF (LROJ(TJM)-J)1770,1760,1770     PT2=0401
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SUBROUTINE PART2

74/74 OPT=2

FTN 4.6+420

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1760 IF (LRO(I,J,K)=3) 1770,1780,1790
1770 CONTINUE
480 GO TO 1790
1780 H(I,J,K) = H(I,J,K) + HRO(I,J,K)*DELX/(DELX**2)
1790 CONTINUE
2020 CONTINUE
      IF (KCH.GT.0) CALL CHANL(3)
485 C
C     THE TIME INDICES ARE STEPPED TO THE NEW LEVEL.
      NT = NT + 1
      NTIME = NTIME + 1
      J=IND=JIND+1
490 IF (SUM/COUNT=100.) 2075,2140,2140
C     TEST THE STABILITY OF THE COMPUTATIONS VIA AVE ABS(H).
C     COMPUTATIONS ARE STABLE. CALCULATIONS CONTINUE.
C     COMPUTATIONS ARE UNSTABLE. AN ON-LINE MESSAGE IS PRINTED
495 C     TEST NT FOR THE OUTPUT OF U,V,W,D,X,Y FIELDS.
      TIM=NTIME-JNTIME
      ITH=TIM
      HINT=INTER
      IF ((TIM/HINT)-(ITH/I-TER) 2055,2055,2090
500 2055 CALL SAVE(2)
      2090 IF (NT=100) 2110,2105,2100
C     OUTPUT U,V,W,D,X,Y FIELDS. RESET NT=0 AND STEP NN.
      2105 IF (INFLD.EQ.0) GO TO 2110
505 CALL CHANL(4)
      GO TO 2110
      2100 NT = 0
      NN = NN + 1
      HOUR = NTIME/HF
      CALL CHANL(4)
510 2110 CONTINUE
C
      2130 IF (NN=NTIME) 2160,2160,200
C     STORM COMPLETED. FINAL OUTPUT ON TAPES.
      2140 PRINT 2150,NN
515 2150 FORMAT (21H STOP AS AT NTIME = ,I4)
      STOP
C
      2160 CALL SAVE(3)
      CALL CONTIN(2)
520 RETURN
      END

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PT2=0402
PT2=0403
PT2=0404
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PT2=0432

PT2=0433
PT2=0434
PT2=0440
PT2=0441

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1      C
2      C
3      C      SUBROUTINE CHANL(N)                                CHNL0001
4
5      COMMON/RLK1/ IAC(100),JBC(100),IZC(100),IZY(100),ICDCK(100)    CHNL0004
6      1,ICDCK(100),ICDCK(100),ICDCK(100),LROI(4),LROJ(4),DIST(24)    CHNL0005
7      2,CHST(30),RO(A,30),MGP(8),XR(8,6),YR(8,6),MRH(8)              CHNL0006
8      COMMON/RLK2/ IZ(28,20),UC(28,20),V(28,20),H(28,20),NTIME    CHNL0007
9      COMMON/RLK3/ NM,MMIN,MMAX,NFU,INFLD,IM,JH,KK,MMAX,LMAX,DELT    CHNL0008
10     1,COO,FK,HG1,INUT,KI,LJ,KII,LJJ,JBL,JBR,KMM,LMM,RF,CONST,S    CHNL0009
11     2,IPRO,JMRO,KR,JSTR,IND,NON,KIM,NORT,NTIME,INTIME,NONIND,GRAV    CHNL0010
12     3,KCNP,DFU,INTER                                                CHNL0011
13     COMMON/RLK4/ HQT(8),CRD(8),KB(24),X1(28,21),Y1(28,21),X2(28,21)    CHNL0012
14     1,Y2(28,21),X(28),Y(28),HGI(28),MG2(28),MB1(4),MH(9),VV(28)    CHNL0013
15     2,MG(28),MR2(8)                                                CHNL0014
16     COMMON/RLK5/ ICG(130),JCG(130),IACX(130),IACY(130),IZCX(130)    CHNL0015
17     1,IZCY(130),QCYP(130),QCKN(130),QCYP(130),QCVN(130),HCF(130),HP(130)    CHNL0016
18     2,KCH,KCY(130),KCY(130),KCH(130),UCT(130),UCF(130),KRI(130),IRON    CHNL0017
19     3,KEN(2,130),VCT(130),VCF(130),ACGX(130),AGUY(130),KXCP(130)    CHNL0018
20     4,KCVP(130),KLB(90),KLM,IFC(130),FC                            CHNL0019
21     COMMON/RLK7/ IEND,NF,IBL,NJ,2(40)                                CHNL0020
22     COMMON/RLK9/ KZ,LZ,NUM0,C1,C2,C3,IMH,JMH,LY,NF,NEXT1,IT,KC,IFIRST    CHNL0021
23     1,J=IND,NEX1,XND,NF=3,XNORT, C4,HAIN,AJ,AI,LJK,KIK              CHNL0022
24     EQUIVALENCE (O1,DAC)                                            CHNL0023
25     C
26     ABSF(X) = ABS(X)                                                CHNL0024
27     SQRTR(X) = SQRT(X)                                              CHNL0025
28     C
29     GO TO (1000,2000,3000,4000),N                                  CHNL0026
30     C
31     CHANNEL CODE 1 IS FOR READING CHANNEL DATA AND ESTABLISHING KEY ARRAYS
32     CHANNEL CODE 2 IS FOR FLOW AND HEIGHT CALCULATIONS IN CHANNELS
33     CHANNEL CODE 3 IS FOR CALCULATION OF H ON BLOCKS CONTAINING CHANNELS
34     CHANNEL CODE 4 IS FOR LISTING OF CHANNEL OUTPUT
35     C
36     ENTRY POINT 1 FOR READING CHANNEL DATA, INITIALIZATION AND FOR
37     ESTABLISHING KEY ARRAYS FOR ROUTINE CALCULATIONS
38     C
39     1000 PRINT 500                                                    CHNL0027
40     PRINT 500                                                        CHNL0028
41     500 FORMAT(1 THE FOLLOWING ARE SUBGRID CHANNEL DATA= Z VALUES IN FEET)    CHNL0029
42     1ET(1/2)                                                        CHNL0030
43     C=(DELY**2)/DELT                                                CHNL0031
44     C
45     A NEGATIVE I-CX OR I-ACY IDENTIFIES THOSE CHANNELS WITH BARRIERS OF
46     EQUAL ELEVATION ON BOTH SIDES SUCH AS A JETTY SYSTEM
47     FOR SINGLE BARRIERS, THE LATTER IS TAKEN ON THE INNER SIDE OF THE
48     CHANNEL BLOCK IF IZC IS NEGATIVE, WHILE ON THE OUTER SIDE IF IZC IS
49     POSITIVE
50     DO K=1,KCH
51     READ 501, IENT, ICG(K),JCG(K),IACX(K),IZCX(K),IACY(K),IZCY(K)    CHNL0032
52     1,IFC(K)                                                         CHNL0033
53     IF(IFC(K),EQ,0) IFC(K)=FC*10000                                CHNL0035

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```

55      501 FORMAT(11,2X,15,9(3X,15))
        IF(IIDENT.NE.8) GO TO 510
        50 CONTINUE
        DO 100 K=1,KCM
          KEN(1,K)=0
          KEN(2,K)=0
          K9I(K)=0
          KCX(K) = 0
          KCY(K) = 0
          KCXP(K) = 0
          KCYP(K) = 0
          65      I = ICG(K)
                  J = JCG(K)
                  DO 80 L=1,KCM
                    IF(ICG(L).EQ.(I+1).AND.JCG(L).EQ.J) KCYP(K) = L
                    IF(ICG(L).EQ.I.AND.JCG(L).EQ.(J+1)) KCXP(K) = L
                    70      IF(ICG(L).EQ.(I-1).AND.JCG(L).EQ.J) KCY(K) = L
                            IF(ICG(L).EQ.I.AND.JCG(L).EQ.(J-1)) KCX(K) = L
                  80 CONTINUE
                  KCB(K) = 0
                  IF(KM.EQ.0) GO TO 91
                  75      DO 90 L=1,KM
                            IF(IB(L).EQ.I.AND.JB(L).EQ.J) KCB(K) = L
                  90 CONTINUE
                  91 CONTINUE
                  UCT(K) = 0.0
                  80      UCF(K) = 0.0
                            VCT(K) = 0.0
                            VCF(K) = 0.0
                            MP(K) = M(I,J)
                  85      PRINT 502, K, ICG(K), JCG(K), I-CX(K), IZCX(K), I-CY(K), IZCY(K), IFC(K)
                            502 FORMAT('  K=1,15,1 ICG=1,13,1 JCG=1,13,1 I-CY=1,15,1 IZCX=1,14,1 I-CX=1,15,1 IZCY=1,14,1 IFC=1,14,1')
                  100 CONTINUE
C
C      ARRAY KLB IDENTIFIES BARRIER BLOCKS WHICH ARE NOT COMMON
C      WITH CHANNEL BLOCKS
        90      LC= 0
                DO 105 K=1,KM
                  I=IB(K)
                  J=JB(K)
                  95      DO 102 L=1,KCM
                            IF(ICG(L).EQ.I.AND.JCG(L).EQ.J) GO TO 105
                  102 CONTINUE
                  LC=LC+1
                  KLB(LC)=K
                  100      105 CONTINUE
                  KLM=LC
C
C      THE FOLLOWING CREATES A SPECIAL INDEX FOR CHANNEL STARTING AND END
C      ANY BLOCK WITH NEGATIVE IRC OR JUC IDENTIFIES A CHANNEL END POINT
105      C      ARRAY KEN IDENTIFIES WHAT TYPE OF END POINT EXISTS ACCORDING TO THE
C      1 KCX M      5 KCA Q

```

CHNL0036
 CHNL0037
 CHNL0038
 CHNL0039
 CHNL0040
 CHNL0041
 CHNL0042
 CHNL0043
 CHNL0044
 CHNL0045
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 CHNL0048
 CHNL0049
 CHNL0050
 CHNL0051
 CHNL0052
 CHNL0053
 CHNL0054
 CHNL0055
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 CHNL0057
 CHNL0058
 CHNL0059
 CHNL0060
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 CHNL0065
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 CHNL0075
 CHNL0076
 CHNL0077
 CHNL0078
 CHNL0079
 CHNL0080

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	IF (XJ,LE,4) GO TO 460	CHNL0173
	LQ=LR+1	CHNL0174
215	LR=0	CHNL0175
	DO 450 L=1,IMRO	CHNL0176
	IF (LRO(I),EQ,I,AND,LROJ(L),EQ,J) LR=L	CHNL0177
	450 CONTINUE	CHNL0178
	KRI(K)=LR	CHNL0179
220	IF (LR,GT,0) LQ=LR+1	CHNL0180
	460 IF (JCG(K),GT,0) GO TO 300	CHNL0181
	IF (LS,EQ,2) GO TO 300	CHNL0182
	LS=2	CHNL0183
	GO TO 410	CHNL0184
225	300 CONTINUE	CHNL0185
	IF (LRH,EQ,IMRO) GO TO 480	CHNL0186
	C	
	PRINT 470, LRH,IMRO	CHNL0187
230	470 FORMAT(1,/,//)*****WARNING***** ONLY(1,13,	CHNL0188
	1 CHANNEL END POINTS(//) MATCH THE(1,13,(RTVER INPUT POSITIONS(CHNL0189
	2,/,//)*****WARNING*****ONLY(1,13,)	CHNL0190
	C	
	480 CONTINUE	CHNL0191
	PRINT 549	CHNL0192
235	549 FORMAT(1,1)	CHNL0193
	DO 600 K=1,KCM	CHNL0194
	PRINT 550, K,KCX(K),KCY(K),KCP(K),KCB(K),ICG(K),JCG(K)	CHNL0195
	1,KEN(1,K),KEN(2,K),KRI(K)	CHNL0196
	550 FORMAT(1, K=1,13,(KCX=1,13,(KCY=1,13,(KCP=1,13,	CHNL0197
240	1 KCB=1,13,(ICG=1,13,(JCG=1,13,(KEN=1,13,(KEN2=1,13	CHNL0198
	2,(KRI=1,13)	CHNL0199
	600 CONTINUE	CHNL0200
	PRINT 551, KCM	CHNL0201
	551 FORMAT(1,1,/,//)10X,(KCM=1,15,/,//)	CHNL0202
245	C	
	RETURN	CHNL0203
	510 PRINT 503	CHNL0204
	503 STOP BECAUSE CARDS WITH IDENT = 8 EXPECTED(1,/,//)	CHNL0205
	PRINT 504, IDENT	CHNL0206
250	504 FORMAT(3X,(IDENT=1,14)	CHNL0207
	STOP	CHNL0208
	C	
	C ENTRY POINT 2 FOR FLOW AND HEIGHT CALCULATIONS IN CHANNELS	
	C	
255	2000 SOT = SDELT	CHNL0209
	DO 2500 K=1,KCM	CHNL0210
	I= ICG(K)	CHNL0211
	I= IABS(I)	CHNL0212
260	J= JCG(K)	CHNL0213
	J= IABS(J)	CHNL0214
	PUSHU = SOT*(X1(I,J)+DFU*(X2(I,J)-X1(I,J)))	CHNL0215
	PUSHV = SOT*(Y1(I,J)+DFU*(Y2(I,J)-Y1(I,J)))	CHNL0216
	M1 = H(I,J)	CHNL0217
	Z1 = IZ(I,J)	CHNL0218
265	MCI = MC(K)	CHNL0219

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	C	2	KCY	M	6	KCY	Q	
	C	3	KCXP	M	7	KCXP	Q	
	C	4	KCYP	M	8	KCYP	Q	
110	C							
		I80=0					CHNL0081	
		DO 200 K=1,KCH					CHNL0082	
		I=ICG(K)					CHNL0083	
		J=JCG(K)					CHNL0084	
115	C							
		IF(KCX(K).NE.0) GO TO 110					CHNL0085	
		IF(INCX(K).EQ.0) GO TO 110					CHNL0086	
		I80=I80+1					CHNL0087	
		K8=KCY+I80					CHNL0088	
120		KCY(K)=K8					CHNL0089	
		ICG(K)=I					CHNL0090	
		KEN(1,K)=1					CHNL0091	
		IF(J.EQ.1) GO TO 110					CHNL0092	
		Z=IZ(I,J=1)					CHNL0093	
125		IF((M(I,J=1)=Z).LE.0) KEN(1,K)=5					CHNL0094	
	C							
		110 IF(KCY(K).NE.0) GO TO 120					CHNL0095	
		IF(INCY(K).EQ.0) GO TO 120					CHNL0096	
		I80=I80+1					CHNL0097	
130		K8=KCY+I80					CHNL0098	
		KCY(K)=K8					CHNL0099	
		IF(ICG(K).LT.0) JCG(K)=J					CHNL0100	
		ICG(K)=I					CHNL0101	
		L=1					CHNL0102	
135		IF(JCG(K).LT.0) L=2					CHNL0103	
		KEN(L,K)=2					CHNL0104	
		IF(I.EQ.1.AND.J.LE.JBL) GO TO 120					CHNL0105	
		KEN(L,K)=6					CHNL0106	
		IF(I.EQ.1) GO TO 120					CHNL0107	
140		Z=IZ(I=1,J)					CHNL0108	
		IF((M(I=1,J)=Z).GT.0) KEN(L,K)=2					CHNL0109	
	C							
		120 K8=KCXP(K)					CHNL0110	
		K8=KCYP(K)					CHNL0111	
145		IF(INCY(K).NE.0) GO TO 130					CHNL0112	
		IF(KY.EQ.0) GO TO 121					CHNL0113	
		IF(INCY(KY).NE.0) GO TO 130					CHNL0114	
		121 IF(KK.EQ.0) GO TO 125					CHNL0115	
		IF(INCX(KY).NE.0) GO TO 130					CHNL0116	
150		I80=I80+1					CHNL0117	
		K8=KCY+I80					CHNL0118	
		125 IF(ICG(K).LT.0) JCG(K)=J					CHNL0119	
		ICG(K)=I					CHNL0120	
		L=1					CHNL0121	
155		IF(JCG(K).LT.0) L=2					CHNL0122	
		KEN(L,K)=7					CHNL0123	
		IF(J.EQ.JMM) GO TO 130					CHNL0124	
		Z=IZ(I,J=1)					CHNL0125	
		IF((M(I,J=1)=Z).GT.0) KEN(L,K)=3					CHNL0126	

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160      C
      130 IF(I-CH(K),NE,0) GO TO 200
      IF(KX,EQ,0) GO TO 131
      IF(I-CH(KX),NE,0) GO TO 200
      131 IF(KY,EQ,0) GO TO 135
      IF(I-CY(KY),NE,0) GO TO 200
165      IBO=IBO+1
      KCP(K)=KCY+IRO
      135 IF(ICG(K),LT,0) JCG(K)=-J
      ICG(K)=-I
170      L=1
      IF(JCG(K),LT,0) L=2
      KEN(L,K)=#
      IF(I.GE.IXX,AND,J.GT.JXR) GO TO 200
      KEN(L,K)=#
175      IF(I.GE.IYY) GO TO 200
      Z=IZ(I+1,J)
      IF((I+1,J)=Z),LE,0) KEN(L,K)=#
      200 CONTINUE
      C
180      C      IBO# IS THE TOTAL NUMBER OF CHANNEL END POINTS OF ANY KIND
      IBO# = IBO
      KCP# = KCM + IBO# + 1
      DO 210 K=1,KCMP
      KC(K)=MGI
185      GCXP(K) = 0.
      GCYP(K) = 0.
      GCXN(K) = 0.
      GCYN(K) = 0.
      AOGX(K) = 0.
      AOGY(K) = 0.
190      210 CONTINUE
      C
      C      APRAY KRI IDENTIFIES THE LOCATIONS OF RIVER INPUT FOR Q TYPE END POINTS
195      LR#0
      LQ#0
      DO 300 K=1,KCM
      I= ICG(K)
      J= IARS(I)
      200      J= JCG(K)
      J= JARS(J)
      ITOP=KCP
      IF(KCX(K),EQ,0) KCX(K)=ITOP
      IF(KCY(K),EQ,0) KCY(K)=ITOP
      205      IF(KCXP(K),EQ,0) KCXP(K)=ITOP
      IF(KCYP(K),EQ,0) KCYP(K)=ITOP
      C
      KRI(K)= 0
      IF(I=RO,EQ,0) GO TO 480
      LS#1
      210      IF(ICG(K),GT,0) GO TO 460
      410 KJ#KEN(LS,K)
      CHNL0127
      CHNL0128
      CHNL0129
      CHNL0130
      CHNL0131
      CHNL0132
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      CHNL0172
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		D1 = M1-Z1	CHNL0220
		CF = IFC(K)	CHNL0221
		CF = CF*DELT/10000.	CHNL0222
270		KK = MCR(K)	CHNL0223
		ACS = I-CK(K)	CHNL0224
		ACRABS = ACS	CHNL0225
		IF(ACRABS.NE.0.) GO TO 2250	CHNL0226
		LS = 1	CHNL0227
275	C	Z2 = IZ(I+1,J)	CHNL0228
		M2 = M(I+1,J)	CHNL0229
		D2 = M2-Z2	CHNL0230
		QN = QCN(K)	CHNL0231
		QP = QCP(K)	CHNL0232
280		GT = MCT(K)	CHNL0233
		GF = UCF(K)	CHNL0234
		POT = PUSHU	CHNL0235
		PUC = PUSHV*HC	CHNL0236
		KAKCY(K)	CHNL0237
285		ZCS = IZCX(K)	CHNL0238
		ZC = ARS(ZCS)	CHNL0239
		IF(KK.EQ.0) GO TO 2310	CHNL0240
		ZAC = IZX(KK)	CHNL0241
		ZBC = ZAC/10.	CHNL0242
290		COOI = ICOSX(KK)	CHNL0243
		COOI = COOI/1000.	CHNL0244
		COOI = ICOSX(KK)	CHNL0245
		COOI = COOI/1000.	CHNL0246
		GO TO 2020	CHNL0247
295	C	*****OUTER RE-ENTRY POINT (X AND Y CHANNELS)	
		2010 COOI = COO	CHNL0248
		COOI = COO	CHNL0249
300	C	2020 MN = MC(KA)	CHNL0250
		MAC = (MCI+MN)/2.0	CHNL0251
		DAC = MAC-ZC	CHNL0252
		IF(DAC.GT. 0.0) GO TO 20205	CHNL0253
305	C	PRINT 20206, DAC, K	CHNL0254
		20206 FORMAT(' DAC=(F7.2) AT CHANNEL BLOCK(=I4,////)	CHNL0255
		GO TO 4000	CHNL0256
	C	20205 CEL = SORT(GRAY*DAC)	CHNL0257
310		ALP = C3*CEL	CHNL0258
		CALP = 1.0 - ALP	CHNL0259
		MA = ALP*AN + CALP*CI	CHNL0260
		ME = CALP*BN + ALP*CI	CHNL0261
		GA = ALP*ON + CALP*JP	CHNL0262
315		GA = CALP*ON + ALP*JP	CHNL0263
		LP = 1	CHNL0264
		LP = 0	CHNL0265
	C		

SUBROUTINE CHANL

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320	DI = D1	CHNL0266
	DII = D4C	CHNL0267
	MI = M1	CHNL0268
	MII = MAC	CHNL0269
	QI = Q1	CHNL0270
	MI = DELX * MC	CHNL0271
325	MII = MC	CHNL0272
	IF(MI.GT.0) GO TO 2022	CHNL0273
	ZH=Z1	CHNL0274
	GO TO 2021	CHNL0275
	ZH=Z0C	CHNL0276
330	IF(MI.GT.0) GO TO 2021	CHNL0277
	IF(ZCS.GT.0) ZH=Z1	CHNL0278
2021	LD=1	CHNL0279
	IF(Z1.GT.78) ZH=Z1	CHNL0280
	ZH=Z0	CHNL0281
335	C	
	*****INNER RE-ENTRY POINT (SIDES 1 AND 2 OF CHANNEL)	
	2025 IF(MII-ZR) 2030,2030,2040	CHNL0282
	2030 IF (MI-ZR) 2060,2060,2070	CHNL0283
	2040 IF(MI-ZR) 2075,2075,2080	CHNL0284
340	2060 GOUT = 0.	CHNL0285
	GO TO 2100	CHNL0286
	C OVERFLOW FROM REGION I TO REGION II	
2070	DH=MI-ZR	CHNL0287
	GO TO 2090	CHNL0288
345	C OVERFLOW FROM REGION II TO REGION I	
2075	DH=Z-MII	CHNL0289
	GO TO 2090	CHNL0290
	C SUBMERGED BARRIERS	
2080	GO TO (2081,2082) * LF	CHNL0291
350	2081 GOUT= MI-MII*(MI-MII)/((MI+MII)*DELT)	CHNL0292
	LF= 2	CHNL0293
	GO TO (2110,2120) * LG	CHNL0294
2082	GO TO (2083,2084) * LS	CHNL0295
2083	GOUT= U(I+1,J)	CHNL0296
355	GO TO 2085	CHNL0297
2084	GOUT= V(I,J+1)	CHNL0298
2085	QT = GOUT * ((MI-2.*MAC+M2)*MC-(MI-MAC)*(MC**2)/DELX)/(2.*DELT)	CHNL0299
	HQ= (QT-GOUT)/2.	
	QTS = GOUT+HQ	
360	GOUT= GOUT+HQ	
	GO TO 2120	CHNL0302
	C	
2090	GOUT = COMDMSQRT(GH*V*ABS(DH))	CHNL0303
	LD=LG	CHNL0304
365	2100 GO TO (2110,2120)*LG	CHNL0305
	C	
2110	DI = D4C	CHNL0306
	DII = D2	CHNL0307
	MI = MAC	CHNL0308
370	MII = M2	CHNL0309
	QI = Q2	CHNL0310

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      HI = MC
      H1 = DELX
      GT = DOUT
375      IF (H1.GT.0) GO TO 2112
      ZH=Z2
      GO TO 2111
2112      ZH=ZHC
      IF (H1.LT.0) GO TO 2111
380      IF (ZCS.LT.0) ZH=Z2
2111      LQ=2
      IF (Z2.GT.ZH) ZH=Z2
      ZH=Z2
      GO TO 2025
385      C*****END OF INNER RE-ENTRY
      C
      2120      QF=QOUT
      C
      C      THE FOLLOWING TESTS CONSTRAIN THE CHANNEL OVERFLOW (GT AND/OR GF) SUCH
      C      THAT (GF-GT) CANNOT PRODUCE AN IMPOSSIBLE CHANGE IN MC IN ONE TIME STEP
      C      (IE: MC SHOULD NOT FALL BELOW SILL DEPTH NOR RISE ABOVE THE HIGHER
      C      OF THE ADJOINING BLOCK H DUE TO OVERFLOW ALONE).
      C
      IF (LG.EQ.0) GO TO 2190
      IF (LF.EQ.0) GO TO 2190
      IF ((GF-GT).LE.0.0) GO TO 2140
      NET OUTFLOW= HARRIENS OVERTOPPING
      ZMIN=ZM1
      IF (ZD2.(Y,ZMIN) ZMIN=ZB2
      GNET = (HAC-ZMIN)*AC/DELT
      IF ((GF-GT).LE.GNET) GO TO 2190
      IF ((GF-GT).GT.0.0) GO TO 2130
      QFS=JF**2
      QTS=JF**2
      BUM=JNET/(QFS+QTS)
      QF=JF+QFS
      QTS=JF+QTS
      GO TO 2190
      2130      IF (GF.LT.0.0) GO TO 2135
      2134      QF=JF+QTS
      GO TO 2190
      2135      GT = -(GNET-QF)
      GO TO 2190
      C      NET INFLOW= HARRIENS OVERTOPPING
      2140      HMAX = H1
      IF (H2.GT.HMAX) HMAX = H2
      GNET = (HMAX-HAC)*AC/DELT
      IF ((GT-GF).LE.GNET) GO TO 2190
      IF ((GT-GF).GT.0.0) GO TO 2150
      QFS = GF**2
      QTS = GT**2
      BUM = GNET/(QFS+QTS)
      GF = GF+QFS
      GT = GT+QTS
      CHNL0311
      CHNL0312
      CHNL0313
      CHNL0314
      CHNL0315
      CHNL0316
      CHNL0317
      CHNL0318
      CHNL0319
      CHNL0320
      CHNL0321
      CHNL0322
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      CHNL0324
      CHNL0325
      CHNL0326
      CHNL0327
      CHNL0328
      CHNL0329
      CHNL0330
      CHNL0331
      CHNL0332
      CHNL0333
      CHNL0334
      CHNL0335
      CHNL0336
      CHNL0337
      CHNL0338
      CHNL0339
      CHNL0340
      CHNL0341
      CHNL0342
      CHNL0343
      CHNL0344
      CHNL0345
      CHNL0346
      CHNL0347
      CHNL0348
      CHNL0349
      CHNL0350
      CHNL0351
      CHNL0352
      CHNL0353

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SUBROUTINE CHANL

74/74 CP1#2

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```
425      GO TO 2194
      2150 IF (GT, 0.0) GO TO 2155
      2154 GT = QNET + QF
      GO TO 2194
      2155 QF = QT - QNET
      GO TO 2194
430      2160 GO TO (2170, 2180) * LO
      2170 IF (QT, GT, 0.0) GO TO 2175
      C BARRIER 1 OVERTOPPING OUTWARDS- OTHER SIDE SUBMERGED
      QNET = (MAC-ZR1)*C/DELT
      435 IF ((QF-QT).LE.QNET) GO TO 2190
      GO = QF-QNET
      IF (GO, GT, 0.0) GO = 0.
      GO TO 2170
      C BARRIER 1 OVERTOPPING INWARDS- OTHER SIDE SUBMERGED
      440 2175 QNET = (MAC-ZR2)*C/DELT
      IF ((QT-QF).LE.QNET) GO TO 2190
      GO = QT-QF
      IF (GO, LT, 0.0) GO = 0.
      445 2179 GT = GO
      GO TO 2194
      2180 IF (QF, LT, 0.0) GO TO 2185
      C BARRIER 2 OVERTOPPING OUTWARDS- OTHER SIDE SUBMERGED
      QNET = (MAC-ZR2)*C/DELT
      450 IF ((QF-QT).LE.QNET) GO TO 2190
      GO = QF-QT
      IF (GO, LT, 0.0) GO = 0.
      GO TO 2180
      C BARRIER 2 OVERTOPPING INWARDS- OTHER SIDE SUBMERGED
      455 2185 QNET = (MAC-ZR1)*C/DELT
      IF ((QT-QF).LE.QNET) GO TO 2190
      GO = QT-QNET
      IF (GO, GT, 0.0) GO = 0.
      2189 QF = GO
      C END OF ADJUSTMENT OF QT AND/OR QF
      460 2190 CONTINUE
      C
      C CHANNEL COMPUTATIONS
      AA = AC*CF
      465 GAM=1.0+CF*SQRT((QN**2+QP**2)/2.)/(AC*DAC**2)
      AOG = AA/GAM
      BOOM = CEI*(DELT*(QT-QF) + AC*RAIN)
      BP=(GA+AA*BA+PI*BOOM)/GAM
      B=(GA+AA*BA+PI*BOOM)/GAM
      470 GO TO (2200, 2300)*LS
      C
      2200 UCT(K) = QT
      UCP(K) = QF
      U(I+1,J) = QF
      475 QCX(K) = BP
      QCX(K) = BV
      AOGX(K) = AOG
      C
```

CHNL0354
CHNL0355
CHNL0356
CHNL0357
CHNL0358
CHNL0359
CHNL0360
CHNL0361

CHNL0362
CHNL0363
CHNL0364
CHNL0365
CHNL0366

CHNL0367
CHNL0368
CHNL0369
CHNL0370
CHNL0371
CHNL0372
CHNL0373

CHNL0374
CHNL0375
CHNL0376
CHNL0377
CHNL0378

CHNL0379
CHNL0380
CHNL0381
CHNL0382
CHNL0383
CHNL0384

CHNL0385
CHNL0386
CHNL0387
CHNL0388
CHNL0389
CHNL0390
CHNL0391

CHNL0392
CHNL0393
CHNL0394
CHNL0395
CHNL0396
CHNL0397

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SUBROUTINE CHANL

74/74 OPT=2

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	2250	ICS=ICV(K)	CHNL0398
		ICRS=ICV(K)	CHNL0399
480		IF(AC.EQ.0.) GO TO 2500	CHNL0400
		LS = 2	CHNL0401
		ZP = IZ(I,J+1)	CHNL0402
		H2 = H(I,J+1)	CHNL0403
		D2 = H2-ZP	CHNL0404
485		QT = QCYN(K)	CHNL0405
		QP = QCYP(K)	CHNL0406
		GT = VCT(K)	CHNL0407
		GF = VCF(K)	CHNL0408
		POT = PUSHV	CHNL0409
490		PUC = PUSHV+C	CHNL0410
		KA = KCV(K)	CHNL0411
		ZCS=IZCV(K)	CHNL0412
		ZCR=ARS(ZCS)	CHNL0413
		IF(KK.EQ.0) GO TO 2010	CHNL0414
495		ZHC= IZV(KK)	CHNL0415
		ZRC= ZHC/10.	CHNL0416
		COOI= ICDDY(KK)	CHNL0417
		COUI= COOT/1000.	CHNL0418
		COOI= ICDDY(KK)	CHNL0419
500		COOI= COOT/1000.	CHNL0420
		GO TO 2020	CHNL0421
		C*****END OF OUTER RE-ENTRY	
		C	
	2300	VCT(K) = QT	CHNL0422
505		VCF(K) = GF	CHNL0423
		V(I,J+1) = QF	CHNL0424
		QCYP(K) = QP	CHNL0425
		QCYN(K) = QT	CHNL0426
		AQGY(K) = AQ	CHNL0427
510	2500	CONTINUE	CHNL0428
		C	
	DO 2700	K=1,KCM	CHNL0429
		I= ICG(K)	CHNL0430
		I= IARS(I)	CHNL0431
515		J= JCG(K)	CHNL0432
		J= IARS(J)	CHNL0433
		HXB = I-CX(K)	CHNL0434
		HXB = ABSF(-X)	CHNL0435
		HXB = I-CV(K)	CHNL0436
520		HXB = ABSF(-Y)	CHNL0437
		KX = KCXP(K)	CHNL0438
		KY = KCYP(K)	CHNL0439
		BA = QCXP(K)	CHNL0440
		BB = QCYP(K)	CHNL0441
525		AGA = AQGY(K)	CHNL0442
		AGB = AQGY(K)	CHNL0443
		RCR2(CV(KX))	CHNL0444
		AGC=AGX(KX)	CHNL0445
		BOR2(CV(KY))	CHNL0446
530		AGD=AGY(KY)	CHNL0447

	C	HCM = (RA+RR+RC+RD)/(AGA+AGR+AGC+AGD)	CHNL0448
		QA=RA-AGA*HCM	CHNL0449
		QB=RB-AGB*HCM	CHNL0450
535		QCXN(KX) = BC+AGC*HCM	CHNL0451
		QCVN(KY) = RD+AGD*HCM	CHNL0452
		HC(K) = HCM	CHNL0453
		IF(ICG(K).LT.0) GO TO 2600	CHNL0454
		GO TO 2695	CHNL0455
540	C		
	C	BOUNDARY CONDITIONS FOR Q END POINTS	
	2600	L=1	CHNL0456
	2605	KEY=KENV(L,K)	CHNL0457
		GO TO(2690,2690,2630,2640,2650,2660,2670,2680), KEY	CHNL0458
545	2630	QA=QCXP(K)	CHNL0459
		GO TO 2690	CHNL0460
	2640	QB=QCVP(K)	CHNL0461
		GO TO 2690	CHNL0462
550	C		
	C	THE FOLLOWING ASSUMES Q=0 AT END IF NO DISCHARGE DATA EXISTS	
	2650	BA=QCXN(K)	CHNL0463
		KA=KCX(K)	CHNL0464
		KT=KHI(K)	CHNL0465
		QCXN(K)=0.	CHNL0466
555		IF(KT.GT.0) QCXN(K)=HRO(KT)	CHNL0467
		HC(KS)=(QCXN(K)-BA)/AOGX(K)	CHNL0468
		GO TO 2690	CHNL0469
	2660	BA=QCVN(K)	CHNL0470
		KA=KCY(K)	CHNL0471
560		KT=KHI(K)	CHNL0472
		QCVN(K)=0.	CHNL0473
		IF(KT.GT.0) QCVN(K)=HRO(KT)	CHNL0474
		HC(KS)=(QCVN(K)-BA)/AOGY(K)	CHNL0475
		GO TO 2690	CHNL0476
565	2670	BA=QCXP(K)	CHNL0477
		KT=KHI(K)	CHNL0478
		QA=0.	CHNL0479
		IF(KT.GT.0) QA=HRO(KT)	CHNL0480
		HC(K)=(QA-QA)/AOGX(K)	CHNL0481
570		GO TO 2690	CHNL0482
	2680	BA=QCVP(K)	CHNL0483
		KT=KHI(K)	CHNL0484
		QB=0.	CHNL0485
		IF(KT.GT.0) QB=HRO(KT)	CHNL0486
575		HC(K)=(BA-QB)/AOGY(K)	CHNL0487
	C		
	2690	IF(JGG(K).GT.0) GO TO 2695	CHNL0488
		IF(L.EQ.2) GO TO 2695	CHNL0489
		L=2	CHNL0490
580		GO TO 2695	CHNL0491
	C		
	2695	UCXP(K)=QA	CHNL0492
		QCVP(K)=QB	CHNL0493

SUBROUTINE CHANL

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585      2700 CONTINUE                                CHNL0494
C      RETURN                                          CHNL0495
C      ENTRY POINT 3 FOR HEIGHT CALCULATIONS ON BLOCKS WITH CHANNELS
C
590      3000 DO 3050 K=1,KCM
          I= ICG(K)                                CHNL0497
          J= IARS(I)                                CHNL0498
          J= JCG(K)                                  CHNL0499
          J= IARS(J)                                CHNL0500
595      IF(I.EQ.IM.OR.J.EQ.JM) GO TO 3050           CHNL0501
          Z=IZ(I,J)                                  CHNL0502
          H(I,J)=HG(I)                               CHNL0503
          IF(J.EQ.1) GO TO 3050                      CHNL0504
          UT=UCT(K)                                  CHNL0505
          VT=VCT(K)                                  CHNL0506
          X= I-CX(K)                                  CHNL0507
          X= ARSF(X)                                  CHNL0508
          Y= I-CY(K)                                  CHNL0509
          Y= ARSF(Y)                                  CHNL0510
605      IF(X.EQ.0.) UT=U(I+J)                      CHNL0511
          IF(Y.EQ.0.) VT=V(I+J)                    CHNL0512
          SETUP=DELT*((U(T,J)-UT)/(DELT-X)+(V(I,J)-VT)/(DELT-Y)) CHNL0513
          H(I,J)=H(K)+SETUP*RAIN                     CHNL0514
          IF(H(I,J).LE.7) H(I,J)=Z                  CHNL0515
610      HP(K)= H(T,J)                                CHNL0516
          DO 3050 K=1,KCM
          I= ICG(K)
          J= IARS(I)
          J= JCG(K)
          J= IARS(J)
          Z= IZ(I,J)
          IF(ICG(K).LT.0) GO TO 3100
          GO TO 3500
          CHNL0517
          CHNL0518
C      BOUNDARY CONDITIONS FOR H END POINTS
C      IN THESE CALCULATIONS HC EQUALS THE H OF THE ADJOINING WATER BLOCK
C      HC AND Q ARE SOLVED FROM SIMULTANEOUS EQUATIONS WHICH ALLOW FOR THE
C      VOLUME TRANSPORT TO OR FROM THE ADJOINING BLOCK VIA CHANNEL FLOW Q
620      3100 L=1
          TCF=2.*CQ
          KEY=KFNL(K)                                CHNL0519
          GO TO (3110,3120,3130,3140,3300,3300,3300,3300), KEY CHNL0520
          CHNL0521
          3110 KSKCX(K)                                CHNL0522
          BAW=QCXN(K)                                  CHNL0523
          IF(J.EQ.1) GO TO 3115                        CHNL0524
          HA=H(I,J+1)-QCXN(KS)/TCF                    CHNL0525
          DIV=1.+ADGX(K)/TCF                            CHNL0526
          QCXN(K)=(BAW+ADGX(K)*HA)/DIV                  CHNL0527
          HC(KS)=(HA+BAW/TCF)/DIV                       CHNL0528
          CHNL0529
          GO TO 3116                                     CHNL0530
635      3115 HC(KS)=HG(I)                                CHNL0531
          CHNL0531

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SUBROUTINE CHNL

74/74 OPT=2

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		QCXN(K)=RAV*ADGY(K)*HC(KS)	
		GO TO 3300	
640	3110	H(I+J)=H(KS)	CHNL0532
		QCXN(KS)=QCXN(K)	CHNL0533
		GO TO 3300	CHNL0534
	3120	AS=KCY(K)	CHNL0535
		BA=QCXN(K)	CHNL0536
645		IF(I.EQ.1) GO TO 3125	CHNL0537
		HAM=H(I+J)-QCXN(KS)/TCF	CHNL0538
		DIV=1.0+ADGY(K)/TCF	CHNL0539
		QCXN(K)=(RAV*ADGY(K)+HAM)/DIV	CHNL0540
		HC(KS)=(HAM+RAV/TCF)/DIV	CHNL0541
650		GO TO 3124	CHNL0542
	3125	HC(KS)=G/11	CHNL0543
		QCXN(K)=RAV*ADGY(K)*HC(KS)	CHNL0544
		GO TO 3300	CHNL0545
655	3126	H(I+J)=H(KS)	CHNL0546
		QCXN(KS)=QCXN(K)	CHNL0547
		GO TO 3300	CHNL0548
	3130	KS=KXCP(K)	CHNL0549
		BA=KXCP(K)	CHNL0550
		VAR=0.5/C4	CHNL0551
660		IF(KS.GT.WCH) GO TO 3132	CHNL0552
		AC=1-CY(KS)	CHNL0553
		AC=ABSF(AC)	CHNL0554
		VAR=C3*AC/(DELX+AC)	CHNL0555
665	3132	HAM=H(I+J)+QCXN(KS)*VAR	CHNL0556
		DIV=1.0+VAR*ADGY(K)	CHNL0557
		QCXN(K)=(RAV+HAM*ADGY(K))/DIV	CHNL0558
		HC(K)=(RAV+VAR*HAM)/DIV	CHNL0559
		H(I+J)=H(K)	CHNL0560
		HC(KS)=HC(K)	CHNL0561
670		QCXN(KS)=QCXN(K)	CHNL0562
		GO TO 3300	CHNL0563
	3140	KS=KCYP(K)	CHNL0564
		BA=KCYP(K)	CHNL0565
		IF(I.EQ.1) GO TO 3145	CHNL0566
675		VAR=0.5/C4	CHNL0567
		IF(KS.GT.WCH) GO TO 3142	CHNL0568
		AC=1-CY(K)	CHNL0569
		AC=ABSF(AC)	CHNL0570
		VAR=C3*AC/(DELX+AC)	CHNL0571
680	3142	HAM=H(I+J)+KCYP(KS)*VAR	CHNL0572
		DIV=1.0+VAR*ADGY(K)	CHNL0573
		QCXN(K)=(RAV+HAM*ADGY(K))/DIV	CHNL0574
		HC(K)=(RAV+VAR*HAM)/DIV	CHNL0575
		GO TO 3144	CHNL0576
685	3145	HC(K)=G/11	CHNL0577
		QCXN(K)=RAV*ADGY(K)*HC(K)	CHNL0578
		GO TO 3300	CHNL0579
	3146	H(I+J)=H(K)	CHNL0580
		HC(KS)=HC(K)	CHNL0581
		QCXN(KS)=QCXN(K)	CHNL0582
			CHNL0583
			CHNL0584

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SUBROUTINE CHANL

74/74 OPT=2

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	JL=JAL	CHNL0628
	IF(JGR.GT.JRL) JL=JHR	CHNL0629
745	JL=JL+1	CHNL0630
	DO 5500 I=1,IW	CHNL0631
	DO 5400 J=JL,JW	CHNL0632
	Z=IZ(I,I)	CHNL0633
	MIJRM(I,J)	CHNL0634
750	IF(Z.GT.0) -IJ=IJ-Z	CHNL0635
	IF(KCM.EQ.0) GO TO 5200	CHNL0636
	DO 5100 K=1,KCM	CHNL0637
	IC=ICG(K)	CHNL0638
	JC=JCG(K)	CHNL0639
755	IF(IABS(IC).EQ.I.AND.IABS(JC).EQ.J) GO TO 5300	CHNL0640
	5100 CONTINUE	CHNL0641
	5200 VOL=VOL+I*J*Ca	CHNL0642
	GO TO 5400	CHNL0643
	5300 I=I+CX(K)	CHNL0644
760	I=I+ABSF(-X)	CHNL0645
	J=J+ICV(K)	CHNL0646
	J=J+ABSF(-Y)	CHNL0647
	K=K+CK(K)	CHNL0648
	K=K+CV(K)	CHNL0649
765	VOL=VOL+I*J*(DELX=-X)*(DELX=-Y)+((HC(K)+HC(KX))*-X+	CHNL0650
	1*(HC(K)+HC(KY))*-Y)*DELX/2.-HC(K)*-X*-Y	CHNL0651
	5400 CONTINUE	CHNL0652
	5500 CONTINUE	CHNL0653
	VOL=VOL/1000000.	CHNL0654
770	JL=JL+1	CHNL0655
	PRINT 5600, VOL, JL	CHNL0656
	5600 FORMAT(1, // 1X, (VOLUME OF WATER ABOVE MSL =1, F12.1,	CHNL0657
	1 (MILLIONS OF CU FT)1, // 10X, ((THE SEAWARD ROWS THRU J=1, 13,	CHNL0658
	2 (ARE EXCLUDED)1, //)	CHNL0659
775	PRINT 4010	CHNL0660
	C	
	IF(N. EQ. 4) RETURN	CHNL0661
	C	
	PRINT 5700	CHNL0662
780	5700 FORMAT(1, //1 PROBLEM TERMINATED BECAUSE A CHANNEL HAS GONE DRY(CHNL0663
	1, //1)	CHNL0664
	STOP	CHNL0665
	C	
	END	CHNL0666

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SUBROUTINE SAVE

74/74 COTR2

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```

1      C
2      C
3      SUBROUTINE SAVE(JIN)                                SAVE0001
4
5      COMMON/RLK2/ IZ(28,20),U(28,20),V(28,20),W(28,20),NTIME    SAVE0004
6      COMMON/RLK3/ NM,MMIN,MMAX,NFU,INFLO,IM,JM,KM,KMAX,LMAX,DELX,DELT  SAVE0005
7      1,CD0,FK,HCI,IOU,K1,LJ,K11,LJJ,JBL,JBR,KMM,LMM,RF,CONST,S    SAVE0006
8      2,IMRU,JHRO,KR,ISTR,IND,NQ,KIM,NORT,NTIME,INTIME,NQIND,GRAV    SAVE0007
9      3,KCM,DFU,INTER                                                SAVE0008
10     COMMON/RLK5/ ICG(130),JCG(130),I-CX(130),I-CY(130),I-CZ(130)  SAVE0009
11     1,I-CY(130),QCXP(130),QCXN(130),QCYP(130),QCVN(130),MC(130),HP(130)  SAVE0010
12     2,KCM,KCX(130),KCY(130),KCB(130),UCT(130),UCF(130),KRI(130),IBCM  SAVE0011
13     3,KEN(2,130),VCT(130),VCF(130),ADGX(130),ADGY(130),KCXP(130)    SAVE0012
14     4,KCYP(130),KLR(50),KLM,IFC(130),FC                            SAVE0013
15     COMMON/RLK6/ MS(9,72),QS(6,72),TIME(72)                      SAVE0014
16     1,J,IND,NEWI,XND,NE=3,XNORT, C4,PAIN,AJ,I+LJK,KIK             SAVE0015
17     COMMON/RLK10/ NGAGE,NFLOW,IGAGE(12),JGAGE(12),KFLOW(6),XMIN,XMAX  SAVE0017
18
19     C
20     C      THIS ROUTINE SAVES WATER LEVELS AND FLOW RATES AT CERTAIN
21     C      KEY POINTS AS SPECIFIED IN INPUT BY USER. THE TIME SEQUENCES OF
22     C      THESE QUANTITIES ARE OUTPUT BY THE THIRD PART OF THIS ROUTINE.
23     C
24     GO TO(1000,2000,3000),JIN
25     1000 READ 135, (IGAGE(K),JGAGE(K),K=1,NGAGE)                SAVE0018
26     135 FORMAT(20I4)                                           SAVE0019
27     PRINT 136                                                    SAVE0020
28     136 FORMAT(1 I, /3X, 'HYDROGRAPH GAGE LOCATIONS')          SAVE0021
29     DO 100 K=1,NGAGE                                           SAVE0022
30     100 IGAGE(K)                                                SAVE0023
31     J= JGAGE(K)                                                 SAVE0024
32     IF(J.NE.0) GO TO 105                                       SAVE0025
33     PRINT 130, K, I                                             SAVE0026
34     130 FORMAT(5X, IGAGE(1:13), ' CHANNEL NO. K= ', I4)        SAVE0027
35     GO TO 100                                                  SAVE0028
36     105 PRINT 137, K, I, J                                       SAVE0029
37     137 FORMAT(5X, IGAGE(1:13), ' BLOCK NO. I= ', I3, ' J= ', I3)  SAVE0030
38     100 CONTINUE                                              SAVE0031
39     PRINT 138                                                    SAVE0032
40     138 FORMAT(1 I, /3X, 'KEY FLOW LOCATIONS')                 SAVE0033
41     C
42     READ 135, (KFLOW(K), K=1,NFLOW)                            SAVE0034
43     PRINT 139, (KFLOW(K), K=1,NFLOW)                            SAVE0035
44     139 FORMAT(5X, 'CHANNEL BLOCKS(1014)')                    SAVE0036
45     RETURN                                                       SAVE0037
46
47     C
48     2000 TS=NTIME-INTIME                                         SAVE0038
49     HINT=INTER                                                  SAVE0039
50     NET/HINT + 1                                                SAVE0040
51     TTE=NTIME                                                  SAVE0041
52     TIME(N)=TTE*DFLT/3600                                       SAVE0042
53     DO 200 K=1,NGAGE                                           SAVE0043
54     200 I=IGAGE(K)                                              SAVE0044
55     I=IGAGE(K)                                                  SAVE0045

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SUBROUTINE SAVE

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55      J=JGAGE(K)
      IF (I.GT.NCM) GO TO 199
      MS(K,N)=MC(I)
199    IF(J.NE.0) MS(K,N)=M(I,J)
200    CONTINUE
C
60      DO 300 J=1,NFLO-
      K=KFLD(J)
      KEYS=KEN(1,K)
      IF(KEV.NE.0) GO TO 205
      KEYS=2
65      IF(IACX(K).NE.0) KEYS=1
205    GO TO(210,220,230,240,210,220,230,240), KEYS
210    QS(J,N)=QCVN(K)/1000.
      GO TO 300
220    QS(J,N)=QCVN(K)/1000.
      GO TO 300
70      230    QS(J,N)=QCVN(K)/1000.
      GO TO 300
240    QS(J,N)=QCVN(K)/1000.
300    CONTINUE
75      NU=72
      IF(N.FQ.72) GO TO 310
      RETURN
C
3000   NU = (NU-INTIME)/INTER
80     IF(NU.EQ.0) RETURN
310    PRINT 410
400    FORMAT(20X,'WATER LEVEL HYDROGRAPHS (FT) AND KEY FLOWS (1000 CFS)
      1 //)
85     PRINT 410, (J,J=1,NGAGE), (K,K=1,NFLO-)
410    FORMAT(2X,'(HOUR:10.1518)
      DO 500 N=1,NU
      PRINT 420, TIME(N), (MS(J,N)+J=1,NGAGE), (QS(K,N)+K=1,NFLO-)
420    FORMAT(F6.1,15F8.2)
90     425    FORMAT(F6.1,12F8.2)
500    CONTINUE
      DO 510 N=1,NU
95     430    FORMAT(F6.1,10F8.2)
510    CONTINUE
      PRINT 10
10     10    FORMAT(1H1)
      IF(NU.EQ.72) INTIME=NTIME
      RETURN
100    C
      END
C
1      C
      C
      SUBROUTINE CONTN(L)
C
5      COMMON/ALV1/A(822)/ALV2/B(1961)/BLK3/C(42)/ALV4/D(2585)/
      1BLK5/E(2759)/ALV6/F(2800)/BLK7/G(44)/ALV9/H(25)/ALV10/P(34)
C
      GO TO (100,200),L
C
10     100    CONTINUE
      700    FORMAT(20A6)
      RETURN
C
15     200    CONTINUE
      RETURN
      C
      END

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SUBROUTINE PLOT

74/74 CPT=2

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55	122 = IZ(I+1,JJ)*10 126 = IZ(KR) XBARR = .TRUE. IF (IZ4.EQ. 171 .OR. IZ4.EQ. 122) XBARR = .FALSE. 122 = IZ(I+JJ+1)*10 126 = IZ(KR) YBARR = .TRUE. IF (IZ4.EQ. 171 .OR. IZ4.EQ. 122) YBARR = .FALSE.	PLOT0185 PLOT0190 PLOT0195 PLOT0200 PLOT0205 PLOT0210 PLOT0215 PLOT0220
	C IF (.NOT. XBARR) GO TO 250	PLOT0225
65	C X BARRIERS C IF (I-CX(K) .LT. 0) GO TO 230 C IF (IZCX(K) .LE. 0) GO TO 231	PLOT0230 PLOT0235
70	C OUTER BARRIER C DO 202 L=1,10 202 PAGE(I+4,J+L-3) = X GO TO 250	PLOT0240 PLOT0245 PLOT0250
75	C INNER BARRIER C 231 DO 203 L=1,10 203 PAGE(I+2,J+L-3) = X GO TO 250	PLOT0255 PLOT0260 PLOT0265
80	C BOTH BARRIERS C 230 DO 204 L=1,10 PAGE(I+4,J+L-3) = X 204 PAGE(I+2,J+L-3) = X	PLOT0270 PLOT0275 PLOT0280
85	C Y BARRIERS C 250 IF (.NOT. YBARR) GO TO 240 IF (I-CY(K) .LT. 0) GO TO 240 IF (IZCY(K) .LE. 0) GO TO 241	PLOT0285 PLOT0290 PLOT0295
90	C OUTER BARRIER C DO 205 L=1,5 205 PAGE(I+L-2,J+4) = X GO TO 800	PLOT0300 PLOT0305 PLOT0310
95	C INNER BARRIER C 241 DO 206 L=1,5 206 PAGE(I+L-2,J+4) = X GO TO 800	PLOT0315 PLOT0320 PLOT0325
100	C BOTH BARRIERS C 240 DO 207 L=1,5 PAGE(I+L-2,J+4) = X 207 PAGE(I+L-2,J+4) = X 800 CONTINUE	PLOT0330 PLOT0335 PLOT0340 PLOT0345
15	C	

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SUBROUTINE PLOT

74/74 DTR2

FTN 4.0+420

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C      LAND BARRIERS
C
110      DO 804 K=1,KLM                                PLOT0350
C      TEST FOR BARRIER IN X DIRECTION, Y DIRECTION, OR BOTH
C
115      KB = KLB(K)                                PLOT0355
          II = IP(KR)                                PLOT0360
          JJ = JP(KR)                                PLOT0365
          I = IABS(II)*4-1                            PLOT0370
          J = IABS(JJ)*4-1                            PLOT0375
          IZ1 = IZ(I,JJ)*10                          PLOT0380
          IZ2 = IZ(I+1,JJ)*10                        PLOT0385
120      IZB = IZ(KR)                                PLOT0390
          XBARR = .TRUE.                             PLOT0395
          IF ( IZP .EQ. IZ1 .OR. IZB .EQ. IZ2 ) XBARR = .FALSE. PLOT0400
          IZ2 = IZ(I,JJ+1)*10                        PLOT0405
          IZB = IZ(KR)                                PLOT0410
125      YBARR = .TRUE.                             PLOT0415
          IF ( IZB .EQ. IZ1 .OR. IZB .EQ. IZ2 ) YBARR = .FALSE. PLOT0420
C
C      X BARRIER
C
130      IF ( .NOT. XBARR ) GO TO 220                PLOT0425
          DO 208 L=1,7                                PLOT0430
208      PAGE(I+2,J+L-3) = X                          PLOT0435
C
C      Y BARRIER
C
135      220 IF ( .NOT. YBARR ) GO TO 804            PLOT0440
          DO 209 L=1,5                                PLOT0445
209      PAGE(I+L-3,J+4) = X                          PLOT0450
          804 CONTINUE                                PLOT0455
140      C
C      DRAW CHANNELS
C
251      DO 802 K=1,KCM                                PLOT0460
          I = IABS(ICG(K))*4-1                        PLOT0465
          J = IABS(JCG(K))*4-1                        PLOT0470
          I4 = I+3                                    PLOT0475
          J4 = J+3                                    PLOT0480
          IF ( I-CX(K) .EQ. 0 ) GO TO 300             PLOT0485
          DO 200 L=1,7                                PLOT0490
150      200 PAGE(I4,J+L-1) = MLINE                    PLOT0495
          IF ( KCX(K) .GT. KCM ) PAGE(I4,J) = PLUS    PLOT0500
          300 IF ( I-CY(K) .EQ. 0 ) GO TO 301          PLOT0505
          DO 201 L=1,3                                PLOT0510
          PAGE(I4+L-1,J+5) = BLNK                     PLOT0515
          PAGE(I4+L-1,J+7) = BLNK                     PLOT0520
155      201 PAGE(I4+L-1,J4) = VLINE                    PLOT0525
          IF ( KCY(K) .GT. KCM ) PAGE(I4,J4) = PLUS    PLOT0530
          301 PAGE(I4,J4) = PLUS                       PLOT0535
          802 CONTINUE                                PLOT0540
160      C
C      WRITE OUT THE PAGE
C
          WRITE(6,501)(J,I=1,19),((PAGE(4*K-1,J),J=3,134),K,(PAGE(4*K,J),J=1,28),K=1,28)
          501 FORMAT (11(1,19(14,3X),14,/,1(1,5X,17(1,1,5X,1(1,7,28(1X,
165      1 132A1,/,1X,12,130A1,/,2(1X,132A1,/,)),1(1
          RETURN
          END

```

APPENDIX B

DESCRIPTION OF THE SURGE II CODED PROGRAM

The general strategy of the program is discussed and certain special features are pointed out which may not be apparent without detailed study of the program. Operational aspects of the program are discussed in some detail in Appendix C.

The version of the program adapted for use on the GE 400 computer system by the Corps of Engineers consists of the following parts or subroutines:

- MAIN whose primary job is to read and check the sequencing of the basic data for the block computations;
- PART 2 which controls the basic computational sequencing, initialization, and updating of storage, interpolation of coarse wind fields for the actual grid, and routine computation of U, V, and H for all blocks, considering barriers (basically, the SURGE I program);
- CHANL(1) which is called only once to read channel data and to establish certain key arrays for routine calculation;
- CHANL(2) which is called routinely to compute flow and water levels in channels and at channel end points;
- CHANL(3) whose task is the routine calculation of H on blocks containing channels;
- CHANL(4) which is called for listing of channel computations;
- LIST(1) which is called only once to read control data for block listings and to list the topographic Z field;
- LIST(2) which lists the H field for blocks if called;
- LIST(3) which lists the U, V, and H fields for blocks if called in place of LIST(2);
- SAVE(1) which is called only once to read the positions of certain gage locations for water level or flow;
- SAVE(2) which is called routinely at preselected time intervals to save water levels and flow for gage locations defined by SAVE(1);
- CONTIN(1) which is called only once to read basic storage in COMMON BLOCKS 1 to 10 in the case of a continuation of a given problem;
- CONTIN(2) which is called at the termination of a run to output the continuation data called for by CONTIN(1).

The version of the program used in the testing and calibration work, using an IBM 360/65 computer system, has an additional assembler language subroutine for plotting positions of barriers and channels (see Fig. 15). This is useful in checking input data for channels and barriers to spot possible errors in coding the positions of channel blocks and barrier blocks. Unfortunately, this subroutine is not compatible with the GE 400 system. Subroutine PLOT in Appendix A however can be used for this purpose. Subroutine LIST is not used in the version of the program in Appendix A.

1. Flow Diagram.

A schematic flow diagram for the SURGE II program is given in Figure B-1. If a new problem is being run then the first phase is reading in the basic data and checking the data sequencing to make sure it is in order and complete. This is carried out in MAIN and the beginning of PART 2 which calls subroutines CHANL(1), SAVE(1), and LIST(1).

Initialization of block arrays is carried out in PART 2; initialization of channel arrays and establishing of key arrays are carried out by CHANL(1). These key arrays are discussed in a subsequent subsection.

Step 4 of the flow diagram is the beginning (or reentry point) of the routine computations for each time. After generating, the detailed interpolated fields of x and y components of wind stress for the blocks (step 4) and all blocks (i.e., all I, J) are swept to compute the flow components, U and V , ignoring at first the presence (if any) of subgrid scale channels, but considering barriers for any barrier blocks (step 5).

In step 6 CHANL(2) is called to sweep through all channel blocks to evaluate all channels Q and H except those for H-end points and all lateral flows to and from channels. In the latter operation, the flows U and V computed in step 5 are replaced by corrected U or V between blocks, considering the presence of the channels.

Step 7, which is carried out in PART 2, sweeps all I, J to compute water levels on blocks ignoring for the present, the presence of any subgrid scale channels.

In step 8, CHANL(3) is called to correct the block H values on those blocks containing channels and to compute the H and Q values at H-end points of channels. This also provides corrected H values for those blocks into which the channels discharge.

Steps 10 and 11 are output operations for block and channel computations carried out in PART 2 and CHANL(4). This is followed by a time updating and test for end, dependent upon a prescribed maximum number of time steps. Before termination of a run, the contents of all data in COMMON are saved for possible continuation of the problem, if desired.

2. Identification of Adjacent Channel Blocks.

To provide rapid access to values of H and Q in channels adjoining a given channel reach, special arrays are generated in subroutine

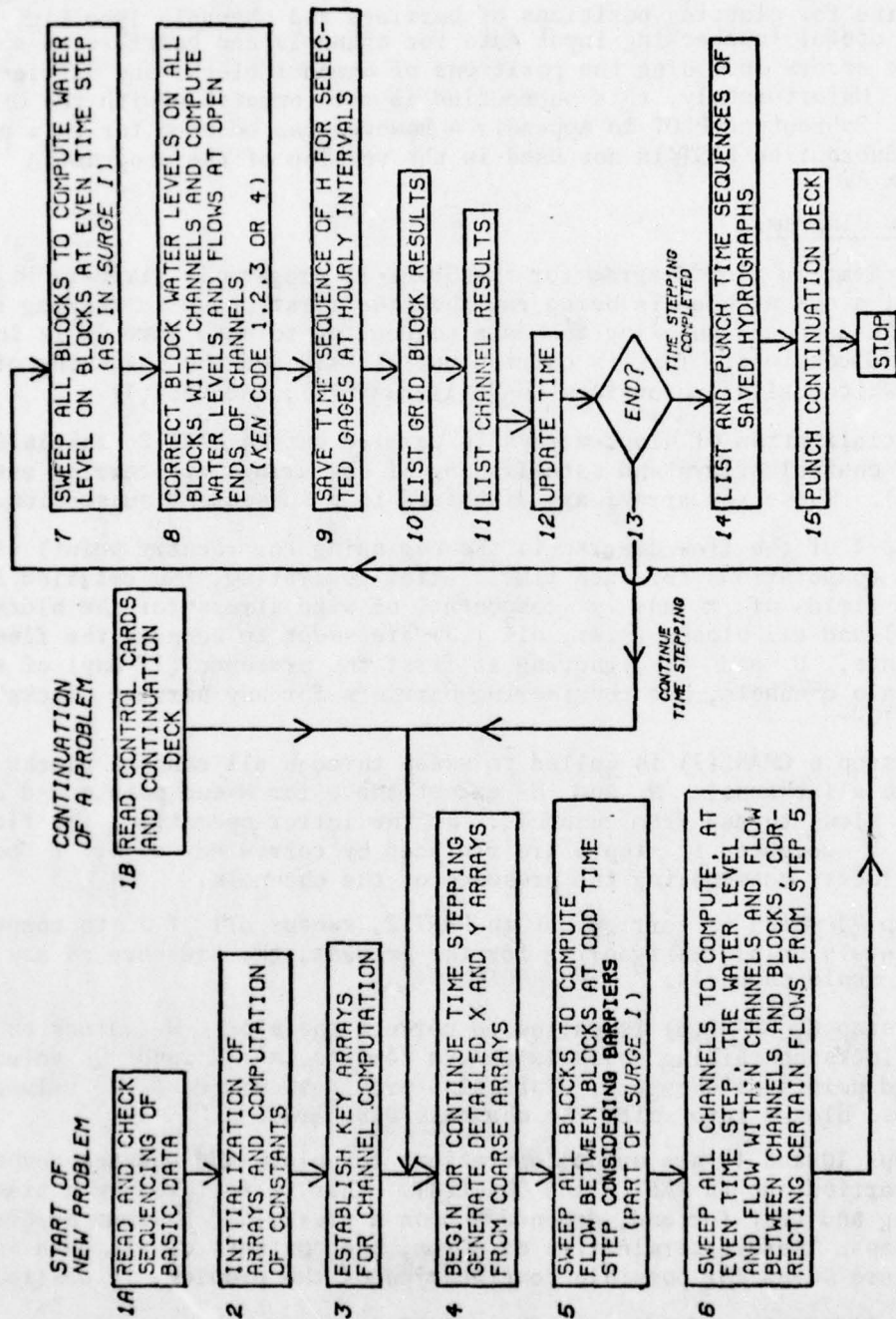


Figure B-1. Generalized flow diagram for SURGE II.

CHANL(1). There are four such arrays: $KCX(K)$, $KCY(K)$, $KCXP(K)$, and $KCYP(K)$. These give the channel block identification index for those channel blocks which are adjacent to the K th channel block as indicated in Figure B-2. Thus, $KCX(K)$ is the identification of the channel block which has an x-side channel adjoining channel block K on the negative characteristic side (i.e., on a preceding row), while $KCXP(K)$ is the identification of the channel block which has an x-side channel adjoining channel block K on the positive side (i.e., on a following row). $KCY(K)$ and $KCYP(K)$ have analogous meanings for blocks with y-side channels adjoining that of block K . These arrays are generated by an appropriate series of tests in which the I, J values of blocks adjacent to that of channel block K are compared with the ICG and JCG values of all other channel blocks. This is carried out only once during any run, and is not particularly time consuming; moreover, it avoids any human error which may easily occur if such arrays were required as input.

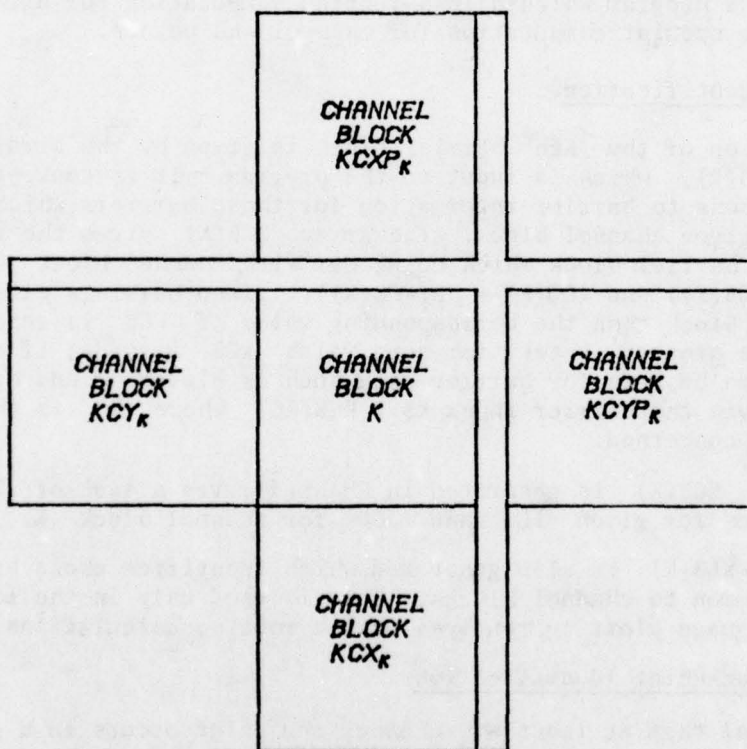


Figure B-2. Channel block identification for channels adjacent to those of block K .

The arrays KCX and $KCXP$ have the properties $KCXP(KCX(K)) = K$ and $KCX(KCXP(K)) = K$ with similar relations for KCY and $KCYP$.

As an example of the use of such arrays, suppose the value of HC in an x channel adjoining that of channel block K is needed. This could

be addressed as $HC(KX)$ where $KX = KXC(K)$. Using Figure 8 as an example, the values of channel flow entering the junction from channels 1 and 2 would be addressed by $QCXP(K1)$ and $QCYP(K1)$, respectively, where $K1$ designates the channel block containing channels 1 and 2. However, the flow leaving the junction would be addressed by $QCYN(K2)$ where $K2 = KCYP(K1)$ and $QCXN(K3)$ where $K3 = KCXP(K1)$. While redundant storage of such H and Q values would also satisfy the requirement of rapid access to such values adjoining a given channel block, the use of the integral arrays KCX , KCY , $KCXP$, and $KCYP$ saves storage for most computer systems.

An examination of the listings of the values of the arrays KCX , KCY , $KCXP$, and $KCYP$, as output by the program, indicates that the maximum value of any of these can and usually does exceed the number of input channel blocks (KCM). The reason for this is that dummy storage positions are created for blocks adjoining channel end points. This is an artifice of the program which allows routine computation for all channel reaches before special computation for channel end points.

3. Barrier Identification.

The position of the K th barrier block is given by the array pair, $IB(K)$ and $JB(K)$, which is input to the program. It is convenient to have rapid access to barrier information for those barriers which happen to fall on a given channel block. The array $KCB(K)$ gives the identification of the barrier block which coincides with channel block K . Thus, $ICG(K) = IB(KCB(K))$ and $JCG(K) = JB(KCB(K))$. If no barriers exist in a given channel block then the corresponding value of KCB is zero. Thus, in the routine program, a test for zero value KCB is made; if nonzero, then a call can be made for barrier data such as elevation and barrier coefficients via the barrier index $KB = KCB(KC)$ where KC is the channel block concerned.

The array $KCB(K)$ is generated in $CHANL(1)$, via a scan of all IB and JB values for given ICG and JCG for channel block K .

An array $KLB(K)$ is also generated which identifies those barrier blocks not common to channel blocks. This is used only in the IBM 360/65 assembler language plotting routine, not in routine calculations.

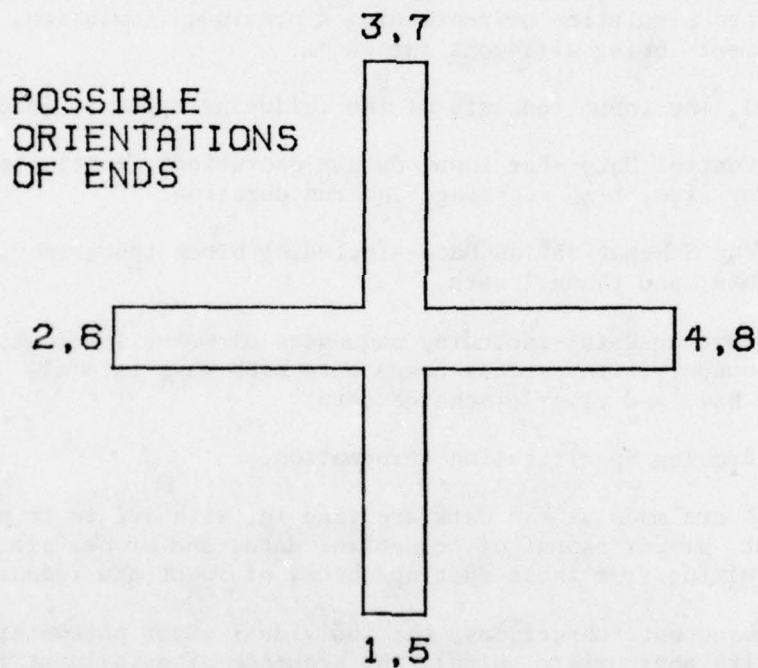
4. Channel End-Point Identification.

As a signal that at least one channel end point occurs in a channel block K , the value of $ICG(K)$ is negative. If two end points occur, the value of $JCG(K)$ is also negative; otherwise, it is positive. If no channel end point occurs, then both ICG and JCG for the block are positive. This positive-negative coding is generated automatically in $CHANL(1)$ by appropriate testing; namely, to check if a valid channel connects at each end of a valid channel in the block concerned.

In addition, the arrays $KEN(1,K)$ and $KEN(2,K)$ are generated in $CHANL(1)$ to identify the type of end point for, at most, two potential

channel terminations in given channel block K . If there is no channel termination both $KEN(1,K)$ and $KEN(2,K)$ are zero; if one termination occurs for block K , $KEN(1,K)$ will have an integral value from 1 to 8 and $KEN(2,K)$ will be zero; if two terminations occur, both KEN arrays will have nonzero value. In use, $KEN(2,K)$ is called only if $JCG(K)$ is negative.

The coding for the type of end point is indicated schematically in Figure B-3. Values of KEN from 1 to 4 represent "H-end" type terminations where a ponding block immediately adjoins the channel end. Values of KEN from 5 to 8 are those for which Q is specified; e.g., river discharge. Values within either group indicate the relative orientation of the channel end point in question to assure calling the correct data and using the right signs in the routine calculations.



	<u>TYPE OF END</u>
$KEN_K = 1, 2, 3, 4$	HC=H OF ADJACENT BLOCK
$KEN_K = 5, 6, 7, 8$	Q SPECIFIED

Figure B-3. Identification of type and orientation of a channel end point by the coded identifier $KEN(K)$.

APPENDIX C

USER'S GUIDE TO SURGE II

The coded program SURGE II is intended for use in the numerical simulation of storm surges or astronomical tides in bays and estuaries for specified time sequences of water level at the seaward boundary of the bay or estuary and specified wind stress and other storm data over the bay or estuary. The user may use one of two distinct modes of operation: (a) the storm mode, in which all storm data are required as well as seaward hydrograph data; or (b) the tide mode, in which no storm data are required, the only forcing being the input water level variation at the seaward boundary. Moreover, in both modes the user has the option of initiating a new simulation or continuing a previous simulation, the input requirements being different for each.

In general, the input consists of the following types of information:

(a) Control Data--For input-output operations, initialization, array size, time stepping, and run duration.

(b) Bay Schematization Data--including block topography, barrier data, and channel data.

(c) Forcing Data--including sequences of water level at seaward boundary, wind-stress components over bay, rainfall data over bay, and river discharge data.

(d) Problem Specification Information.

Certain checks are made as the data are read in, with regard to proper order of input, proper amount of sequential data, and proper size arrays. All stops resulting from these editing checks of input are identified.

In the subsequent subsections, the individual input parameters are identified (with appropriate units), the sequence of data input for the different modes of operation is given in some detail, and special requirements concerning data input for barriers and channels are discussed, followed by a summary of output information and output options.

1. Definition of Input Variables.

The following variables are listed in the order in which they are input (asterisks separate data blocks):

ICARD Control index: 0 for starting, 1 for continuation.

***** *Block 0*

IDENT Data block identification;

IBL starting column (I value) for listing of block H output
 (normally taken as 1);

KCM total number of blocks with channels (including null channels,
 see subsec. 6 of this app.);

NOWIND control for storm data input: 0 for normal input operation for
 wind stress, rainfall, and runoff; -1 for omitting such input
 for tide computations;

INTER interval in SAVE operation (time interval is INTER*DELT);

NGAGE number of H gage locations saved;

NFLOW number of Q gage locations saved;

IMIN minimum expected H (feet);

IMAX maximum expected H (feet).

NOTE----IMIN and IMAX are used only in subroutine GRAF, applicable to
 IBM 360 or 370.

***** *Block 1*

NTIME Initial time level (normally 0, unless a continuation run is
 being carried out, in which case NTIME should equal the final
 value of the previous run);

NM maximum number of time steps for the problem;

MMIN minimum "map time" for wind-stress input;

MMAx maximum map time for wind-stress input;

NFU number of iterations per map time interval;

IOUT interval for routine output from blocks and channels equals
 IOUT + 1;

INFLD special output flag: 0 for standard output, 1 for extra
 listing of channel output for one iteration preceding normal
 listing.

***** *Block 2*

IM Total number of x-grid intervals;

JM total number of y-grid intervals;

KM total number of blocks having barriers;

KMAX total number of coarse x-grid points for wind-stress input;

LMAX total number of coarse y-grid points for wind-stress input.

***** *Block 3*

DELX Spatial grid interval or block size (nautical miles);

DELT time interval between block H and flow computations (seconds);

CDO overflow coefficient for natural low-lying ground such as
barrier islands;

FK bed-resistance coefficient for blocks;

FC bed-resistance coefficient for channels (used only if values
for individual channels are not entered);

HGI initial water level above MSL in the bay (feet).

***** *Block 4*

KI Number of interpolation subdivisions of each coarse x-grid
interval $KI*(KMAX-1) = IM$;

LJ number of interpolation subdivisions of each coarse y-grid
interval $LJ*(LMAX-1) = JM$;

KII number of coarse x-grid intervals;

LJJ number of coarse y-grid intervals;

JBL, JBR number of "open boundary" J-intervals on left and right
of system (not used in version in App. A).

***** *Block 5*

IB(K) I location index for barrier block K;

JB(K) J location index for barrier block K;

IZX(K) elevation of x-barrier (right side) on barrier block K (tenths
of feet);

IZY(K) elevation of y-barrier (upper side) on barrier block K (tenths
of feet);

ICDOX(K) overflow coefficient for x-barrier (value $\times 1,000$) on Kth
barrier block;

ICDOY(K) overflow coefficient for y-barrier (value $\times 1,000$) on Kth barrier block;

ICDSX(K) submerged wier coefficient for x-barrier (value $\times 1,000$) on Kth barrier block;

ICDSY(K) submerged wier coefficient for y-barrier (value $\times 1,000$) on Kth barrier block.

***** Block 6

IZ(I,J) Elevation of ground or seabed (feet) relative to MSL datum for block location I,J.

***** Block 7

IMRO Number of river input (runoff) locations;

JMRO number of map times with runoff values;

KR number of channel-stress values (normally same as JMRO);

ISTR start of rain (map time);

IND end of rain (map time);

NOW number of iterations between river input values (normally same as NFU);

KIM number of iterations between channel-stress values (normally same as NFU);

NORT number of iterations per hour for rain (normally same as INTER).

***** Block 8

RF Total rainfall (inches);

CONST fraction of rainfall not absorbed by ground;

S conversion factor for wind stress $(5,280/3,600)^2 \times 1.1/10$.

***** Block 9

LROI(K) I location index for Kth river input block;

LROJ(K) J location index for Kth river input block.

***** *Block 10*

DIST(M) Percent of total rainfall per hour for 24 hours.

***** *Block 11*

CHST(M) Channel-stress values at map time M (entries are used only if KCM = 0).

***** *Block 12*

RO(K,M) Discharge (cubic feet per second) from Kth river input block at map time M.

***** *Block 13*

MTIME Map time for given block of wind-stress input and seaward water level.

***** *Block 14*

HGR(K) Seaward water level above MSL (feet) at MTIME for coarse grid position K.

***** *Block 15*

HBR(J) Water level on right open boundary above MSL (feet) at MTIME for grid position J (not used in version in App. A).

***** *Block 16*

XR(K,L) Wind-stress component in the x direction (units of (miles per hour)²/10) for coarse grid position K,L at time MTIME.

***** *Block 17*

YR(K,L) Wind-stress component in the y direction (units of (miles per hour)²/10) for coarse grid position K,L at time MTIME.

***** *Block 18*

ICG(K) I location index for channel block K;

JCG(K) J location index for channel block K;

IWCX(K) width of x channel (right side) on channel block K (feet), with sign (see subsec. 6 of this app.);

IZCX(K) depth of x channel bed on channel block K (feet), with sign (see subsec. 6 of this app.);

IWCY(K) width of y channel (upper side) on channel block K (feet), with sign (see subsec. 6 of this app.);

IZCY(K) depth of y channel bed on channel block K (feet), with sign (see subsec. 6 of this app.);

IFC(K) bed-resistance coefficient for channels on block K (value $\times 10,000$), if entry is zero (blank) then IFC is taken as FC (entered in *Block 3*) $\times 10,000$.

***** *Block 19*

IGAGE(K) Location index for the Kth hydrograph, if JGAGE(K) $\neq 0$ then IGAGE(K) is the I location of a block H; if JGAGE(K) = 0 then IGAGE(K) is the channel block index for a channel H;

JGAGE(K) if not zero, this is the J location of a block H; if zero, a channel H is indicated;

KFLOW(K) channel block index for the Kth flow gage, the flow being that of the lower end of the x channel, or the left end of a y channel if an x channel does not exist, or a channel end point if one exists in the identified channel block.

***** *Block 20*

IEND Maximum I in listing of block arrays of H, U, and V;

NF number of iterations between listings;

IBEGIN first I in listing of block arrays;

NJ maximum J in listing of block arrays;

NCARD total number of alphanumeric problem identification cards;

ALPHA(J) alphanumeric character data which identify the problem and gage locations by name.

2. Input for Initiating Storm Surge Simulation.

The sequence of input for starting a problem in the storm surge mode is given below in the form of a summary of the READ statements active in this mode, together with a summary of the appropriate FORMATS for data input in different blocks. For all data blocks requiring an entry of the identification integer IDENT, only the *units digit* of the data block number is entered in column 1 of the data input card.

Control Card

READ 1 , ICARD (0 for starting)

Block 0 (1 card)

READ 1 , IDENT, IBL, KCM, NOWIND, INTER, NGAGE, NFLOW, IMIN, IMAX

NOTE-----IMIN and IMAX are left blank unless subroutine GRAF is used.

Block 1 (1 card)

READ 100 , IDENT, NTIME, NM, MMIN, MMAX, NFU, IOUT, INFLD

Block 2 (1 card)

READ 100 , IDENT, IM, JM, KM, KMAX, LMAX

Block 3 (1 card)

READ 250 , IDENT, DELX, DELT, CDO, FK, FC, HGI

Block 4 (1 card)

READ 100 , IDENT, KI, LJ, KII, LJJ, JBL, JBR

Block 5 (total of KM cards of barrier data)

DO 500 K = 1, KM

READ 100 , IDENT, IB(J), JB(K), IZX(K), IZY(K), ICDOX(K), ICDOY(K),
ICDSX(K), ICDSY(K)

500 CONTINUE

*Block 6 (total of 2*IM cards of block topography)*

DO 550 I = 1, IM

READ 100 , IDENT, (IZ(I,J), J = 1,10)

READ 100 , IDENT, (IZ(I,J), J = 11, JM)

550 CONTINUE

Block 7 (1 card)

READ 100 , IDENT, IMRO, JMRO, KR, ISTR, IND, NOW, KIM, NORT

Block 8 (1 card)

READ 250 , IDENT, RF, CONST, S

Block 9 (1 or 2 cards, dependent on IMRO)

READ 100 , IDENT, (LROI(K), LROJ(K), K = 1,5)

IF (IMRO.LT.6) GO TO 575

READ 100 , IDENT, (LROI(K), LROJ(K), K = 6, IMRO)

575 CONTINUE

Block 10 (3 cards)

READ 250 , IDENT, (DIST(M), M = 1,10)

READ 250 , IDENT, (DIST(M), M = 11,20)

READ 250 , IDENT, (DIST(M), M = 21,24)

Block 11 (L + 1 card where L = KR/10. If KR = 0, block 11 input is omitted.)

READ 250 , IDENT, (CHST(K), K = 1,11)

READ 250 , IDENT, (CHST(K), K = 11,20)

...

READ 250 , IDENT, (CHST(K), K = KL, KR (KL = 10 * L + 1)

Block 12 (JMRO cards of river discharge data)

DO 700 M = 1, JMRO

READ 250 , IDENT, (RO(K,M), K = 1, IMRO)

700 CONTINUE

Wind Stress and Water Level Forcing

(MTL sets of blocks 13 to 17 where MTL = MMAX - MMIN + 1)

710 CONTINUE

Block 13 (1 card)

READ 100 , IDENT, MTIME

Block 14 (1 card)

READ 250 , IDENT, (HGR(K), K = 1, KMAX)

Block 15 (1 card)

READ 250 , IDENT, (HBR(J), J = 2,8)

Block 16 (KMAX cards)

DO 790 K = 1, KMAX

READ 250 , IDENT, (XR(K,L), L = 1, LMAX)

790 CONTINUE

Block 17 (KMAX cards)

DO 800 K = 1, KMAX

READ 250 , IDENT, (YR(K,L), L = 1, LMAX)

800 CONTINUE

IF (MTIME - MMAX) 710, 1,015, 1,015 (710 returns to read block 13)

1,015 (CONTINUE)

Block 18 (KCM cards with channel data. If KCM = 0, the READ statement is bypassed and block 18 should be omitted.)

IF (KCM.GT.0) CALL CHANL(1)

DO 50 K = 1, KCM

READ 100 , IDENT, ICG(K), JCG(K), IWCX(K), IZCX(K), IWCY(K), IZCY(K),
IFC(K)

50 CONTINUE

Block 19 (2 cards)

CALL SAVE(1)

READ 350 , (IGAGE(K), JGAGE(K), K = 1, NGAGE)

READ 350 , (KFLOW(K), K = 1, NFLOW)

Block 20 (NCARD + 1 card)

CALL LIST(1)

READ 1 , IDENT, IEND, NF, IBEGIN, NJ, NCARD

DO 250 J = 1, NCARD

READ 450 , (ALPHA(J), J = 1,40)

250 CONTINUE

Format Statements for Input. The following formats were used in all the testing operations. It is recommended, however, that for routine operations those READ statements using FORMAT 1 be replaced by FORMAT 100 to make all basic numerical input consistent in card column range.

1 FORMAT (I1, I3, 19, I4)

100 FORMAT (I1, 2X, I5, 9(3X, I5)

250 FORMAT (I1, F7.0, 9F 8.0)

350 FORMAT (20 I 4)

450 FORMAT (15A2, 15A2, 10A2)

3. Input for Tide Mode.

For calibration of a given bay system, under virtually no wind conditions, for its response to forcing by astronomical tide at the seaward boundary and a steady-state river discharge, allowance is made in the coded program to bypass the detailed input of wind-stress components, and rainfall and channel-stress data; moreover, since a steady river discharge is assumed only a single card is required to define this input. In essence, the data blocks 10 to 17 are replaced by a shortened version of block 12 plus a modified version of block 14 in which tide data at the seaward boundary are prescribed at hourly intervals as the map time intervals. The input is summarized as follows:

Control Card: 0 in column 1

Block 0: see Section IV,1, NOWIND = -1

Blocks 1 to 9 see Section IV,2

Block 12 (1 card for steady river discharges)

READ 250 , IDENT, (RO(K,M), K = 1, IMRO)

Astrotide Block (1 card for each 12 hours)

905 READ 910, IGA, MTIME, (H(1,J), J = 1,12)

MU = MTIME + 12

IF (MU.LT.MMAX) GO TO 905

910 FORMAT (I2, I4, 12F 6.2)

(IGA = 1)

Blocks 18 to 20: see Section IV,2

Comments on Tide Mode. The map time interval for the tide mode is 1 hour. The MTIME entry for the astrotide block is the time (hour) of the first of 12-hourly values of HG (entered as H(1,J)). The tide is assumed uniform along the seaward boundary of the bay system, hence one HG value per hour is sufficient.

In starting the tide mode from rest state ($U = V = 0$ and $H = HGI = 0$), usually one or two diurnal tide cycles are required for the numerical model to reach a nearly periodic response to an almost periodic input. Thus, if the final diurnal cycle is to be free of initial transients, at least 72 hours of HG data should be provided. This may require an adjustment in the dimensions given in COMMON/BLK6/ which appears in subprograms MAIN, PART 2, and CONTIN, if the full data set is to be stored for one run. An alternative is to make use of the continuation option, using less data input per run (e.g., 24 hours).

4. Input for Continuation of a Run.

Since the main purpose of the tide mode is for calibration of the bed friction coefficients for blocks and channels, it is expected that many trial runs will be made for a given bay system. In order to keep the machine time to a minimum for each successive run, it is desirable to use an initial field of U , V , or H which is close to the true response at the starting time. This can be accomplished by using the resulting U , V , and H arrays from a previous tide run for the bay system as the initial values. (This should be done even if the previous run has different values of the bed friction coefficients.) The mechanism for

accomplishing this is the use of the continuation mode option, as controlled by ICARD. In this mode, the contents of common from a previous run are input along with any additional forcing function data.

To make the program as flexible as possible, the continuation option can be used for either storm surge problems or astrotide problems, the only difference in input being in the type of forcing function input. Such forcing function data should be consistent with the continuation time. Moreover, the value of NTIME input in data block 1 should be equal to the final NTIME in the previous run which is continued.

The sequence of input for continuation of a problem is as follows:

- Control Card:* 1 in column 1
- Contin Deck:* Contents of COMMON output from a previous run
- Blocks 0 to 3:* see subsection 2 of this app. (4 cards)
- Forcing Deck:* For storm surge mode, blocks 13 to 17, inclusive.
For tide mode-astrotide deck.

A flow diagram summarizing the READ operations as controlled by ICARD and NOWIND is given in Figure C-1.

5. Comments on Barrier Input.

a. Possible Barrier Locations. All barriers in the schematization occur parallel to the sides of a given barrier block. Barrier data qualified by an X in the coded name (e.g., IZX, ICDOX, ICDXS) refer to barriers normal to the x-axis on the right side of the barrier block; those qualified by a Y in the coded name (e.g., IXY, etc.) refer to barriers normal to the y-axis on the upper side of the barrier block. If a channel exists parallel to either barrier, then such a barrier may occur on either or both sides of the parallel channel, depending upon the coding of the associated channel input data (as discussed in a subsequent subsection). Barriers which might exist along the left or lower side of a given block are represented by appropriate data coding of a barrier block in a previous row or column.

b. Precaution. It should be emphasized that for any barrier block it is up to the user to supply appropriate barrier elevations ZB for both the right and upper sides of the barrier block even if a real barrier occurs only on one side of the block. The important point to observe is that the specified ZB values should always equal or exceed the larger of the block elevations at or adjacent to the side of the barrier block in question. Otherwise, errors can occur in the computations.

c. Array Size. The number of barrier blocks KM is normally limited to less than 100.

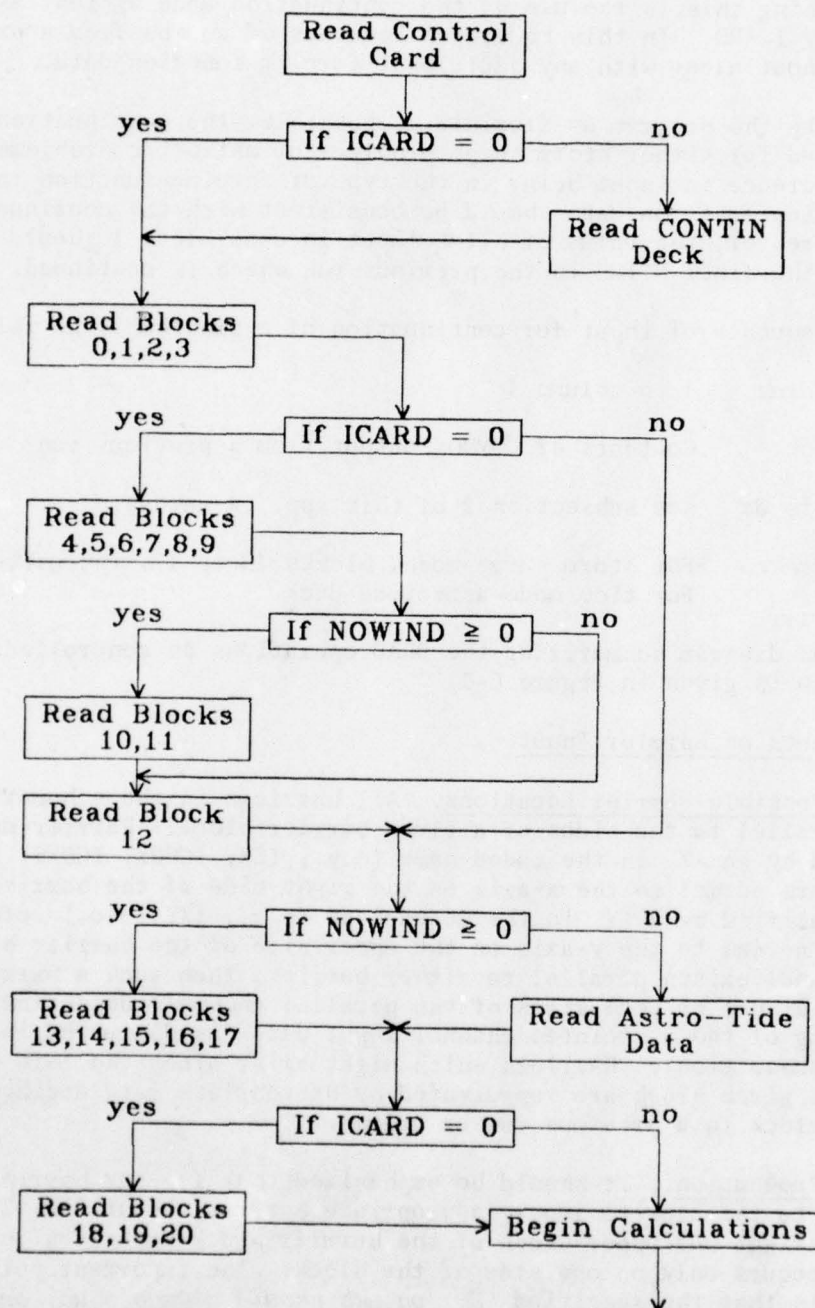


Figure C-1. Flow diagram for read statements.

6. Comments on Channel Input.

a. Possible Channel Locations. All channels in the schematization occur along the right side or the upper side of a given channel block. Channel data qualified by an X in the coded name (e.g., IWCX, IZCX) refer to channels normal to the X-axis on the right side of the channel block; those qualified by a Y in the coded name (e.g., IWCY, IZCY) refer to channels normal to the Y-axis on the upper side of the channel block. If a block has both an X and Y channel, one data card specifies both.

b. Channel Junctions. In the schematization of a channel system junctions can occur with adjoining channel reaches parallel to each other or perpendicular. Moreover, one-, two-, or three-way branches are possible.

Four possible right-angle channel junctions are illustrated in Figure C-2. The simplest junction is that shown in the upper right panel of the figure where the joining channel reaches are in the same channel block K1. Right-angle junctions involving two adjacent channel blocks are illustrated in the upper left and lower right panels of Figure C-2.

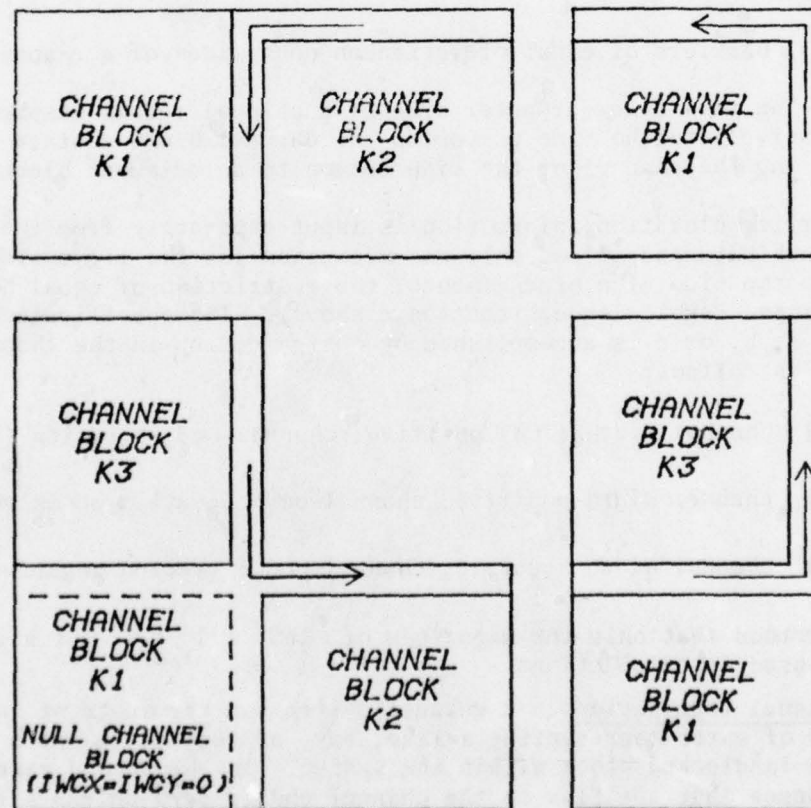


Figure C-2. Four possible simple bends for a channel reach.

The final possible right turn is illustrated in the lower left panel of the figure. In this case, the program requires that a channel block (K1) join the connecting channels of the nonadjoining channel blocks (K2 and K3) even though no channels exist on the joining block K1. In such circumstances, the required "null" channel block would have zero width for both the X and Y channels ($IWCS = IWCY = 0$) as input. The H value at the junction of the connecting channel reaches for this case is stored as $HC(K1)$; i.e., in association with the null channel block.

Collinear adjoining channels always involve two adjacent channel blocks. Four possible junctions of this type are illustrated in Figure B-2 in relation to central channel block K.

c. Channels with Levees. The program allows for the following possible situations with respect to barriers parallel to channels:

- (a) Single barrier on the "inner" lateral boundary of a channel;
- (b) single barrier on the "outer" lateral boundary of a channel;
- (c) barriers of *equal* elevation on both sides of a channel.

NOTE.--The term inner or outer side of a channel refers respectively to the side common to the channel block containing the channel or the side common to an adjacent block.

The barrier elevation information is input separately from the channel block data and allows only one elevation for the right side and one for the top side of a block (hence, the restriction of equal barrier heights for the double levee situation c above). The specification for situations a, b, or c is accomplished by a sign coding in the channel block data as follows:

- (a) Channel width (ICW) positive, channel-bed elevation (IZC)
- (b) channel width positive, channel-bed elevation positive;
- (c) channel width negative, channel-bed elevation negative.

It is understood that only the magnitude of IWC and IZC for a given channel is used in calculations.

d. Channel Terminations. A channel system can terminate at (a) a larger body of water representing a lake, bay, or sea; or (b) at a boundary or in a landlocked block within the system. In the second case, the program assumes that the flow at the channel end is zero unless a river discharge to the channel is specified (see input) and that the channel end block is one block inside the boundary block.

e. Restriction. Only channels with the channel bed below the mean water level (MWL) reference are allowed. The actual elevation used in calculations is $-|IZC|$, regardless of the sign on the input of IZC for a given channel.

f. Array Size. The number of channel blocks (including null channel blocks) is KCM. However, (CHANL(1)) creates arrays of length KCMP > KCM. The value of KCMP exceeds KCM by one plus the number of channels which terminate on the exterior boundary of the grid including the seaward boundary. Since KCMP is limited to 130, KCM should be less by the amount described above.

7. Output.

a. Listings of Input and Key Arrays. All input data are listed in easily identifiable form in the order in which the data are entered through block 18. Immediately following the basic channel input is a listing of the key arrays for channels, as discussed in Appendix A, including the assignment of sign coding for ICG and JCG.

Also printed out, in the same block format as the routine listings of H, are the block elevations.

b. Sequential Output. Normally, the routine output of computed values includes block H arrays and listings of all channel variables at pre-determined intervals of time (as determined by IOUT). It is possible to list the U, V, and H arrays for blocks by changing the CALL LIST(2) statement following statement 2,100 in PART 2 to CALL LIST(3).

For channel listings, refer to Figure 6 for notation; the listings are ordered by channel block number K. The block location I,J is repeated (negative signs indicating end points). This is followed by HX, the water level (feet) and QXN, the volume transport (cubic feet per second) at the lower end of the x channel, then QCP, the transport at the upper end of the x channel. These are followed by HY, QYN, and QYP representing, respectively, the water level and flow at the left end and flow at the right end of the y channel. Next is HC, the water level at the junction of the x and y channels. The last four entries in the channel listings are the transports (in cubic feet per second) to the channel from the channel block and from the channel to an adjacent block for the x and y channels. The HC value is meaningful for null channels only.

c. Saved Time Sequences. Subroutine SAVE, if used, saves sequences of water level and flow at preselected locations (as identified in block 19 of the input). In the original version of the subroutine used with an IBM 360-65 computer the saved information was punched on cards to facilitate later graphing of the sequences.

APPENDIX D

COMPLETE DATA LISTING OF INPUT FOR
SABINE-CALCASIEU REGION WITH
FORCING DATA FOR HURRICANE CARLA

ICARD# 0 IBL# 1 KCM# 121 NOWIND# 1 INTER# 15 NGAGE# 9 NFLOW# 2 IMIN# -1 IMAX# 10
 IDNT# 1 NTIME# 0 NM# 900 MMIN# 0 MMAX# 24 NFU# 45 IOUT# 449 INFLD# 0
 IDNT# 2 IM# 28 JM# 20 KM# 91 KMAX# 8 LMAX# 6
 IDNT# 3 DELX# 2.0 N MI DELT#240. SEC CDO# .200 FK# .0010 FC# .0010 HGI# 3.200 FT
 IDNT# 4 KI# 4 LJ# 4 KII# 7 LJJ# 5 JBL# 2 JBR# 1

MIN OR MMAX IN ERROR

IDNT# 5 BARRIER DATA= Z VALUES IN TENTHS OF FEET. CD VALUES ARE TIMES 1000

K# 1	IM# 8	JR# 1	ZX# 50	ZY# 10	CDOX# 200	CDUY# 200	CDSX# 400	CDSY# 400
K# 2	IM# 20	JR# 1	ZX# -150	ZY# 60	CDOX# 200	CDUY# 200	CDSX# 400	CDSY# 400
K# 3	IM# 21	JR# 1	ZX# -150	ZY# 60	CDOX# 200	CDUY# 200	CDSX# 400	CDSY# 400
K# 4	IM# 22	JR# 1	ZX# 50	ZY# 60	CDOX# 200	CDUY# 200	CDSX# 400	CDSY# 400
K# 5	IM# 23	JR# 1	ZX# -80	ZY# 50	CDOX# 200	CDUY# 200	CDSX# 400	CDSY# 400
K# 6	IM# 24	JR# 1	ZX# -100	ZY# 50	CDOX# 200	CDUY# 200	CDSX# 400	CDSY# 400
K# 7	IM# 25	JR# 1	ZX# -100	ZY# 50	CDOX# 200	CDUY# 200	CDSX# 400	CDSY# 400
K# 8	IM# 26	JR# 1	ZX# -100	ZY# 50	CDOX# 200	CDUY# 200	CDSX# 400	CDSY# 400
K# 9	IM# 27	JR# 1	ZX# -100	ZY# 50	CDOX# 200	CDUY# 200	CDSX# 400	CDSY# 400
K# 10	IM# 28	JR# 1	ZX# -100	ZY# 50	CDOX# 200	CDUY# 200	CDSX# 400	CDSY# 400
K# 11	IM# 1	JR# 2	ZX# -80	ZY# 60	CDOX# 200	CDUY# 200	CDSX# 400	CDSY# 400
K# 12	IM# 2	JR# 2	ZX# -100	ZY# 60	CDOX# 200	CDUY# 200	CDSX# 400	CDSY# 400
K# 13	IM# 3	JR# 2	ZX# -120	ZY# 60	CDOX# 200	CDUY# 200	CDSX# 400	CDSY# 400
K# 14	IM# 4	JR# 2	ZX# -100	ZY# 60	CDOX# 200	CDUY# 200	CDSX# 400	CDSY# 400
K# 15	IM# 5	JR# 2	ZX# -70	ZY# 60	CDOX# 200	CDUY# 200	CDSX# 400	CDSY# 400
K# 16	IM# 6	JR# 2	ZX# 10	ZY# 60	CDOX# 200	CDUY# 200	CDSX# 400	CDSY# 400
K# 17	IM# 8	JR# 2	ZX# 50	ZY# 20	CDOX# 200	CDUY# 200	CDSX# 400	CDSY# 400
K# 18	IM# 13	JR# 2	ZX# -130	ZY# 50	CDOX# 200	CDUY# 200	CDSX# 400	CDSY# 400
K# 19	IM# 14	JR# 2	ZX# -120	ZY# 50	CDOX# 200	CDUY# 200	CDSX# 400	CDSY# 400
K# 20	IM# 15	JR# 2	ZX# -120	ZY# 50	CDOX# 200	CDUY# 200	CDSX# 400	CDSY# 400
K# 21	IM# 16	JR# 2	ZX# -120	ZY# 50	CDOX# 200	CDUY# 200	CDSX# 400	CDSY# 400
K# 22	IM# 17	JR# 2	ZX# -110	ZY# 50	CDOX# 200	CDUY# 200	CDSX# 400	CDSY# 400
K# 23	IM# 18	JR# 2	ZX# -80	ZY# 50	CDOX# 200	CDUY# 200	CDSX# 400	CDSY# 400
K# 24	IM# 19	JR# 2	ZX# 60	ZY# 40	CDOX# 200	CDUY# 200	CDSX# 400	CDSY# 400
K# 25	IM# 22	JR# 2	ZX# 80	ZY# 10	CDOX# 200	CDUY# 200	CDSX# 400	CDSY# 400
K# 26	IM# 6	JR# 3	ZX# 60	ZY# 10	CDOX# 200	CDUY# 200	CDSX# 400	CDSY# 400
K# 27	IM# 7	JR# 3	ZX# 20	ZY# 50	CDOX# 200	CDUY# 200	CDSX# 400	CDSY# 400
K# 28	IM# 8	JR# 3	ZX# 50	ZY# 50	CDOX# 200	CDUY# 200	CDSX# 400	CDSY# 400
K# 29	IM# 9	JR# 3	ZX# 10	ZY# 40	CDOX# 200	CDUY# 200	CDSX# 400	CDSY# 400
K# 30	IM# 10	JR# 3	ZX# 10	ZY# 40	CDOX# 200	CDUY# 200	CDSX# 400	CDSY# 400
K# 31	IM# 11	JR# 3	ZX# 30	ZY# 40	CDOX# 200	CDUY# 200	CDSX# 400	CDSY# 400
K# 32	IM# 12	JR# 3	ZX# 50	ZY# 50	CDOX# 200	CDUY# 200	CDSX# 400	CDSY# 400
K# 33	IM# 22	JR# 3	ZX# 80	ZY# 10	CDOX# 200	CDUY# 200	CDSX# 400	CDSY# 400
K# 34	IM# 1	JR# 4	ZX# 10	ZY# 50	CDOX# 200	CDUY# 200	CDSX# 400	CDSY# 400
K# 35	IM# 2	JR# 4	ZX# 10	ZY# 50	CDOX# 200	CDUY# 200	CDSX# 400	CDSY# 400
K# 36	IM# 3	JR# 4	ZX# 10	ZY# 50	CDOX# 200	CDUY# 200	CDSX# 400	CDSY# 400
K# 37	IM# 7	JR# 4	ZX# 50	ZY# 50	CDOX# 200	CDUY# 200	CDSX# 400	CDSY# 400
K# 38	IM# 22	JR# 4	ZX# 30	ZY# -30	CDOX# 200	CDUY# 200	CDSX# 400	CDSY# 400
K# 39	IM# 3	JR# 5	ZX# 50	ZY# 10	CDOX# 200	CDUY# 200	CDSX# 400	CDSY# 400
K# 40	IM# 4	JR# 5	ZX# 10	ZY# 50	CDOX# 200	CDUY# 200	CDSX# 400	CDSY# 400
K# 41	IM# 5	JR# 5	ZX# 10	ZY# 30	CDOX# 200	CDUY# 200	CDSX# 400	CDSY# 400
K# 42	IM# 4	JR# 6	ZX# 30	ZY# 10	CDOX# 200	CDUY# 200	CDSX# 400	CDSY# 400
K# 43	IM# 6	JR# 5	ZX# 50	ZY# 10	CDOX# 200	CDUY# 200	CDSX# 400	CDSY# 400

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K# 44	I# 5	J# 6	Z# 30	Z# 30	CDOX# 200	CDOY# 200	CDSX# 400	CDSY# 400
K# 45	I# 6	J# 6	Z# 50	Z# 50	CDOX# 200	CDOY# 200	CDSX# 400	CDSY# 400
K# 46	I# 15	J# 6	Z# 10	Z# 10	CDOX# 200	CDOY# 200	CDSX# 400	CDSY# 400
K# 47	I# 16	J# 6	Z# 10	Z# 10	CDOX# 200	CDOY# 200	CDSX# 400	CDSY# 400
K# 48	I# 17	J# 6	Z# 0	Z# 0	CDOX# 200	CDOY# 200	CDSX# 400	CDSY# 400
K# 49	I# 23	J# 6	Z# 100	Z# 100	CDOX# 200	CDOY# 200	CDSX# 400	CDSY# 400
K# 50	I# 4	J# 7	Z# 30	Z# 30	CDOX# 200	CDOY# 200	CDSX# 400	CDSY# 400
K# 51	I# 6	J# 7	Z# 50	Z# 50	CDOX# 200	CDOY# 200	CDSX# 400	CDSY# 400
K# 52	I# 5	J# 7	Z# 30	Z# 30	CDOX# 200	CDOY# 200	CDSX# 400	CDSY# 400
K# 53	I# 7	J# 7	Z# -40	Z# -40	CDOX# 200	CDOY# 200	CDSX# 400	CDSY# 400
K# 54	I# 8	J# 7	Z# -40	Z# -40	CDOX# 200	CDOY# 200	CDSX# 400	CDSY# 400
K# 55	I# 14	J# 7	Z# 50	Z# 50	CDOX# 200	CDOY# 200	CDSX# 400	CDSY# 400
K# 56	I# 17	J# 7	Z# 40	Z# 40	CDOX# 200	CDOY# 200	CDSX# 400	CDSY# 400
K# 57	I# 23	J# 7	Z# 100	Z# 100	CDOX# 200	CDOY# 200	CDSX# 400	CDSY# 400
K# 58	I# 4	J# 8	Z# 30	Z# 30	CDOX# 200	CDOY# 200	CDSX# 400	CDSY# 400
K# 59	I# 5	J# 8	Z# 50	Z# 50	CDOX# 200	CDOY# 200	CDSX# 400	CDSY# 400
K# 60	I# 6	J# 8	Z# 50	Z# 50	CDOX# 200	CDOY# 200	CDSX# 400	CDSY# 400
K# 61	I# 7	J# 8	Z# 50	Z# 50	CDOX# 200	CDOY# 200	CDSX# 400	CDSY# 400
K# 62	I# 8	J# 8	Z# 140	Z# 140	CDOX# 200	CDOY# 200	CDSX# 400	CDSY# 400
K# 63	I# 9	J# 8	Z# -40	Z# -40	CDOX# 200	CDOY# 200	CDSX# 400	CDSY# 400
K# 64	I# 14	J# 8	Z# 50	Z# 50	CDOX# 200	CDOY# 200	CDSX# 400	CDSY# 400
K# 65	I# 15	J# 8	Z# 0	Z# 0	CDOX# 200	CDOY# 200	CDSX# 400	CDSY# 400
K# 66	I# 16	J# 8	Z# 0	Z# 0	CDOX# 200	CDOY# 200	CDSX# 400	CDSY# 400
K# 67	I# 17	J# 8	Z# 50	Z# 50	CDOX# 200	CDOY# 200	CDSX# 400	CDSY# 400
K# 68	I# 23	J# 8	Z# 120	Z# 120	CDOX# 200	CDOY# 200	CDSX# 400	CDSY# 400
K# 69	I# 6	J# 9	Z# 50	Z# 50	CDOX# 200	CDOY# 200	CDSX# 400	CDSY# 400
K# 70	I# 7	J# 9	Z# 50	Z# 50	CDOX# 200	CDOY# 200	CDSX# 400	CDSY# 400
K# 71	I# 9	J# 9	Z# 140	Z# 140	CDOX# 200	CDOY# 200	CDSX# 400	CDSY# 400
K# 72	I# 23	J# 9	Z# 120	Z# 120	CDOX# 200	CDOY# 200	CDSX# 400	CDSY# 400
K# 73	I# 9	J# 10	Z# 140	Z# 140	CDOX# 200	CDOY# 200	CDSX# 400	CDSY# 400
K# 74	I# 20	J# 10	Z# 10	Z# 10	CDOX# 200	CDOY# 200	CDSX# 400	CDSY# 400
K# 75	I# 21	J# 10	Z# 10	Z# 10	CDOX# 200	CDOY# 200	CDSX# 400	CDSY# 400
K# 76	I# 22	J# 10	Z# 10	Z# 10	CDOX# 200	CDOY# 200	CDSX# 400	CDSY# 400
K# 77	I# 23	J# 10	Z# 120	Z# 120	CDOX# 200	CDOY# 200	CDSX# 400	CDSY# 400
K# 78	I# 15	J# 11	Z# 20	Z# 20	CDOX# 200	CDOY# 200	CDSX# 400	CDSY# 400
K# 79	I# 16	J# 11	Z# 10	Z# 10	CDOX# 200	CDOY# 200	CDSX# 400	CDSY# 400
K# 80	I# 17	J# 11	Z# 20	Z# 20	CDOX# 200	CDOY# 200	CDSX# 400	CDSY# 400
K# 81	I# 18	J# 11	Z# 50	Z# 50	CDOX# 200	CDOY# 200	CDSX# 400	CDSY# 400
K# 82	I# 14	J# 11	Z# 100	Z# 100	CDOX# 200	CDOY# 200	CDSX# 400	CDSY# 400
K# 83	I# 6	J# 12	Z# 80	Z# 80	CDOX# 200	CDOY# 200	CDSX# 400	CDSY# 400
K# 84	I# 7	J# 12	Z# 50	Z# 50	CDOX# 200	CDOY# 200	CDSX# 400	CDSY# 400
K# 85	I# 14	J# 12	Z# 20	Z# 20	CDOX# 200	CDOY# 200	CDSX# 400	CDSY# 400
K# 86	I# 5	J# 13	Z# 50	Z# 50	CDOX# 200	CDOY# 200	CDSX# 400	CDSY# 400
K# 87	I# 13	J# 13	Z# 50	Z# 50	CDOX# 200	CDOY# 200	CDSX# 400	CDSY# 400
K# 88	I# 14	J# 13	Z# 30	Z# 30	CDOX# 200	CDOY# 200	CDSX# 400	CDSY# 400
K# 89	I# 5	J# 14	Z# 10	Z# 10	CDOX# 200	CDOY# 200	CDSX# 400	CDSY# 400
K# 90	I# 5	J# 16	Z# 50	Z# 50	CDOX# 200	CDOY# 200	CDSX# 400	CDSY# 400
K# 91	I# 4	J# 17	Z# 50	Z# 50	CDOX# 200	CDOY# 200	CDSX# 400	CDSY# 400

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IDNT# 6		BLOCK TOPOGRAPHY- Z VALUES IN FEET									
1	-24	-8	1	1	2	3	3	8	7	16	
1	11	19	23	24	32	35	30	20	35	100	
2	-24	-10	0	1	2	2	3	8	8	11	
2	7	14	15	23	25	35	30	20	35	100	
3	-24	-13	1	1	1	1	1	2	6	9	
3	4	7	11	16	22	25	30	7	7	100	
4	-24	-12	1	1	1	0	1	1	7	4	
4	12	13	14	20	23	15	3	7	7	100	
5	-24	-10	-1	0	1	-1	-1	-1	5	8	
5	15	17	15	1	1	1	7	7	7	100	
6	-18	-7	1	1	0	1	2	3	3	3	
6	15	7	1	1	7	1	10	20	25	100	
7	-8	1	1	1	-5	-5	-4	5	-1	5	
7	13	1	3	18	18	15	20	25	25	100	
8	-4	1	2	0	1	-5	-6	2	5	12	
8	12	1	6	15	18	20	25	25	30	100	
9	-15	2	2	1	1	-6	-7	-6	2	7	
9	1	1	13	15	17	15	25	27	30	100	
10	-20	-8	1	1	1	-5	-8	-8	-6	-4	
10	1	7	11	12	18	21	22	28	30	100	
11	-22	-10	1	3	1	1	-6	-7	-7	-6	
11	1	7	8	11	18	20	22	30	35	100	
12	-22	-13	3	2	1	1	1	1	-4	-3	
12	4	6	12	12	16	20	22	25	30	100	
13	-23	-15	5	2	1	1	1	1	1	1	
13	1	8	10	12	13	18	22	30	32	100	
14	-23	-13	4	1	1	1	1	1	1	1	
14	1	1	3	7	13	18	20	25	20	100	
15	-23	-12	2	1	1	1	0	0	1	1	
15	2	2	1	2	3	7	7	10	12	100	
16	-23	-12	1	1	1	1	0	0	1	1	
16	1	1	3	8	8	10	10	15	15	100	
17	-23	-12	1	1	1	0	0	0	1	3	
17	1	1	1	9	16	20	15	25	30	100	
18	-22	-11	1	1	1	0	0	1	3	3	
18	2	2	4	10	15	18	20	25	30	100	
19	-10	-8	-1	1	1	1	1	1	1	4	
19	5	8	8	11	14	18	20	20	32	100	
20	-17	1	1	1	1	1	1	1	1	1	
20	3	8	10	14	14	18	20	22	25	100	
21	-15	1	1	1	-4	1	0	7	-1	1	
21	3	6	10	12	14	18	20	22	25	100	
22	-15	1	1	-3	-4	1	1	5	0	1	
22	5	6	10	12	14	18	20	22	25	100	
23	-8	1	1	-4	-5	1	1	5	1	1	
23	4	6	12	13	14	18	20	22	25	100	
24	-10	1	1	-6	-7	-7	-7	-6	-6	1	
24	3	1	10	12	15	15	20	22	25	100	
25	-10	1	1	-5	-6	-5	-2	-6	1	1	
25	5	8	8	12	15	15	20	22	25	100	
26	-10	1	1	1	1	1	1	1	1	1	
26	4	8	1	-5	15	15	20	22	25	100	
27	-10	1	1	1	1	1	1	1	1	1	
27	4	8	8	1	12	15	20	22	25	100	
28	-10	100	100	100	100	100	100	100	100	100	
28	100	100	100	100	100	100	100	100	100	100	

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IDNT# 7 IMRO# 3 JMRD# 25 KF# 25 ISTR# 25 IND# 16 NUW# 45 KIM# 45 NORT# 15
 IDNT# 8 RF# 4.000 CONST# .9000 S# .23662

IDNT# 9 3 PAIRS OF I,J FOR RUNOFF LOCATIONS
 9 28 15 4 19 14 19 0 0 0 0

IDNT# 10 PERCENT RAINFALL EACH MAPTIME
 0 .006 .0070 .0070 .0080 .0100 .0140 .0180 .0230 .0240 .0280
 0 .032 .0360 .0470 .0630 .0840 .1070 .1460 .1400 .0650 .0390
 0 .030 .0250 .0210 .0200

IDNT# 11 CHANNEL STRESS VALUES AT MAPTIMES
 1 0.000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000
 1 0.000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000
 1 0.000 0.0000 0.0000 0.0000 0.0000

IDNT# 12 3 SETS OF RUNOFF VALUES IN CFS FOR 25 MAPTIMES
 2 800. 1107. 1520.
 2 800. 1099. 1480.
 2 800. 1099. 1480.
 2 800. 1099. 1480.
 2 800. 1099. 1480.
 2 800. 1099. 1480.
 2 800. 1099. 1480.
 2 800. 1099. 1480.
 2 800. 1099. 1480.
 2 800. 1099. 1480.
 2 900. 1152. 1560.
 2 900. 1152. 1560.
 2 900. 1152. 1560.
 2 900. 1152. 1560.
 2 900. 1152. 1560.
 2 900. 1152. 1560.
 2 900. 1152. 1560.
 2 900. 1152. 1560.
 2 900. 1152. 1560.
 2 1310. 1812. 2060.
 2 1310. 1812. 2060.
 2 1310. 1812. 2060.
 2 1310. 1812. 2060.
 2 1310. 1812. 2060.
 2 1310. 1812. 2060.
 2 1310. 1812. 2060.
 2 1310. 1812. 2060.
 2 1310. 1812. 2060.

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HGR FOLLOWED BY HGR ARRAY (IN FEET) AT MTIME= 0
 4 4.500 4.5000 4.5000 4.5000 4.5000 4.5000 4.5000 4.5000
 5 3.200 3.2000 3.2000 3.2000 3.2000 3.2000 3.2000 3.2000

XR VALUES (IDNT=6) AND YR VALUES (IDNT=7) AT MPTIME= 0
 6 -.029 -.0101 -.0090 -.0080 -.0062 -.0054
 6 -.027 -.0164 -.0091 -.0081 -.0063 -.0055
 6 -.030 -.0128 -.0100 -.0073 -.0064 -.0055
 6 -.028 -.0143 -.0116 -.0073 -.0064 -.0055
 6 -.028 -.0130 -.0105 -.0073 -.0064 -.0056
 6 -.026 -.0225 -.0094 -.0074 -.0065 -.0056
 6 -.026 -.0225 -.0094 -.0074 -.0065 -.0056
 6 -.026 -.0225 -.0094 -.0074 -.0065 -.0056
 7 -.011 -.0037 -.0033 -.0029 -.0023 -.0020
 7 -.009 -.0055 -.0030 -.0026 -.0020 -.0018
 7 -.009 -.0037 -.0030 -.0020 -.0017 -.0015
 7 -.007 -.0036 -.0031 -.0020 -.0017 -.0015
 7 -.006 -.0030 -.0024 -.0017 -.0015 -.0013
 7 -.005 -.0044 -.0018 -.0014 -.0013 -.0011
 7 -.005 -.0044 -.0018 -.0014 -.0013 -.0011
 7 -.005 -.0044 -.0018 -.0014 -.0013 -.0011

HGR FOLLOWED BY HGR ARRAY (IN FEET) AT MTIME= 1
 4 5.500 5.5000 5.5000 5.5000 5.5000 5.5000 5.5000 5.5000
 5 4.500 4.5000 4.5000 4.5000 4.5000 4.5000 4.5000 4.5000

XR VALUES (IDNT=6) AND YR VALUES (IDNT=7) AT MPTIME= 1
 6 -.033 -.0112 -.0112 -.0090 -.0070 -.0054
 6 -.031 -.0200 -.0102 -.0091 -.0071 -.0054
 6 -.032 -.0141 -.0120 -.0091 -.0071 -.0055
 6 -.032 -.0172 -.0142 -.0092 -.0072 -.0055
 6 -.032 -.0144 -.0117 -.0093 -.0073 -.0055
 6 -.030 -.0244 -.0118 -.0093 -.0073 -.0064
 6 -.030 -.0244 -.0118 -.0093 -.0073 -.0064
 6 -.030 -.0244 -.0118 -.0093 -.0073 -.0064
 7 -.013 -.0043 -.0043 -.0035 -.0027 -.0021
 7 -.011 -.0059 -.0035 -.0031 -.0025 -.0019
 7 -.010 -.0043 -.0038 -.0030 -.0023 -.0018
 7 -.009 -.0046 -.0041 -.0027 -.0022 -.0017
 7 -.008 -.0033 -.0029 -.0023 -.0020 -.0015
 7 -.006 -.0048 -.0025 -.0018 -.0017 -.0015
 7 -.006 -.0048 -.0025 -.0018 -.0017 -.0015
 7 -.006 -.0048 -.0025 -.0018 -.0017 -.0015

HGR FOLLOWED BY HGR ARRAY (IN FEET) AT MTIME= 2
 4 6.200 6.2000 6.2000 6.2000 6.2000 6.2000 6.2000 6.2000
 5 5.200 5.2000 5.2000 5.2000 5.2000 5.2000 5.2000

XR VALUES(IDNT=6) AND YR VALUES (IDNT=7) AT MPTIME= 2
 6 -.037 -.0136 -.0123 -.0111 -.0098 -.0053
 6 -.036 -.0216 -.0112 -.0101 -.0090 -.0080
 6 -.036 -.0169 -.0130 -.0102 -.0091 -.0081
 6 -.036 -.0187 -.0155 -.0103 -.0091 -.0081
 6 -.034 -.0143 -.0142 -.0104 -.0092 -.0082
 6 -.035 -.0282 -.0144 -.0094 -.0083 -.0073
 6 -.035 -.0282 -.0144 -.0094 -.0083 -.0073
 7 -.015 -.0056 -.0052 -.0047 -.0038 -.0022
 7 -.013 -.0079 -.0043 -.0039 -.0035 -.0031
 7 -.012 -.0055 -.0045 -.0035 -.0031 -.0028
 7 -.011 -.0054 -.0048 -.0032 -.0030 -.0026
 7 -.009 -.0036 -.0038 -.0028 -.0027 -.0024
 7 -.007 -.0060 -.0033 -.0022 -.0021 -.0018
 7 -.007 -.0060 -.0033 -.0022 -.0021 -.0018

HGR FOLLOWED BY HGR ARRAY (IN FEET) AT MTIME= 3
 4 6.000 6.0000 6.0000 6.0000 6.0000 6.0000 6.0000 6.0000
 5 5.200 5.2000 5.2000 5.2000 5.2000 5.2000 5.2000

XR VALUES(IDNT=6) AND YR VALUES (IDNT=7) AT MPTIME= 3
 6 -.040 -.0138 -.0125 -.0112 -.0100 -.0089
 6 -.038 -.0201 -.0126 -.0102 -.0091 -.0081
 6 -.039 -.0203 -.0130 -.0103 -.0092 -.0081
 6 -.037 -.0205 -.0142 -.0104 -.0092 -.0082
 6 -.035 -.0174 -.0158 -.0105 -.0083 -.0073
 6 -.033 -.0305 -.0145 -.0094 -.0083 -.0074
 6 -.033 -.0305 -.0145 -.0094 -.0083 -.0074
 6 -.033 -.0305 -.0145 -.0094 -.0083 -.0074
 7 -.015 -.0053 -.0048 -.0043 -.0040 -.0036
 7 -.013 -.0066 -.0044 -.0035 -.0031 -.0028
 7 -.011 -.0059 -.0062 -.0032 -.0028 -.0025
 7 -.009 -.0051 -.0038 -.0028 -.0027 -.0024
 7 -.007 -.0037 -.0037 -.0024 -.0021 -.0018
 7 -.005 -.0054 -.0026 -.0018 -.0018 -.0016
 7 -.005 -.0054 -.0026 -.0018 -.0018 -.0016
 7 -.005 -.0054 -.0026 -.0018 -.0018 -.0016

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HGR FOLLOWED BY HBR ARRAY (IN FEET) AT MTIME# 4

4	5.800	5.8000	5.8000	5.8000	5.8000	5.8000	5.8000	5.8000
5	5.200	5.2000	5.2000	5.2000	5.2000	5.2000	5.2000	5.2000

XR VALUES (IDNT#6) AND YR VALUES (IDNT#7) AT MPTIME# 4

6	-.052	-.0205	-.0172	-.0156	-.0142	-.0128
6	-.053	-.0282	-.0173	-.0158	-.0130	-.0117
6	-.050	-.0226	-.0175	-.0145	-.0131	-.0118
6	-.048	-.0247	-.0176	-.0146	-.0118	-.0095
6	-.045	-.0229	-.0211	-.0133	-.0119	-.0106
6	-.043	-.0378	-.0194	-.0133	-.0107	-.0085
6	-.043	-.0378	-.0194	-.0133	-.0107	-.0085
6	-.043	-.0378	-.0194	-.0133	-.0107	-.0085
7	-.012	-.0051	-.0046	-.0045	-.0041	-.0039
7	-.010	-.0060	-.0040	-.0037	-.0032	-.0029
7	-.008	-.0040	-.0047	-.0028	-.0028	-.0025
7	-.006	-.0031	-.0025	-.0023	-.0021	-.0017
7	-.003	-.0020	-.0022	-.0016	-.0017	-.0017
7	-.002	-.0020	-.0014	-.0012	-.0011	-.0010
7	-.002	-.0020	-.0014	-.0012	-.0011	-.0010
7	-.002	-.0020	-.0014	-.0012	-.0011	-.0010

HGR FOLLOWED BY HBR ARRAY (IN FEET) AT MTIME# 5

4	6.200	6.2000	6.2000	6.2000	6.2000	6.2000	6.2000	6.2000
5	5.500	5.5000	5.5000	5.5000	5.5000	5.5000	5.5000	5.5000

XR VALUES (IDNT#6) AND YR VALUES (IDNT#7) AT MPTIME# 5

6	-.065	-.0244	-.0225	-.0189	-.0173	-.0158
6	-.062	-.0373	-.0208	-.0192	-.0159	-.0131
6	-.059	-.0308	-.0210	-.0176	-.0160	-.0145
6	-.057	-.0248	-.0229	-.0177	-.0161	-.0146
6	-.057	-.0268	-.0248	-.0162	-.0147	-.0133
6	-.051	-.0454	-.0212	-.0162	-.0147	-.0133
6	-.051	-.0454	-.0212	-.0162	-.0147	-.0133
6	-.051	-.0454	-.0212	-.0162	-.0147	-.0133
7	-.012	-.0048	-.0048	-.0044	-.0040	-.0039
7	-.009	-.0060	-.0037	-.0034	-.0031	-.0026
7	-.005	-.0033	-.0043	-.0028	-.0026	-.0026
7	-.003	-.0018	-.0020	-.0019	-.0020	-.0021
7	-.001	-.0010	-.0013	-.0011	-.0013	-.0014
7	.001	0.0000	-.0004	-.0006	-.0008	-.0009
7	.001	0.0000	-.0004	-.0006	-.0008	-.0009
7	.001	0.0000	-.0004	-.0006	-.0008	-.0009

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HGR FOLLOWED BY HGR ARRAY (IN FEET) AT MTIME= 6
 4 6.500 6.5000 6.5000 6.5000 6.5000 6.5000 6.5000 6.5000
 5 5.600 5.6000 5.6000 5.6000 5.6000 5.6000 5.6000

XR VALUES (IDNT=6) AND YR VALUES (IDNT=7) AT MPTIME= 6
 6 -.069 -.0248 -.0211 -.0193 -.0161 -.0146
 6 -.063 -.0332 -.0211 -.0177 -.0162 -.0146
 6 -.060 -.0258 -.0220 -.0162 -.0147 -.0133
 6 -.054 -.0230 -.0230 -.0162 -.0133 -.0106
 6 -.051 -.0229 -.0229 -.0147 -.0120 -.0096
 6 -.045 -.0352 -.0194 -.0133 -.0120 -.0108
 6 -.045 -.0352 -.0194 -.0133 -.0120 -.0108
 6 -.045 -.0352 -.0194 -.0133 -.0120 -.0108
 7 -.005 -.0022 -.0022 -.0024 -.0023 -.0023
 7 -.001 -.0012 -.0011 -.0016 -.0017 -.0018
 7 .002 0.0000 -.0006 -.0006 -.0010 -.0012
 7 .004 .0008 .0004 0.0000 -.0005 -.0008
 7 .005 .0020 .0012 .0005 .0002 0.0000
 7 .006 .0043 .0017 .0009 .0006 .0004
 7 .006 .0043 .0017 .0009 .0006 .0004
 7 .006 .0043 .0017 .0009 .0006 .0004

HGR FOLLOWED BY HGR ARRAY (IN FEET) AT MTIME= 7
 4 6.200 6.2000 6.2000 6.2000 6.2000 6.2000 6.2000 6.2000
 5 5.400 5.4000 5.4000 5.4000 5.4000 5.4000 5.4000

XR VALUES (IDNT=6) AND YR VALUES (IDNT=7) AT MPTIME= 7
 6 -.065 -.0247 -.0211 -.0194 -.0178 -.0162
 6 -.059 -.0307 -.0211 -.0177 -.0162 -.0147
 6 -.056 -.0286 -.0210 -.0162 -.0147 -.0133
 6 -.050 -.0207 -.0209 -.0146 -.0132 -.0114
 6 -.044 -.0205 -.0207 -.0131 -.0119 -.0107
 6 -.039 -.0278 -.0189 -.0117 -.0106 -.0095
 6 -.039 -.0278 -.0189 -.0117 -.0106 -.0095
 6 -.039 -.0278 -.0189 -.0117 -.0106 -.0095
 7 .008 .0026 .0015 .0010 .0006 .0003
 7 .010 .0043 .0022 .0015 .0011 .0008
 7 .010 .0040 .0030 .0014 .0010 .0007
 7 .011 .0044 .0037 .0023 .0018 .0015
 7 .012 .0051 .0044 .0025 .0019 .0015
 7 .012 .0079 .0047 .0027 .0020 .0017
 7 .012 .0079 .0047 .0027 .0020 .0017
 7 .012 .0079 .0047 .0027 .0020 .0017

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HGR FOLLOWED BY HGR ARRAY (IN FEET) AT MTIME= 8
 4 6.000 6.0000 6.0000 6.0000 6.0000 6.0000 6.0000 6.0000
 5 5.300 5.3000 5.3000 5.3000 5.3000 5.3000 5.3000

XR VALUES (IDNT=6) AND YR VALUES (IDNT=7) AT MPTIME= 8
 6 -.048 -.0145 -.0156 -.0143 -.0130 -.0118
 6 -.042 -.0216 -.0155 -.0128 -.0116 -.0105
 6 -.039 -.0198 -.0150 -.0114 -.0103 -.0092
 6 -.034 -.0150 -.0138 -.0114 -.0091 -.0072
 6 -.030 -.0134 -.0136 -.0089 -.0080 -.0072
 6 -.026 -.0175 -.0109 -.0078 -.0079 -.0080
 6 -.026 -.0175 -.0109 -.0078 -.0079 -.0080
 7 .017 .0060 .0045 .0038 .0036 .0025
 7 .016 .0078 .0050 .0037 .0031 .0026
 7 .017 .0076 .0060 .0037 .0031 .0026
 7 .016 .0063 .0053 .0042 .0030 .0022
 7 .015 .0062 .0056 .0036 .0029 .0023
 7 .014 .0085 .0051 .0035 .0032 .0024
 7 .014 .0085 .0051 .0035 .0032 .0024
 7 .014 .0085 .0051 .0035 .0032 .0024

HGR FOLLOWED BY HGR ARRAY (IN FEET) AT MTIME= 9
 4 7.200 7.2000 7.2000 7.2000 7.2000 7.2000 7.2000 7.2000
 5 6.200 6.2000 6.2000 6.2000 6.2000 6.2000 6.2000

XR VALUES (IDNT=6) AND YR VALUES (IDNT=7) AT MPTIME= 9
 6 -.060 -.0249 -.0216 -.0201 -.0186 -.0171
 6 -.057 -.0303 -.0213 -.0183 -.0148 -.0155
 6 -.050 -.0296 -.0240 -.0165 -.0152 -.0139
 6 -.045 -.0222 -.0208 -.0148 -.0137 -.0126
 6 -.037 -.0170 -.0189 -.0133 -.0122 -.0110
 6 -.036 -.0248 -.0142 -.0119 -.0108 -.0161
 6 -.036 -.0248 -.0142 -.0119 -.0108 -.0161
 6 -.036 -.0248 -.0142 -.0119 -.0108 -.0161
 7 .027 .0100 .0078 .0065 .0057 .0049
 7 .026 .0135 .0080 .0066 .0058 .0050
 7 .027 .0150 .0090 .0067 .0058 .0050
 7 .025 .0113 .0097 .0066 .0055 .0046
 7 .022 .0094 .0096 .0065 .0054 .0049
 7 .023 .0148 .0079 .0061 .0053 .0075
 7 .023 .0148 .0079 .0061 .0053 .0075
 7 .023 .0148 .0079 .0061 .0053 .0075

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HGR FOLLOWED BY HBR ARRAY (IN FEET) AT MTIME= 10

4	7.000	7.0000	7.0000	7.0000	7.0000	7.0000	7.0000	7.0000
5	6.000	6.0000	6.0000	6.0000	6.0000	6.0000	6.0000	6.0000

XR VALUES (IDNT=6) AND YR VALUES (IDNT=7) AT MPTIME= 10

6	-.048	-.0194	-.0174	-.0166	-.0140	-.0115
6	-.042	-.0224	-.0178	-.0137	-.0125	-.0113
6	-.037	-.0187	-.0150	-.0122	-.0112	-.0101
6	-.032	-.0170	-.0132	-.0109	-.0099	-.0090
6	-.028	-.0141	-.0157	-.0096	-.0087	-.0078
6	-.024	-.0165	-.0104	-.0065	-.0076	-.0068
6	-.024	-.0165	-.0104	-.0065	-.0076	-.0068
6	-.024	-.0165	-.0104	-.0065	-.0076	-.0068
7	.024	.0086	.0076	.0064	.0048	.0035
7	.023	.0109	.0079	.0055	.0048	.0041
7	.021	.0099	.0080	.0054	.0045	.0037
7	.020	.0094	.0067	.0051	.0042	.0034
7	.018	.0081	.0083	.0049	.0041	.0035
7	.016	.0103	.0060	.0045	.0039	.0033
7	.016	.0103	.0060	.0045	.0039	.0033
7	.016	.0103	.0060	.0045	.0039	.0033

HGR FOLLOWED BY HBR ARRAY (IN FEET) AT MTIME= 11

4	5.800	5.8000	5.8000	5.8000	5.8000	5.8000	5.8000	5.8000
5	5.800	5.8000	5.8000	5.8000	5.8000	5.8000	5.8000	5.8000

XR VALUES (IDNT=6) AND YR VALUES (IDNT=7) AT MPTIME= 11

6	-.054	-.0213	-.0199	-.0169	-.0156	-.0143
6	-.046	-.0245	-.0196	-.0153	-.0140	-.0128
6	-.040	-.0225	-.0180	-.0138	-.0126	-.0114
6	-.037	-.0189	-.0147	-.0123	-.0112	-.0102
6	-.033	-.0157	-.0175	-.0110	-.0099	-.0089
6	-.028	-.0199	-.0130	-.0098	-.0088	-.0079
6	-.028	-.0199	-.0130	-.0098	-.0088	-.0079
6	-.028	-.0199	-.0130	-.0098	-.0088	-.0079
7	.024	.0086	.0072	.0055	.0045	.0036
7	.022	.0109	.0074	.0055	.0045	.0037
7	.021	.0105	.0075	.0053	.0046	.0039
7	.021	.0096	.0069	.0052	.0043	.0035
7	.019	.0083	.0085	.0049	.0042	.0036
7	.018	.0115	.0069	.0045	.0039	.0033
7	.018	.0115	.0069	.0045	.0039	.0033
7	.018	.0115	.0069	.0045	.0039	.0033

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HGR FOLLOWED BY HGR ARRAY (IN FEET) AT MTIME= 12

4	6.200	6.2000	6.2000	6.2000	6.2000	6.2000	6.2000	6.2000
5	5.300	5.3000	5.3000	5.3000	5.3000	5.3000	5.3000	5.3000

XR VALUES(IDNT#6) AND YR VALUES (IDNT#7) AT MAPTIME= 12

6	-.040	-.0186	-.0148	-.0139	-.0129	-.0119
6	-.035	-.0169	-.0146	-.0124	-.0115	-.0105
6	-.029	-.0165	-.0130	-.0100	-.0091	-.0083
6	-.025	-.0136	-.0110	-.0087	-.0081	-.0074
6	-.021	-.0101	-.0115	-.0077	-.0070	-.0063
6	-.018	-.0121	-.0092	-.0059	-.0061	-.0055
6	-.018	-.0121	-.0092	-.0059	-.0061	-.0055
6	-.018	-.0121	-.0092	-.0059	-.0061	-.0055
7	.031	.0135	.0094	.0084	.0071	.0061
7	.029	.0127	.0102	.0080	.0069	.0058
7	.025	.0133	.0095	.0067	.0057	.0048
7	.022	.0114	.0091	.0063	.0052	.0043
7	.020	.0088	.0093	.0058	.0049	.0041
7	.018	.0109	.0077	.0046	.0044	.0037
7	.018	.0109	.0077	.0046	.0044	.0037
7	.018	.0109	.0077	.0046	.0044	.0037

HGR FOLLOWED BY HGR ARRAY (IN FEET) AT MTIME= 13

4	6.600	6.6000	6.6000	6.6000	6.6000	6.7000	6.7000	6.7000
5	5.800	5.8000	5.8000	5.8000	5.8000	5.8000	5.8000	5.8000

XR VALUES(IDNT#6) AND YR VALUES (IDNT#7) AT MAPTIME= 13

6	-.049	-.0255	-.0218	-.0211	-.0201	-.0191
6	-.042	-.0251	-.0194	-.0179	-.0169	-.0159
6	-.037	-.0247	-.0180	-.0174	-.0153	-.0133
6	-.033	-.0197	-.0179	-.0145	-.0126	-.0109
6	-.028	-.0154	-.0150	-.0108	-.0111	-.0102
6	-.024	-.0193	-.0135	-.0096	-.0079	-.0066
6	-.024	-.0193	-.0135	-.0096	-.0079	-.0066
6	-.024	-.0193	-.0135	-.0096	-.0079	-.0066
7	.053	.0246	.0189	.0165	.0146	.0128
7	.047	.0251	.0179	.0144	.0127	.0111
7	.043	.0255	.0170	.0151	.0120	.0093
7	.039	.0211	.0173	.0130	.0102	.0076
7	.033	.0171	.0150	.0101	.0097	.0086
7	.029	.0214	.0140	.0093	.0091	.0085
7	.029	.0214	.0140	.0093	.0091	.0085
7	.029	.0214	.0140	.0093	.0091	.0085

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HGR FOLLOWED BY HGR ARRAY (IN FEET) AT MTIME= 14
 4 5.800 5.8000 5.8000 5.9000 6.0000 6.1000 6.2000 6.2000
 5 5.600 5.6000 5.6000 5.6000 5.6000 5.6000 5.6000

XR VALUES (IDNT=6) AND YR VALUES (IDNT=7) AT MPTIME= 14
 6 -.041 -.0223 -.0188 -.0179 -.0171 -.0163
 6 -.036 -.0235 -.0157 -.0151 -.0155 -.0146
 6 -.032 -.0201 -.0150 -.0162 -.0142 -.0121
 6 -.029 -.0186 -.0155 -.0134 -.0128 -.0121
 6 -.025 -.0147 -.0152 -.0110 -.0115 -.0097
 6 -.022 -.0183 -.0126 -.0099 -.0102 -.0105
 6 -.022 -.0183 -.0126 -.0099 -.0102 -.0105
 6 -.022 -.0183 -.0126 -.0099 -.0102 -.0105
 7 .044 .0215 .0163 .0144 .0124 .0106
 7 .040 .0234 .0142 .0122 .0117 .0102
 7 .036 .0207 .0140 .0136 .0107 .0085
 7 .032 .0193 .0144 .0117 .0100 .0085
 7 .029 .0152 .0147 .0099 .0093 .0071
 7 .025 .0196 .0126 .0089 .0086 .0082
 7 .025 .0196 .0126 .0089 .0086 .0082
 7 .025 .0196 .0126 .0089 .0086 .0082

HGR FOLLOWED BY HGR ARRAY (IN FEET) AT MTIME= 15
 4 4.300 4.3000 4.3000 4.5000 4.7000 4.9000 5.1000 5.1000
 5 5.200 5.2000 5.2000 5.2000 5.2000 5.2000 5.2000

XR VALUES (IDNT=6) AND YR VALUES (IDNT=7) AT MPTIME= 15
 6 -.040 -.0222 -.0190 -.0185 -.0191 -.0181
 6 -.036 -.0264 -.0173 -.0168 -.0176 -.0169
 6 -.031 -.0233 -.0170 -.0182 -.0160 -.0138
 6 -.030 -.0203 -.0170 -.0152 -.0147 -.0140
 6 -.029 -.0163 -.0170 -.0128 -.0132 -.0113
 6 -.025 -.0218 -.0145 -.0115 -.0119 -.0102
 6 -.025 -.0218 -.0145 -.0115 -.0119 -.0102
 6 -.025 -.0218 -.0145 -.0115 -.0119 -.0102
 7 .049 .0246 .0190 .0166 .0160 .0141
 7 .044 .0304 .0179 .0157 .0148 .0127
 7 .040 .0267 .0180 .0169 .0139 .0112
 7 .037 .0234 .0179 .0147 .0127 .0110
 7 .035 .0187 .0182 .0124 .0119 .0095
 7 .032 .0250 .0155 .0115 .0111 .0086
 7 .032 .0250 .0155 .0115 .0111 .0086
 7 .032 .0250 .0155 .0115 .0111 .0086

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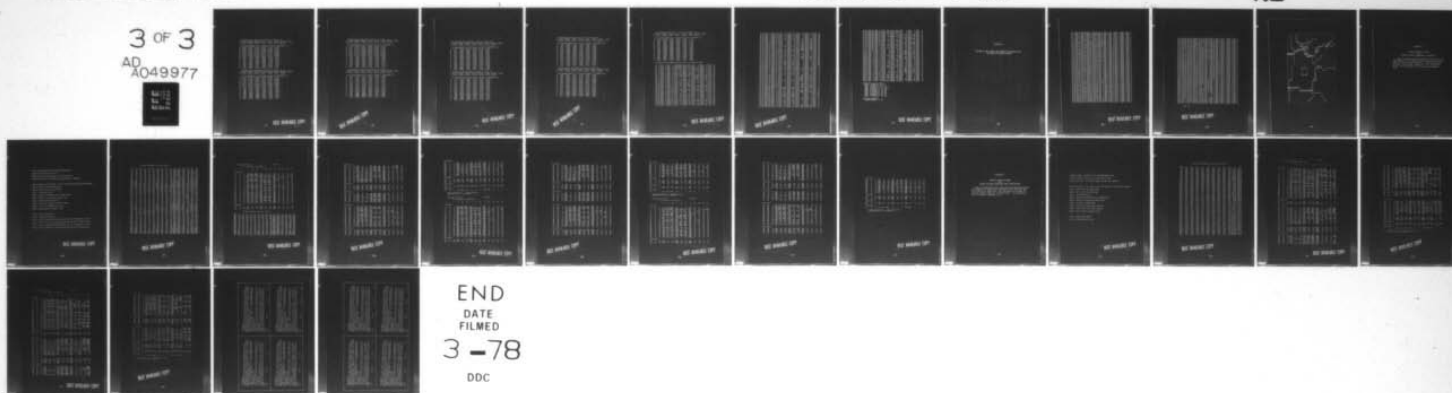
COASTAL STUDIES INC COLLEGE STATION TX
DEVELOPMENT OF SURGE II PROGRAM WITH APPLICATION TO THE SABINE---ETC(U)
NOV 77 R O REID, A C VASTANO, T J REID
CERC-TP-77-13

F/G 4/2
DACW64-74-C-0015
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MGR FOLLOWED BY MBR ARRAY (IN FEET) AT MTIME# 16

4	4.600	4.6000	4.6000	4.7000	4.9000	5.0000	5.1000	5.1000
5	5.100	5.1000	5.1000	5.1000	5.1000	5.1000	5.1000	5.1000

XR VALUES(IDNT#6) AND YR VALUES (IDNT#7) AT MPTIME# 16

6	-.027	-.0156	-.0149	-.0162	-.0174	-.0160
6	-.026	-.0201	-.0138	-.0150	-.0150	-.0160
6	-.025	-.0184	-.0140	-.0150	-.0138	-.0125
6	-.023	-.0161	-.0153	-.0139	-.0128	-.0115
6	-.021	-.0150	-.0142	-.0109	-.0117	-.0125
6	-.019	-.0155	-.0132	-.0100	-.0110	-.0099
6	-.019	-.0155	-.0132	-.0100	-.0110	-.0099
6	-.019	-.0155	-.0132	-.0100	-.0110	-.0099
7	.057	.0293	.0247	.0239	.0230	.0190
7	.054	.0378	.0230	.0222	.0198	.0190
7	.051	.0331	.0240	.0222	.0183	.0149
7	.045	.0290	.0245	.0206	.0169	.0136
7	.040	.0271	.0227	.0161	.0155	.0149
7	.038	.0268	.0211	.0148	.0140	.0110
7	.038	.0268	.0211	.0148	.0140	.0110
7	.038	.0268	.0211	.0148	.0140	.0110

MGR FOLLOWED BY MBR ARRAY (IN FEET) AT MTIME# 17

4	5.400	5.4000	5.4000	5.6000	5.8000	6.0000	6.0000	6.0000
5	5.300	5.3000	5.3000	5.3000	5.3000	5.3000	5.3000	5.3000

XR VALUES(IDNT#6) AND YR VALUES (IDNT#7) AT MPTIME# 17

6	-.015	-.0139	-.0149	-.0138	-.0124	-.0114
6	-.015	-.0125	-.0089	-.0111	-.0121	-.0131
6	-.013	-.0118	-.0089	-.0111	-.0118	-.0122
6	-.014	-.0111	-.0108	-.0104	-.0101	-.0096
6	-.013	-.0097	-.0101	-.0086	-.0094	-.0093
6	-.013	-.0103	-.0092	-.0086	-.0086	-.0085
6	-.013	-.0103	-.0092	-.0086	-.0086	-.0085
6	-.013	-.0103	-.0092	-.0086	-.0086	-.0085
7	.071	.0518	.0486	.0378	.0308	.0243
7	.064	.0464	.0274	.0289	.0285	.0281
7	.058	.0411	.0280	.0289	.0264	.0239
7	.055	.0362	.0314	.0269	.0227	.0189
7	.049	.0317	.0293	.0213	.0210	.0190
7	.044	.0316	.0252	.0213	.0193	.0175
7	.044	.0316	.0252	.0213	.0193	.0175
7	.044	.0316	.0252	.0213	.0193	.0175

MGR FOLLOWED BY MGR ARRAY (IN FEET) AT MTIME= 18
 4 4.700 4.7000 4.7000 4.9000 5.1000 5.3000 5.4000 5.4000
 5 5.000 5.0000 5.0000 5.0000 5.0000 5.0000 5.0000

XR VALUES (IDNT=6) AND YR VALUES (IDNT=7) AT MPTIME= 18

6	.002	.0000	-.0007	-.0020	-.0028	-.0040
6	.001	-.0006	-.0011	-.0022	-.0030	-.0037
6	0.000	-.0011	-.0026	-.0024	-.0031	-.0031
6	-.001	-.0014	-.0019	-.0024	-.0028	-.0028
6	-.001	-.0015	-.0022	-.0023	-.0028	-.0026
6	-.002	-.0016	-.0020	-.0021	-.0026	-.0025
6	-.002	-.0016	-.0020	-.0021	-.0026	-.0025
6	-.002	-.0016	-.0020	-.0021	-.0026	-.0025
7	.048	.0249	.0212	.0229	.0228	.0226
7	.045	.0332	.0211	.0211	.0210	.0208
7	.040	.0310	.0210	.0193	.0192	.0175
7	.038	.0268	.0211	.0193	.0176	.0160
7	.033	.0211	.0211	.0161	.0160	.0131
7	.031	.0229	.0193	.0146	.0145	.0118
7	.031	.0229	.0193	.0146	.0145	.0118
7	.031	.0229	.0193	.0146	.0145	.0118

MGR FOLLOWED BY MGR ARRAY (IN FEET) AT MTIME= 19
 4 3.200 3.2000 3.2000 3.5000 3.8000 4.0000 4.1000 4.1000
 5 4.400 4.4000 4.4000 4.4000 4.4000 4.4000 4.4000

XR VALUES (IDNT=6) AND YR VALUES (IDNT=7) AT MPTIME= 19

6	.016	.0075	.0056	.0050	.0035	.0025
6	.014	.0096	.0046	.0037	.0028	.0019
6	.012	.0072	.0050	.0030	.0022	.0013
6	.011	.0061	.0043	.0030	.0017	.0007
6	.009	.0048	.0035	.0020	.0012	.0004
6	.008	.0046	.0032	.0015	.0011	.0007
6	.008	.0046	.0032	.0015	.0011	.0007
6	.008	.0046	.0032	.0015	.0011	.0007
7	.037	.0301	.0262	.0244	.0246	.0248
7	.035	.0417	.0264	.0266	.0267	.0268
7	.032	.0371	.0240	.0247	.0244	.0248
7	.030	.0349	.0307	.0247	.0244	.0212
7	.047	.0306	.0286	.0229	.0229	.0230
7	.045	.0329	.0306	.0211	.0211	.0212
7	.045	.0329	.0306	.0211	.0211	.0212
7	.045	.0329	.0306	.0211	.0211	.0212

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HGR FOLLOWED BY HGR ARRAY (IN FEET) AT MTIME= 20
 4 2.700 2.7000 2.7000 2.9000 3.2000 3.4000 3.6000 3.6000
 5 4.000 4.0000 4.0000 4.0000 4.0000 4.0000 4.0000

XR VALUES (IDNT=6) AND YR VALUES (IDNT=7) AT MPTIME= 20

6	.018	.0086	.0073	.0076	.0075	.0073
6	.015	.0113	.0064	.0063	.0062	.0059
6	.014	.0082	.0060	.0055	.0054	.0051
6	.012	.0078	.0065	.0054	.0046	.0039
6	.011	.0060	.0057	.0042	.0036	.0028
6	.009	.0062	.0049	.0032	.0033	.0034
6	.009	.0062	.0049	.0032	.0033	.0034
6	.009	.0062	.0049	.0032	.0033	.0034
7	.036	.0193	.0140	.0108	.0217	.0236
7	.035	.0266	.0166	.0144	.0203	.0222
7	.033	.0215	.0145	.0169	.0187	.0205
7	.031	.0216	.0201	.0147	.0172	.0158
7	.029	.0185	.0146	.0157	.0154	.0145
7	.027	.0203	.0171	.0130	.0144	.0159
7	.027	.0203	.0171	.0130	.0144	.0159
7	.027	.0203	.0171	.0130	.0144	.0159

HGR FOLLOWED BY HGR ARRAY (IN FEET) AT MTIME= 21
 4 3.700 3.7000 3.7000 3.9000 3.9000 4.0000 4.1000 4.1000
 5 4.600 4.6000 4.6000 4.6000 4.6000 4.6000 4.6000

XR VALUES (IDNT=6) AND YR VALUES (IDNT=7) AT MPTIME= 21

6	.016	.0074	.0062	.0075	.0070	.0063
6	.013	.0093	.0062	.0063	.0067	.0070
6	.012	.0072	.0060	.0053	.0055	.0058
6	.011	.0070	.0064	.0061	.0048	.0037
6	.010	.0053	.0053	.0043	.0039	.0031
6	.008	.0061	.0043	.0037	.0035	.0030
6	.008	.0061	.0043	.0037	.0035	.0030
6	.008	.0061	.0043	.0037	.0035	.0030
7	.029	.0145	.0134	.0161	.0164	.0195
7	.026	.0190	.0134	.0150	.0165	.0142
7	.026	.0163	.0150	.0138	.0153	.0168
7	.025	.0164	.0166	.0167	.0140	.0114
7	.023	.0138	.0138	.0126	.0128	.0116
7	.022	.0167	.0126	.0114	.0115	.0104
7	.022	.0167	.0126	.0114	.0115	.0104
7	.022	.0167	.0126	.0114	.0115	.0104

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MGR FOLLOWED BY MBR ARRAY (IN FEET) AT MTIME# 22

4	3.500	3.5000	3.5000	3.7000	3.9000	4.0000	4.1000	4.1000
5	4.400	4.4000	4.4000	4.4000	4.4000	4.4000	4.4000	

XR VALUES (IDNT#6) AND YR VALUES (IDNT#7) AT MPTIME# 22

6	.013	.0060	.0050	.0063	.0067	.0071
6	.012	.0079	.0050	.0061	.0058	.0062
6	.010	.0067	.0060	.0051	.0054	.0058
6	.010	.0065	.0069	.0060	.0047	.0030
6	.008	.0049	.0049	.0040	.0039	.0033
6	.007	.0058	.0040	.0034	.0037	.0039
6	.007	.0058	.0040	.0034	.0037	.0039
6	.007	.0058	.0040	.0034	.0037	.0039
7	.021	.0104	.0105	.0118	.0132	.0140
7	.020	.0142	.0106	.0119	.0120	.0134
7	.019	.0132	.0120	.0109	.0122	.0130
7	.019	.0133	.0147	.0135	.0111	.0089
7	.018	.0110	.0110	.0100	.0101	.0090
7	.016	.0130	.0100	.0090	.0101	.0114
7	.016	.0130	.0100	.0090	.0101	.0114
7	.016	.0130	.0100	.0090	.0101	.0114

MGR FOLLOWED BY MBR ARRAY (IN FEET) AT MTIME# 23

4	2.200	2.2000	2.2000	2.4000	2.4000	2.8000	2.9000	2.9000
5	3.500	3.5000	3.5000	3.5000	3.5000	3.5000	3.5000	

XR VALUES (IDNT#6) AND YR VALUES (IDNT#7) AT MPTIME# 23

6	.009	.0041	.0040	.0052	.0055	.0058
6	.008	.0052	.0044	.0049	.0048	.0047
6	.007	.0049	.0050	.0043	.0047	.0051
6	.007	.0047	.0050	.0045	.0044	.0033
6	.006	.0035	.0039	.0033	.0036	.0039
6	.006	.0044	.0032	.0032	.0031	.0029
6	.006	.0044	.0032	.0032	.0031	.0029
6	.006	.0044	.0032	.0032	.0031	.0029
7	.013	.0063	.0072	.0041	.0002	.0105
7	.012	.0061	.0073	.0042	.0043	.0084
7	.011	.0082	.0090	.0074	.0044	.0095
7	.013	.0084	.0105	.0045	.0046	.0068
7	.011	.0060	.0070	.0068	.0077	.0088
7	.011	.0086	.0068	.0068	.0069	.0069
7	.011	.0086	.0068	.0068	.0069	.0069
7	.011	.0086	.0068	.0068	.0069	.0069

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MGP FOLLOWED BY MBR ARRAY (IN FEET) AT MTIME= 24
 4 1.300 1.3000 1.3000 1.5000 1.7000 1.9000 2.1000 2.1000
 5 3.500 3.5000 3.5000 3.5000 3.5000 3.5000 3.5000

XR VALUES (IDNT=6) AND YH VALUES (IDNT=7) AT MTIME= 24

6	.006	.0030	.0035	.0040	.0040	.0049
6	.006	.0034	.0035	.0038	.0042	.0046
6	.005	.0037	.0037	.0032	.0036	.0040
6	.005	.0031	.0045	.0030	.0034	.0038
6	.004	.0026	.0030	.0029	.0033	.0036
6	.004	.0033	.0024	.0024	.0027	.0027
6	.004	.0033	.0024	.0024	.0027	.0027
6	.004	.0033	.0024	.0024	.0027	.0027
7	.008	.0040	.0046	.0053	.0061	.0070
7	.008	.0046	.0047	.0054	.0062	.0072
7	.007	.0055	.0055	.0044	.0055	.0064
7	.007	.0048	.0072	.0049	.0056	.0065
7	.006	.0042	.0049	.0050	.0057	.0065
7	.006	.0057	.0043	.0043	.0051	.0051
7	.006	.0057	.0043	.0043	.0051	.0051
7	.006	.0057	.0043	.0043	.0051	.0051

THE FOLLOWING ARE SURGRID CHANNEL DATA- Z VALUES IN FEET

K=	1	ICG=	8	JCG=	1	I-CX=	-2330	IZCV=	-20	I-WCV=	0	IZCV=	0	IFC=	15
K=	2	ICG=	8	JCG=	2	I-CX=	-2330	IZCV=	-20	I-WCV=	0	IZCV=	0	IFC=	15
K=	3	ICG=	8	JCG=	3	I-CX=	2860	IZCV=	-21	I-WCV=	2860	IZCV=	-21	IFC=	15
K=	4	ICG=	7	JCG=	3	I-CX=	0	IZCV=	0	I-WCV=	0	IZCV=	0	IFC=	15
K=	5	ICG=	7	JCG=	4	I-CX=	2860	IZCV=	-21	I-WCV=	1000	IZCV=	26	IFC=	15
K=	6	ICG=	6	JCG=	4	I-CX=	0	IZCV=	0	I-WCV=	0	IZCV=	0	IFC=	15
K=	7	ICG=	6	JCG=	5	I-CX=	1000	IZCV=	26	I-WCV=	0	IZCV=	0	IFC=	15
K=	8	ICG=	6	JCG=	6	I-CX=	900	IZCV=	26	I-WCV=	300	IZCV=	12	IFC=	20
K=	9	ICG=	6	JCG=	7	I-CX=	-900	IZCV=	-21	I-WCV=	-300	IZCV=	-15	IFC=	20
K=	10	ICG=	7	JCG=	7	I-CX=	0	IZCV=	0	I-WCV=	-900	IZCV=	-26	IFC=	20
K=	11	ICG=	8	JCG=	7	I-CX=	0	IZCV=	0	I-WCV=	-900	IZCV=	-26	IFC=	20
K=	12	ICG=	8	JCG=	8	I-CX=	-900	IZCV=	-26	I-WCV=	0	IZCV=	0	IFC=	20
K=	13	ICG=	9	JCG=	8	I-CX=	0	IZCV=	0	I-WCV=	-900	IZCV=	-26	IFC=	20
K=	14	ICG=	9	JCG=	9	I-CX=	-900	IZCV=	-26	I-WCV=	0	IZCV=	0	IFC=	20
K=	15	ICG=	9	JCG=	10	I-CX=	900	IZCV=	-26	I-WCV=	0	IZCV=	0	IFC=	20
K=	16	ICG=	9	JCG=	11	I-CX=	400	IZCV=	-35	I-WCV=	400	IZCV=	-35	IFC=	25
K=	17	ICG=	8	JCG=	11	I-CX=	0	IZCV=	0	I-WCV=	400	IZCV=	-35	IFC=	25
K=	18	ICG=	7	JCG=	11	I-CX=	0	IZCV=	0	I-WCV=	0	IZCV=	0	IFC=	25
K=	19	ICG=	7	JCG=	12	I-CX=	-400	IZCV=	-35	I-WCV=	-400	IZCV=	-35	IFC=	25
K=	20	ICG=	6	JCG=	12	I-CX=	0	IZCV=	0	I-WCV=	350	IZCV=	43	IFC=	25
K=	21	ICG=	5	JCG=	12	I-CX=	0	IZCV=	0	I-WCV=	0	IZCV=	0	IFC=	25
K=	22	ICG=	5	JCG=	13	I-CX=	350	IZCV=	43	I-WCV=	0	IZCV=	0	IFC=	25
K=	23	ICG=	5	JCG=	14	I-CX=	350	IZCV=	-43	I-WCV=	350	IZCV=	-43	IFC=	25
K=	24	ICG=	4	JCG=	14	I-CX=	0	IZCV=	0	I-WCV=	0	IZCV=	0	IFC=	25
K=	25	ICG=	4	JCG=	15	I-CX=	300	IZCV=	-25	I-WCV=	0	IZCV=	0	IFC=	25
K=	26	ICG=	5	JCG=	15	I-CX=	0	IZCV=	0	I-WCV=	400	IZCV=	-40	IFC=	25
K=	27	ICG=	5	JCG=	16	I-CX=	-400	IZCV=	-40	I-WCV=	-400	IZCV=	-30	IFC=	25
K=	28	ICG=	4	JCG=	16	I-CX=	0	IZCV=	0	I-WCV=	0	IZCV=	0	IFC=	25
K=	29	ICG=	4	JCG=	17	I-CX=	400	IZCV=	-30	I-WCV=	150	IZCV=	-20	IFC=	25
K=	30	ICG=	5	JCG=	17	I-CX=	0	IZCV=	0	I-WCV=	300	IZCV=	-30	IFC=	25

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K# 31	ICG# 5	JCG# 18	IwCX# 400	IZCY# -30	IwCY# 300	IZCY# -30	IFC# 25
K# 32	ICG# 4	JCG# 18	IwCX# 0	IZCY# 0	IwCY# 0	IZCY# 0	IFC# 25
K# 33	ICG# 4	JCG# 19	IwCX# 300	IZCY# -30	IwCY# 0	IZCY# 0	IFC# 25
K# 34	ICG# 3	JCG# 17	IwCX# 0	IZCY# 0	IwCY# 0	IZCY# 0	IFC# 25
K# 35	ICG# 3	JCG# 18	IwCX# 150	IZCY# -20	IwCY# 100	IZCY# -20	IFC# 25
K# 36	ICG# 2	JCG# 18	IwCX# 0	IZCY# 0	IwCY# 100	IZCY# -20	IFC# 25
K# 37	ICG# 10	JCG# 10	IwCX# 0	IZCY# 0	IwCY# 200	IZCY# -12	IFC# 25
K# 38	ICG# 11	JCG# 10	IwCX# 0	IZCY# 0	IwCY# 200	IZCY# -12	IFC# 25
K# 39	ICG# 12	JCG# 10	IwCX# 0	IZCY# 0	IwCY# 200	IZCY# -12	IFC# 25
K# 40	ICG# 12	JCG# 11	IwCX# 200	IZCY# -27	IwCY# 200	IZCY# -20	IFC# 25
K# 41	ICG# 13	JCG# 11	IwCX# 0	IZCY# 0	IwCY# 200	IZCY# -27	IFC# 25
K# 42	ICG# 14	JCG# 11	IwCX# 0	IZCY# 0	IwCY# 200	IZCY# -27	IFC# 25
K# 43	ICG# 14	JCG# 12	IwCX# 200	IZCY# -27	IwCY# -200	IZCY# -27	IFC# 25
K# 44	ICG# 13	JCG# 12	IwCX# 0	IZCY# 0	IwCY# 0	IZCY# 0	IFC# 25
K# 45	ICG# 13	JCG# 13	IwCX# 200	IZCY# 27	IwCY# 0	IZCY# 0	IFC# 25
K# 46	ICG# 14	JCG# 13	IwCX# 0	IZCY# 0	IwCY# 200	IZCY# 27	IFC# 25
K# 47	ICG# 14	JCG# 14	IwCX# 500	IZCY# -20	IwCY# 0	IZCY# 0	IFC# 25
K# 48	ICG# 15	JCG# 14	IwCX# 0	IZCY# 0	IwCY# 350	IZCY# -20	IFC# 25
K# 49	ICG# 15	JCG# 15	IwCX# 350	IZCY# -20	IwCY# 200	IZCY# -20	IFC# 25
K# 50	ICG# 14	JCG# 15	IwCX# 0	IZCY# 0	IwCY# 0	IZCY# 0	IFC# 25
K# 51	ICG# 14	JCG# 16	IwCX# 200	IZCY# -20	IwCY# 0	IZCY# 0	IFC# 25
K# 52	ICG# 14	JCG# 17	IwCX# 200	IZCY# -15	IwCY# 0	IZCY# 0	IFC# 25
K# 53	ICG# 14	JCG# 18	IwCX# 100	IZCY# -10	IwCY# 0	IZCY# 0	IFC# 25
K# 54	ICG# 14	JCG# 19	IwCX# 100	IZCY# -10	IwCY# 0	IZCY# 0	IFC# 25
K# 55	ICG# 11	JCG# 11	IwCX# 0	IZCY# 0	IwCY# 0	IZCY# 0	IFC# 9
K# 56	ICG# 11	JCG# 12	IwCX# 200	IZCY# -20	IwCY# 200	IZCY# -20	IFC# 9
K# 57	ICG# 10	JCG# 12	IwCX# 0	IZCY# 0	IwCY# 0	IZCY# 0	IFC# 9
K# 58	ICG# 10	JCG# 13	IwCX# 200	IZCY# -20	IwCY# 100	IZCY# -20	IFC# 9
K# 59	ICG# 9	JCG# 13	IwCX# 0	IZCY# 0	IwCY# 0	IZCY# 0	IFC# 9
K# 60	ICG# 9	JCG# 14	IwCX# 100	IZCY# -20	IwCY# 0	IZCY# 0	IFC# 9
K# 61	ICG# 15	JCG# 11	IwCX# 0	IZCY# 0	IwCY# 300	IZCY# -12	IFC# 9
K# 62	ICG# 16	JCG# 11	IwCX# 0	IZCY# 0	IwCY# 300	IZCY# -12	IFC# 9
K# 63	ICG# 17	JCG# 11	IwCX# 0	IZCY# 0	IwCY# -300	IZCY# -12	IFC# 9
K# 64	ICG# 18	JCG# 11	IwCX# 0	IZCY# 0	IwCY# -300	IZCY# -12	IFC# 9
K# 65	ICG# 19	JCG# 11	IwCX# 300	IZCY# -12	IwCY# -300	IZCY# -12	IFC# 9
K# 66	ICG# 19	JCG# 10	IwCX# 0	IZCY# 0	IwCY# 0	IZCY# 0	IFC# 9
K# 67	ICG# 20	JCG# 10	IwCX# 0	IZCY# 0	IwCY# 300	IZCY# -12	IFC# 9
K# 68	ICG# 21	JCG# 10	IwCX# 0	IZCY# 0	IwCY# 300	IZCY# -12	IFC# 9
K# 69	ICG# 22	JCG# 10	IwCX# 0	IZCY# 0	IwCY# 300	IZCY# -12	IFC# 9
K# 70	ICG# 23	JCG# 10	IwCX# -400	IZCY# -40	IwCY# 300	IZCY# -12	IFC# 9
K# 71	ICG# 22	JCG# 1	IwCX# -800	IZCY# -32	IwCY# 0	IZCY# 0	IFC# 9
K# 72	ICG# 23	JCG# 1	IwCX# 0	IZCY# 0	IwCY# 1000	IZCY# -16	IFC# 15
K# 73	ICG# 23	JCG# 2	IwCX# 1000	IZCY# -16	IwCY# 1000	IZCY# -16	IFC# 15
K# 74	ICG# 22	JCG# 2	IwCX# 500	IZCY# -40	IwCY# 0	IZCY# 0	IFC# 15
K# 75	ICG# 22	JCG# 3	IwCX# 800	IZCY# -32	IwCY# 0	IZCY# 0	IFC# 15
K# 76	ICG# 22	JCG# 4	IwCX# 1000	IZCY# -20	IwCY# 0	IZCY# 0	IFC# 15
K# 77	ICG# 22	JCG# 5	IwCX# 1000	IZCY# -20	IwCY# 0	IZCY# 0	IFC# 5
K# 78	ICG# 23	JCG# 5	IwCX# 0	IZCY# 0	IwCY# 400	IZCY# -40	IFC# 5
K# 79	ICG# 23	JCG# 6	IwCX# 400	IZCY# 40	IwCY# 0	IZCY# 0	IFC# 5
K# 80	ICG# 23	JCG# 7	IwCX# 400	IZCY# 40	IwCY# 0	IZCY# 0	IFC# 5
K# 81	ICG# 23	JCG# 8	IwCX# 400	IZCY# 40	IwCY# 0	IZCY# 0	IFC# 5
K# 82	ICG# 23	JCG# 9	IwCX# 400	IZCY# 40	IwCY# 0	IZCY# 0	IFC# 5
K# 83	ICG# 24	JCG# 9	IwCX# 0	IZCY# 0	IwCY# 800	IZCY# -25	IFC# 5
K# 84	ICG# 24	JCG# 10	IwCX# 800	IZCY# -25	IwCY# 1000	IZCY# -20	IFC# 5

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K= 85	ICG= 24	JCG= 11	IwCX= 800	IzCX= -25	IwCY= 0	IzCY= 0	IFC= 5
K= 86	ICG= 24	JCG= 12	IwCX= 900	IzCX= -20	IwCY= 0	IzCY= 0	IFC= 5
K= 87	ICG= 24	JCG= 13	IwCX= 1000	IzCX= -25	IwCY= 0	IzCY= 0	IFC= 5
K= 88	ICG= 25	JCG= 13	IwCX= 0	IzCX= 0	IwCY= 1000	IzCY= -25	IFC= 5
K= 89	ICG= 25	JCG= 14	IwCX= 400	IzCX= -40	IwCY= 0	IzCY= 0	IFC= 5
K= 90	ICG= 26	JCG= 14	IwCX= 0	IzCX= 0	IwCY= 800	IzCY= -20	IFC= 5
K= 91	ICG= 27	JCG= 14	IwCX= 0	IzCX= 0	IwCY= 900	IzCY= -20	IFC= 5
K= 92	ICG= 27	JCG= 15	IwCX= 800	IzCX= -20	IwCY= 0	IzCY= 0	IFC= 5
K= 93	ICG= 28	JCG= 15	IwCX= 0	IzCX= 0	IwCY= 800	IzCY= -20	IFC= 5
K= 94	ICG= 25	JCG= 10	IwCX= 0	IzCX= 0	IwCY= 300	IzCY= -12	IFC= 9
K= 95	ICG= 26	JCG= 10	IwCX= 300	IzCX= -12	IwCY= 300	IzCY= -12	IFC= 9
K= 96	ICG= 26	JCG= 9	IwCX= 0	IzCX= 0	IwCY= 0	IzCY= 0	IFC= 9
K= 97	ICG= 27	JCG= 9	IwCX= 300	IzCX= -12	IwCY= 300	IzCY= -12	IFC= 9
K= 98	ICG= 27	JCG= 8	IwCX= 0	IzCX= 0	IwCY= 0	IzCY= 0	IFC= 9
K= 99	ICG= 28	JCG= 8	IwCX= 0	IzCX= 0	IwCY= 300	IzCY= -12	IFC= 9
K=100	ICG= 1	JCG= 4	IwCX= 0	IzCX= 0	IwCY= 300	IzCY= -12	IFC= 9
K=101	ICG= 2	JCG= 4	IwCX= 0	IzCX= 0	IwCY= 300	IzCY= -12	IFC= 9
K=102	ICG= 3	JCG= 4	IwCX= 0	IzCX= 0	IwCY= 300	IzCY= -12	IFC= 9
K=103	ICG= 3	JCG= 5	IwCX= 300	IzCX= 12	IwCY= 0	IzCY= 0	IFC= 9
K=104	ICG= 4	JCG= 5	IwCX= 0	IzCX= 0	IwCY= 300	IzCY= -12	IFC= 9
K=105	ICG= 5	JCG= 5	IwCX= 0	IzCX= 0	IwCY= 300	IzCY= 12	IFC= 9
K=106	ICG= 5	JCG= 6	IwCX= 300	IzCX= -12	IwCY= 0	IzCY= 0	IFC= 9
K=107	ICG= 5	JCG= 7	IwCX= 0	IzCX= 0	IwCY= 0	IzCY= 0	IFC= 9
K=108	ICG= 5	JCG= 8	IwCX= 400	IzCX= 15	IwCY= 400	IzCY= 15	IFC= 9
K=109	ICG= 4	JCG= 8	IwCX= 0	IzCX= 0	IwCY= 0	IzCY= 0	IFC= 9
K=110	ICG= 4	JCG= 9	IwCX= 200	IzCX= -12	IwCY= 200	IzCY= -10	IFC= 9
K=111	ICG= 3	JCG= 9	IwCX= 200	IzCX= 10	IwCY= 0	IzCY= 0	IFC= 9
K=112	ICG= 3	JCG= 8	IwCX= 0	IzCX= 0	IwCY= 200	IzCY= -10	IFC= 9
K=113	ICG= 2	JCG= 8	IwCX= 0	IzCX= 0	IwCY= 200	IzCY= -10	IFC= 9
K=114	ICG= 1	JCG= 8	IwCX= 0	IzCX= 0	IwCY= 200	IzCY= -10	IFC= 9
K=115	ICG= 3	JCG= 10	IwCX= 200	IzCX= -10	IwCY= 200	IzCY= -10	IFC= 9
K=116	ICG= 2	JCG= 10	IwCX= 0	IzCX= 0	IwCY= 0	IzCY= 0	IFC= 9
K=117	ICG= 2	JCG= 11	IwCX= 200	IzCX= -8	IwCY= 0	IzCY= 0	IFC= 9
K=118	ICG= 2	JCG= 12	IwCX= 200	IzCX= -8	IwCY= 0	IzCY= 0	IFC= 9
K=119	ICG= 6	JCG= 8	IwCX= -400	IzCX= -20	IwCY= 0	IzCY= 0	IFC= 9
K=120	ICG= 7	JCG= 8	IwCX= 0	IzCX= 0	IwCY= 200	IzCY= -20	IFC= 9
K=121	ICG= 7	JCG= 5	IwCX= 3000	IzCX= -12	IwCY= 0	IzCY= 0	IFC= 9

HYDROGRAPH GAGE LOCATIONS

GAGE 1 BLOCK M, I= 8 J= 1
 GAGE 2 CHANNEL M, K= 11
 GAGE 3 BLOCK M, I= 11 J= 10
 GAGE 4 CHANNEL M, K= 25
 GAGE 5 CHANNEL M, K= 46
 GAGE 6 CHANNEL M, K= 72
 GAGE 7 CHANNEL M, K= 100
 GAGE 8 CHANNEL M, K= 3
 GAGE 9 CHANNEL M, K= 92

KEY FLOW LOCATIONS

CHANNEL BLOCKS 1 71

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APPENDIX E

LISTING OF KEY ARRAYS AND CHANNEL AND BARRIER PLOT FOR SABINE-CALCASIEU REGION

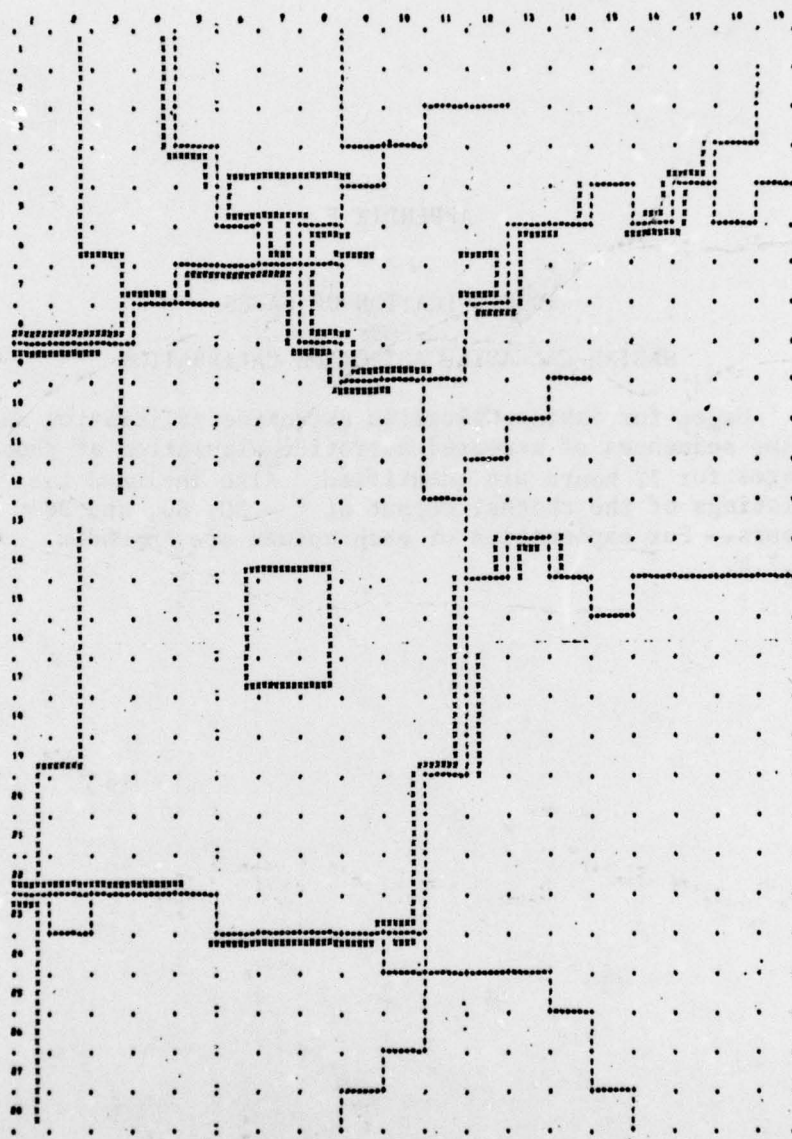
K#	1	KCX#122	KCY#128	KCXP#2	KCV#128	KCB#1	ICG#	-8	JCG#	1	KEN1#	1	KEN2#	0	KRI#	0
K#	2	KCX#1	KCY#128	KCXP#3	KCV#128	KCB#17	ICG#	8	JCG#	2	KEN1#	0	KEN2#	0	KRI#	0
K#	3	KCX#2	KCY#4	KCXP#128	KCV#128	KCB#28	ICG#	8	JCG#	3	KEN1#	0	KEN2#	0	KRI#	0
K#	4	KCX#128	KCY#128	KCXP#5	KCV#3	KCB#27	ICG#	7	JCG#	3	KEN1#	0	KEN2#	0	KRI#	0
K#	5	KCX#4	KCY#6	KCXP#121	KCV#128	KCB#37	ICG#	7	JCG#	4	KEN1#	0	KEN2#	0	KRI#	0
K#	6	KCX#128	KCY#128	KCXP#7	KCV#5	KCB#0	ICG#	6	JCG#	4	KEN1#	0	KEN2#	0	KRI#	0
K#	7	KCX#6	KCY#105	KCXP#8	KCV#121	KCB#43	ICG#	6	JCG#	5	KEN1#	0	KEN2#	0	KRI#	0
K#	8	KCX#7	KCY#106	KCXP#9	KCV#129	KCB#45	ICG#	6	JCG#	6	KEN1#	0	KEN2#	0	KRI#	0
K#	9	KCX#8	KCY#107	KCXP#119	KCV#10	KCB#51	ICG#	6	JCG#	7	KEN1#	0	KEN2#	0	KRI#	0
K#	10	KCX#128	KCY#9	KCXP#120	KCV#11	KCB#53	ICG#	7	JCG#	7	KEN1#	0	KEN2#	0	KRI#	0
K#	11	KCX#128	KCY#10	KCXP#12	KCV#128	KCB#54	ICG#	8	JCG#	7	KEN1#	0	KEN2#	0	KRI#	0
K#	12	KCX#11	KCY#120	KCXP#128	KCV#13	KCB#62	ICG#	8	JCG#	8	KEN1#	0	KEN2#	0	KRI#	0
K#	13	KCX#128	KCY#12	KCXP#14	KCV#128	KCB#63	ICG#	9	JCG#	8	KEN1#	0	KEN2#	0	KRI#	0
K#	14	KCX#13	KCY#128	KCXP#15	KCV#128	KCB#71	ICG#	9	JCG#	9	KEN1#	0	KEN2#	0	KRI#	0
K#	15	KCX#14	KCY#128	KCXP#16	KCV#37	KCB#73	ICG#	9	JCG#	10	KEN1#	0	KEN2#	0	KRI#	0
K#	16	KCX#15	KCY#17	KCXP#128	KCV#128	KCB#0	ICG#	9	JCG#	11	KEN1#	0	KEN2#	0	KRI#	0
K#	17	KCX#128	KCY#18	KCXP#128	KCV#16	KCB#0	ICG#	8	JCG#	11	KEN1#	0	KEN2#	0	KRI#	0
K#	18	KCX#128	KCY#128	KCXP#19	KCV#17	KCB#0	ICG#	7	JCG#	11	KEN1#	0	KEN2#	0	KRI#	0
K#	19	KCX#18	KCY#20	KCXP#128	KCV#128	KCB#84	ICG#	7	JCG#	12	KEN1#	0	KEN2#	0	KRI#	0
K#	20	KCX#128	KCY#21	KCXP#128	KCV#19	KCB#83	ICG#	6	JCG#	12	KEN1#	0	KEN2#	0	KRI#	0
K#	21	KCX#128	KCY#128	KCXP#22	KCV#20	KCB#0	ICG#	5	JCG#	12	KEN1#	0	KEN2#	0	KRI#	0
K#	22	KCX#21	KCY#128	KCXP#23	KCV#128	KCB#86	ICG#	5	JCG#	13	KEN1#	0	KEN2#	0	KRI#	0
K#	23	KCX#22	KCY#24	KCXP#26	KCV#128	KCB#89	ICG#	5	JCG#	14	KEN1#	0	KEN2#	0	KRI#	0
K#	24	KCX#128	KCY#128	KCXP#25	KCV#23	KCB#0	ICG#	4	JCG#	14	KEN1#	0	KEN2#	0	KRI#	0
K#	25	KCX#24	KCY#128	KCXP#28	KCV#26	KCB#0	ICG#	4	JCG#	15	KEN1#	0	KEN2#	0	KRI#	0
K#	26	KCX#23	KCY#25	KCXP#27	KCV#128	KCB#0	ICG#	5	JCG#	15	KEN1#	0	KEN2#	0	KRI#	0
K#	27	KCX#26	KCY#28	KCXP#30	KCV#128	KCB#90	ICG#	5	JCG#	16	KEN1#	0	KEN2#	0	KRI#	0
K#	28	KCX#25	KCY#128	KCXP#29	KCV#27	KCB#0	ICG#	4	JCG#	16	KEN1#	0	KEN2#	0	KRI#	0
K#	29	KCX#28	KCY#34	KCXP#32	KCV#30	KCB#91	ICG#	4	JCG#	17	KEN1#	0	KEN2#	0	KRI#	0
K#	30	KCX#27	KCY#29	KCXP#31	KCV#128	KCB#0	ICG#	5	JCG#	17	KEN1#	0	KEN2#	0	KRI#	0
K#	31	KCX#30	KCY#32	KCXP#128	KCV#128	KCB#0	ICG#	5	JCG#	18	KEN1#	0	KEN2#	0	KRI#	0
K#	32	KCX#29	KCY#35	KCXP#33	KCV#31	KCB#0	ICG#	4	JCG#	18	KEN1#	0	KEN2#	0	KRI#	0
K#	33	KCX#32	KCY#128	KCXP#128	KCV#128	KCB#0	ICG#	-4	JCG#	19	KEN1#	7	KEN2#	0	KRI#	2
K#	34	KCX#128	KCY#128	KCXP#35	KCV#29	KCB#0	ICG#	3	JCG#	17	KEN1#	0	KEN2#	0	KRI#	0
K#	35	KCX#34	KCY#36	KCXP#128	KCV#32	KCB#0	ICG#	3	JCG#	18	KEN1#	0	KEN2#	0	KRI#	0
K#	36	KCX#128	KCY#123	KCXP#128	KCV#35	KCB#0	ICG#	-2	JCG#	18	KEN1#	6	KEN2#	0	KRI#	0
K#	37	KCX#128	KCY#15	KCXP#128	KCV#38	KCB#0	ICG#	10	JCG#	10	KEN1#	0	KEN2#	0	KRI#	0
K#	38	KCX#128	KCY#37	KCXP#55	KCV#39	KCB#0	ICG#	11	JCG#	10	KEN1#	0	KEN2#	0	KRI#	0
K#	39	KCX#128	KCY#38	KCXP#40	KCV#128	KCB#0	ICG#	12	JCG#	10	KEN1#	0	KEN2#	0	KRI#	0
K#	40	KCX#39	KCY#55	KCXP#128	KCV#41	KCB#0	ICG#	12	JCG#	11	KEN1#	0	KEN2#	0	KRI#	0
K#	41	KCX#128	KCY#40	KCXP#44	KCV#42	KCB#0	ICG#	13	JCG#	11	KEN1#	0	KEN2#	0	KRI#	0
K#	42	KCX#128	KCY#41	KCXP#43	KCV#61	KCB#0	ICG#	14	JCG#	11	KEN1#	0	KEN2#	0	KRI#	0
K#	43	KCX#42	KCY#44	KCXP#46	KCV#128	KCB#85	ICG#	14	JCG#	12	KEN1#	0	KEN2#	0	KRI#	0
K#	44	KCX#41	KCY#128	KCXP#45	KCV#43	KCB#0	ICG#	13	JCG#	12	KEN1#	0	KEN2#	0	KRI#	0
K#	45	KCX#44	KCY#128	KCXP#128	KCV#46	KCB#87	ICG#	13	JCG#	13	KEN1#	0	KEN2#	0	KRI#	0
K#	46	KCX#43	KCY#45	KCXP#47	KCV#128	KCB#88	ICG#	14	JCG#	13	KEN1#	0	KEN2#	0	KRI#	0
K#	47	KCX#46	KCY#128	KCXP#50	KCV#48	KCB#0	ICG#	14	JCG#	14	KEN1#	0	KEN2#	0	KRI#	0
K#	48	KCX#128	KCY#47	KCXP#49	KCV#128	KCB#0	ICG#	15	JCG#	14	KEN1#	0	KEN2#	0	KRI#	0
K#	49	KCX#48	KCY#50	KCXP#128	KCV#128	KCB#0	ICG#	15	JCG#	15	KEN1#	0	KEN2#	0	KRI#	0
K#	50	KCX#47	KCY#128	KCXP#51	KCV#49	KCB#0	ICG#	14	JCG#	15	KEN1#	0	KEN2#	0	KRI#	0
K#	51	KCX#50	KCY#128	KCXP#52	KCV#128	KCB#0	ICG#	14	JCG#	16	KEN1#	0	KEN2#	0	KRI#	0
K#	52	KCX#41	KCY#128	KCXP#53	KCV#128	KCB#0	ICG#	14	JCG#	17	KEN1#	0	KEN2#	0	KRI#	0
K#	53	KCX#52	KCY#128	KCXP#54	KCV#128	KCB#0	ICG#	14	JCG#	18	KEN1#	0	KEN2#	0	KRI#	0
K#	54	KCX#53	KCY#128	KCXP#129	KCV#128	KCB#0	ICG#	-14	JCG#	19	KEN1#	7	KEN2#	0	KRI#	3
K#	55	KCX#38	KCY#128	KCXP#56	KCV#40	KCB#0	ICG#	11	JCG#	11	KEN1#	0	KEN2#	0	KRI#	0
K#	56	KCX#55	KCY#57	KCXP#128	KCV#128	KCB#0	ICG#	11	JCG#	12	KEN1#	0	KEN2#	0	KRI#	0
K#	57	KCX#128	KCY#128	KCXP#58	KCV#56	KCB#0	ICG#	10	JCG#	12	KEN1#	0	KEN2#	0	KRI#	0
K#	58	KCX#57	KCY#59	KCXP#128	KCV#128	KCB#0	ICG#	10	JCG#	13	KEN1#	0	KEN2#	0	KRI#	0
K#	59	KCX#128	KCY#128	KCXP#60	KCV#58	KCB#0	ICG#	9	JCG#	13	KEN1#	0	KEN2#	0	KRI#	0
K#	60	KCX#59	KCY#128	KCXP#128	KCV#128	KCB#0	ICG#	-9	JCG#	14	KEN1#	7	KEN2#	0	KRI#	0
K#	61	KCX#128	KCY#42	KCXP#128	KCV#62	KCB#78	ICG#	15	JCG#	11	KEN1#	0	KEN2#	0	KRI#	0
K#	62	KCX#128	KCY#61	KCXP#128	KCV#63	KCB#79	ICG#	16	JCG#	11	KEN1#	0	KEN2#	0	KRI#	0
K#	63	KCX#128	KCY#62	KCXP#128	KCV#64	KCB#80	ICG#	17	JCG#	11	KEN1#	0	KEN2#	0	KRI#	0

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K# 64	KCX# 128	KCY# 63	KCX# 128	KCV# 65	KCB# 81	ICG# 18	JCG# 11	KEN1# 0	KEN2# 0	KRI# 0
K# 65	KCX# 66	KCY# 64	KCX# 128	KCV# 128	KCB# 82	ICG# 19	JCG# 11	KEN1# 0	KEN2# 0	KRI# 0
K# 66	KCX# 128	KCY# 128	KCX# 65	KCV# 67	KCB# 0	ICG# 19	JCG# 10	KEN1# 0	KEN2# 0	KRI# 0
K# 67	KCX# 128	KCY# 66	KCX# 128	KCV# 68	KCB# 74	ICG# 20	JCG# 10	KEN1# 0	KEN2# 0	KRI# 0
K# 68	KCX# 128	KCY# 67	KCX# 128	KCV# 69	KCB# 75	ICG# 21	JCG# 10	KEN1# 0	KEN2# 0	KRI# 0
K# 69	KCX# 128	KCY# 68	KCX# 128	KCV# 70	KCB# 76	ICG# 22	JCG# 10	KEN1# 0	KEN2# 0	KRI# 0
K# 70	KCX# 128	KCY# 69	KCX# 128	KCV# 71	KCB# 77	ICG# 23	JCG# 10	KEN1# 0	KEN2# 0	KRI# 0
K# 71	KCX# 128	KCY# 70	KCX# 128	KCV# 72	KCB# 4	ICG# 22	JCG# 1	KEN1# 1	KEN2# 0	KRI# 0
K# 72	KCX# 128	KCY# 71	KCX# 128	KCV# 128	KCB# 5	ICG# 23	JCG# 1	KEN1# 0	KEN2# 0	KRI# 0
K# 73	KCX# 72	KCY# 74	KCX# 128	KCV# 128	KCB# 0	ICG# 23	JCG# 2	KEN1# 0	KEN2# 0	KRI# 0
K# 74	KCX# 71	KCY# 128	KCX# 75	KCV# 73	KCB# 25	ICG# 22	JCG# 2	KEN1# 0	KEN2# 0	KRI# 0
K# 75	KCX# 74	KCY# 128	KCX# 76	KCV# 128	KCB# 33	ICG# 22	JCG# 3	KEN1# 0	KEN2# 0	KRI# 0
K# 76	KCX# 75	KCY# 128	KCX# 77	KCV# 128	KCB# 38	ICG# 22	JCG# 4	KEN1# 0	KEN2# 0	KRI# 0
K# 77	KCX# 76	KCY# 128	KCX# 128	KCV# 78	KCB# 0	ICG# 22	JCG# 5	KEN1# 0	KEN2# 0	KRI# 0
K# 78	KCX# 128	KCY# 77	KCX# 79	KCV# 128	KCB# 0	ICG# 23	JCG# 5	KEN1# 0	KEN2# 0	KRI# 0
K# 79	KCX# 78	KCY# 128	KCX# 80	KCV# 128	KCB# 49	ICG# 23	JCG# 6	KEN1# 0	KEN2# 0	KRI# 0
K# 80	KCX# 79	KCY# 128	KCX# 81	KCV# 128	KCB# 57	ICG# 23	JCG# 7	KEN1# 0	KEN2# 0	KRI# 0
K# 81	KCX# 80	KCY# 128	KCX# 82	KCV# 128	KCB# 68	ICG# 23	JCG# 8	KEN1# 0	KEN2# 0	KRI# 0
K# 82	KCX# 81	KCY# 128	KCX# 70	KCV# 83	KCB# 72	ICG# 23	JCG# 9	KEN1# 0	KEN2# 0	KRI# 0
K# 83	KCX# 128	KCY# 82	KCX# 84	KCV# 128	KCB# 0	ICG# 24	JCG# 9	KEN1# 0	KEN2# 0	KRI# 0
K# 84	KCX# 83	KCY# 70	KCX# 85	KCV# 94	KCB# 0	ICG# 24	JCG# 10	KEN1# 0	KEN2# 0	KRI# 0
K# 85	KCX# 84	KCY# 128	KCX# 86	KCV# 128	KCB# 0	ICG# 24	JCG# 11	KEN1# 0	KEN2# 0	KRI# 0
K# 86	KCX# 85	KCY# 128	KCX# 87	KCV# 128	KCB# 0	ICG# 24	JCG# 12	KEN1# 0	KEN2# 0	KRI# 0
K# 87	KCX# 86	KCY# 128	KCX# 128	KCV# 88	KCB# 0	ICG# 24	JCG# 13	KEN1# 0	KEN2# 0	KRI# 0
K# 88	KCX# 128	KCY# 87	KCX# 89	KCV# 128	KCB# 0	ICG# 25	JCG# 13	KEN1# 0	KEN2# 0	KRI# 0
K# 89	KCX# 88	KCY# 128	KCX# 128	KCV# 90	KCB# 0	ICG# 25	JCG# 14	KEN1# 0	KEN2# 0	KRI# 0
K# 90	KCX# 128	KCY# 89	KCX# 128	KCV# 91	KCB# 0	ICG# 26	JCG# 14	KEN1# 0	KEN2# 0	KRI# 0
K# 91	KCX# 128	KCY# 90	KCX# 92	KCV# 128	KCB# 0	ICG# 27	JCG# 14	KEN1# 0	KEN2# 0	KRI# 0
K# 92	KCX# 91	KCY# 128	KCX# 128	KCV# 93	KCB# 0	ICG# 27	JCG# 15	KEN1# 0	KEN2# 0	KRI# 0
K# 93	KCX# 128	KCY# 92	KCX# 128	KCV# 128	KCB# 0	ICG# 28	JCG# 15	KEN1# 8	KEN2# 0	KRI# 1
K# 94	KCX# 128	KCY# 84	KCX# 128	KCV# 95	KCB# 0	ICG# 25	JCG# 10	KEN1# 0	KEN2# 0	KRI# 0
K# 95	KCX# 96	KCY# 94	KCX# 128	KCV# 128	KCB# 0	ICG# 26	JCG# 10	KEN1# 0	KEN2# 0	KRI# 0
K# 96	KCX# 128	KCY# 128	KCX# 95	KCV# 97	KCB# 0	ICG# 26	JCG# 9	KEN1# 0	KEN2# 0	KRI# 0
K# 97	KCX# 98	KCY# 96	KCX# 128	KCV# 128	KCB# 0	ICG# 27	JCG# 9	KEN1# 0	KEN2# 0	KRI# 0
K# 98	KCX# 128	KCY# 128	KCX# 97	KCV# 99	KCB# 0	ICG# 27	JCG# 8	KEN1# 0	KEN2# 0	KRI# 0
K# 99	KCX# 128	KCY# 98	KCX# 128	KCV# 128	KCB# 0	ICG# 28	JCG# 8	KEN1# 8	KEN2# 0	KRI# 0
K# 100	KCX# 128	KCY# 125	KCX# 128	KCV# 101	KCB# 34	ICG# -1	JCG# 4	KEN1# 6	KEN2# 0	KRI# 0
K# 101	KCX# 128	KCY# 100	KCX# 128	KCV# 102	KCB# 35	ICG# 2	JCG# 4	KEN1# 0	KEN2# 0	KRI# 0
K# 102	KCX# 128	KCY# 101	KCX# 103	KCV# 128	KCB# 36	ICG# 3	JCG# 4	KEN1# 0	KEN2# 0	KRI# 0
K# 103	KCX# 102	KCY# 128	KCX# 128	KCV# 104	KCB# 39	ICG# 3	JCG# 5	KEN1# 0	KEN2# 0	KRI# 0
K# 104	KCX# 128	KCY# 103	KCX# 128	KCV# 105	KCB# 40	ICG# 4	JCG# 5	KEN1# 0	KEN2# 0	KRI# 0
K# 105	KCX# 128	KCY# 104	KCX# 106	KCV# 7	KCB# 41	ICG# 5	JCG# 5	KEN1# 0	KEN2# 0	KRI# 0
K# 106	KCX# 105	KCY# 128	KCX# 107	KCV# 8	KCB# 44	ICG# 5	JCG# 6	KEN1# 0	KEN2# 0	KRI# 0
K# 107	KCX# 106	KCY# 128	KCX# 108	KCV# 9	KCB# 52	ICG# 5	JCG# 7	KEN1# 0	KEN2# 0	KRI# 0
K# 108	KCX# 107	KCY# 109	KCX# 128	KCV# 119	KCB# 59	ICG# 5	JCG# 8	KEN1# 0	KEN2# 0	KRI# 0
K# 109	KCX# 128	KCY# 112	KCX# 110	KCV# 108	KCB# 58	ICG# 4	JCG# 8	KEN1# 0	KEN2# 0	KRI# 0
K# 110	KCX# 109	KCY# 111	KCX# 128	KCV# 128	KCB# 0	ICG# 4	JCG# 9	KEN1# 0	KEN2# 0	KRI# 0
K# 111	KCX# 112	KCY# 128	KCX# 115	KCV# 110	KCB# 0	ICG# 3	JCG# 9	KEN1# 0	KEN2# 0	KRI# 0
K# 112	KCX# 128	KCY# 113	KCX# 111	KCV# 109	KCB# 0	ICG# 3	JCG# 8	KEN1# 0	KEN2# 0	KRI# 0
K# 113	KCX# 128	KCY# 114	KCX# 128	KCV# 112	KCB# 0	ICG# 2	JCG# 8	KEN1# 0	KEN2# 0	KRI# 0
K# 114	KCX# 128	KCY# 126	KCX# 128	KCV# 113	KCB# 0	ICG# -1	JCG# 8	KEN1# 6	KEN2# 0	KRI# 0
K# 115	KCX# 111	KCY# 116	KCX# 128	KCV# 128	KCB# 0	ICG# 3	JCG# 10	KEN1# 0	KEN2# 0	KRI# 0
K# 116	KCX# 128	KCY# 128	KCX# 117	KCV# 115	KCB# 0	ICG# 2	JCG# 10	KEN1# 0	KEN2# 0	KRI# 0
K# 117	KCX# 116	KCY# 128	KCX# 118	KCV# 128	KCB# 0	ICG# 2	JCG# 11	KEN1# 0	KEN2# 0	KRI# 0
K# 118	KCX# 117	KCY# 128	KCX# 128	KCV# 128	KCB# 0	ICG# -2	JCG# 12	KEN1# 7	KEN2# 0	KRI# 0
K# 119	KCX# 9	KCY# 108	KCX# 128	KCV# 120	KCB# 60	ICG# 6	JCG# 8	KEN1# 0	KEN2# 0	KRI# 0
K# 120	KCX# 10	KCY# 119	KCX# 128	KCV# 127	KCB# 61	ICG# -7	JCG# 8	KEN1# 4	KEN2# 0	KRI# 0
K# 121	KCX# 5	KCY# 7	KCX# 128	KCV# 128	KCB# 0	ICG# -7	JCG# 5	KEN1# 3	KEN2# 0	KRI# 0

KCX# 128

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APPENDIX F

IDENTIFICATION OF GAGES FOR SABINE-CALCASIEU ASTROTIDE CALIBRATION

Gages for Sabine-Calcasieu astrotide calibration and time sequences of accepted astrotide simulation at those gages for 72 hours are identified. Also included are listings of the channel output at $t = 30, 60,$ and 90 hours. For explanation of each column see Appendix C,7,b.

ASTRO TIDE CALIBRATION FOR SABINE-CALCASIEU AREA
SABINE PASS TIDES USED AS INPUT
PERIOD OF RECORD- 0000 AUG.22 TO 2400 AUG.26,1973
CALCULATIONS ALLOW FOR SUB-GRID SCALE CHANNELS AND BARRIERS

TIME SEQUENCES OF WATER LEVEL AND FLOW ARE SAVED FOR THE FOLLOWING PLACES-

GAGE 1 SABINE PASS, SOUTHWEST JETTY
GAGE 2 PORT ARTHUR, CE AREA OFFICE
GAGE 3 NORTH SABINE LAKE
GAGE 4 BEAUMONT, NECHES RIVER AND BRACKES BAYOU
GAGE 5 ORANGE NAVAL STATION, SABINE RIVER
GAGE 6 CAMERON, CALCASIEU PASS
GAGE 7 HACKBERRY, CALCASIEU RIVER AND PASS
GAGE 8 I.N.P. AT CALCASIEU LOCK, WEST
GAGE 9 LAKE CHARLES, CALCASIEU RIVER

FLOW 1 SABINE PASS INFLOW
FLOW 2 CALCASIEU PASS INFLOW
FLOW 3 FLOW TO NECHES RIVER FROM SABINE LAKE AND INTRACOASTAL WATERWAY
FLOW 4 EASTWARD FLOW VIA INTRACOASTAL CANAL JUST EAST OF SABINE RIVER
FLOW 5 FLOW TO SABINE RIVER FROM SABINE LAKE AND INTRACOASTAL WATERWAY
FLOW 6 FLOW TO CALCASIEU RIVER FROM CALCASIEU LAKE AND INTRACOASTAL W.

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WATER LEVEL HYDROGRAPHS (FT) AND KEY FLOWS (1000 CFS)

HOUR	1	2	3	4	5	6	7	8	9	1	2	3	4	5	6
0.0	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
1.0	.09	.00	.00	.07	.09	.33	.01	.01	.02	29.97	18.72	.00	.01	.01	.15
2.0	.47	.05	.00	.08	.21	.36	.14	.03	.02	46.82	23.77	-1.23	.80	-1.92	.15
3.0	.44	.14	.03	.06	.12	.42	.19	.11	.03	57.49	30.78	.74	.53	-2.05	2.17
4.0	.42	.20	.18	.10	.11	.34	.24	.22	.14	62.72	33.49	.55	.29	-1.45	5.70
5.0	.37	.25	.25	.19	.17	.38	.28	.24	.30	60.82	30.52	1.35	.07	-.57	6.02
6.0	.35	.26	.27	.32	.29	.36	.32	.33	.37	55.42	26.94	1.68	.20	-.30	4.17
7.0	.32	.33	.33	.43	.39	.33	.34	.39	.42	42.43	23.21	.75	.16	-.69	2.63
8.0	.29	.38	.39	.47	.44	.31	.37	.42	.45	31.83	18.45	-.67	-.02	-1.01	.98
9.0	.26	.39	.41	.48	.44	.28	.39	.44	.45	22.13	13.83	-1.03	-.08	-1.17	-.05
10.0	.21	.40	.41	.42	.39	.25	.39	.43	.44	8.68	7.45	-2.01	.14	-1.36	-1.21
11.0	.14	.40	.41	.38	.33	.18	.34	.40	.43	-13.60	-1.62	-1.79	.24	-1.79	-2.53
12.0	.04	.37	.40	.36	.28	.10	.32	.38	.38	-35.07	-11.20	-1.52	.65	-2.06	-3.22
13.0	-.24	.31	.35	.35	.22	-.13	.25	.31	.34	-63.84	-27.77	-1.69	.79	-2.01	-4.20
14.0	-.58	.23	.27	.32	.36	-.42	.14	.23	.27	-93.03	-47.50	-2.32	.78	-2.04	-5.72
15.0	-.92	.12	.18	.25	.29	-.72	-.00	.11	.17	-114.74	-65.01	-3.52	1.09	-2.29	-7.98
16.0	-1.25	-.03	.08	.12	.19	-1.02	-.15	-.05	.02	-129.41	-79.12	-5.09	1.62	-2.58	-10.64
17.0	-1.58	-.17	-.09	-.07	.05	-1.31	-.29	-.20	-.18	-141.23	-89.37	-5.69	1.66	-2.86	-12.12
18.0	-1.81	-.35	-.24	-.27	.10	-1.40	-.44	-.37	-.36	-137.71	-88.66	-5.99	1.83	-3.03	-12.47
19.0	-1.24	-.47	-.34	-.48	-.27	-1.19	-.56	-.50	-.55	-117.80	-76.95	-5.45	1.96	-3.07	-12.10
20.0	-.98	-.57	-.53	-.64	-.44	-.94	-.67	-.67	-.72	-95.99	-63.63	-4.38	2.11	-3.02	-10.36
21.0	-.81	-.64	-.64	-.74	-.60	-.59	-.67	-.74	-.83	-63.77	-42.83	-3.12	2.09	-2.87	-7.16
22.0	-.19	-.64	-.70	-.81	-.73	-.05	-.64	-.78	-.87	-9.03	-9.70	-1.50	1.93	-2.53	-3.09
23.0	.57	-.54	-.72	-.79	-.81	.32	-.50	-.73	-.84	57.34	26.11	.40	1.74	-2.05	1.80
24.0	.74	-.40	-.63	-.68	-.82	.55	-.31	-.57	-.73	108.51	54.93	2.84	1.53	-1.27	8.15
25.0	.90	-.24	-.41	-.47	-.73	.71	-.15	-.34	-.48	133.20	72.92	5.14	1.11	-.09	14.46
26.0	.97	-.09	-.24	-.16	-.49	.81	-.00	-.12	-.15	140.55	80.03	5.81	.17	1.33	17.46
27.0	.92	.02	-.10	.13	-.15	.80	.14	.10	.14	135.10	78.59	6.69	-.72	1.89	16.25
28.0	.74	.16	.04	.34	.18	.68	.27	.24	.37	118.35	70.23	2.58	-1.05	1.21	12.65
29.0	.63	.27	.21	.43	.24	.60	.34	.43	.52	103.01	61.91	.19	-1.01	.18	8.17
30.0	.54	.35	.31	.44	.55	.57	.44	.59	.59	90.72	54.56	-1.23	-1.18	-.85	4.42
31.0	.56	.41	.39	.42	.64	.55	.59	.58	.62	77.36	47.37	-1.31	-1.30	-.89	1.57
32.0	.60	.46	.44	.41	.68	.62	.54	.59	.63	69.99	43.78	-.60	-1.14	-1.39	-.48
33.0	.74	.52	.52	.46	.67	.70	.58	.60	.61	67.43	42.31	.43	-.81	-1.74	-1.31
34.0	.77	.57	.57	.55	.65	.74	.61	.62	.60	64.20	40.36	1.11	-.61	-1.62	-.75
35.0	.69	.63	.63	.65	.65	.69	.66	.65	.61	54.31	34.88	1.22	-.58	-1.34	.59
36.0	.60	.64	.64	.74	.68	.63	.66	.67	.60	41.14	27.70	.64	-.45	-1.10	1.50
37.0	.44	.70	.73	.79	.74	.50	.66	.68	.70	20.35	16.22	-.46	-.22	-.99	1.16
38.0	.64	.64	.73	.68	.79	.62	.64	.64	.72	-25.89	-4.86	-1.60	-.03	-1.09	-.11
39.0	.66	.63	.71	.75	.81	.62	.69	.65	.71	-86.20	-38.46	-2.75	.17	-1.43	-2.51
40.0	.68	.68	.61	.68	.79	-.03	.31	.32	.64	-130.88	-67.81	-4.22	.51	-1.94	-7.08
41.0	-1.53	.32	.45	.50	.70	-1.16	.14	.33	.45	-162.62	-85.66	-5.70	1.05	-2.74	-12.67
42.0	-1.71	.13	.26	.28	.51	-1.39	-.05	.11	.18	-166.83	-101.08	-6.51	1.71	-3.56	-16.62
43.0	-1.69	-.02	.09	.04	.27	-1.43	-.21	-.12	-.10	-159.69	-100.74	-6.94	2.16	-3.86	-17.69
44.0	-1.61	-.19	-.10	-.21	.02	-1.25	-.34	-.33	-.37	-141.85	-90.32	-6.07	2.38	-3.67	-16.17
45.0	-1.02	-.32	-.25	-.39	-.19	-.97	-.49	-.49	-.57	-118.83	-75.32	-4.76	2.68	-3.36	-12.82
46.0	-.69	-.40	-.38	-.52	-.38	-.55	-.52	-.62	-.70	-84.58	-53.32	-3.36	2.68	-3.06	-8.71
47.0	.04	-.44	-.47	-.58	-.53	-.12	-.52	-.66	-.76	-40.92	-24.45	-1.90	2.85	-2.67	-4.23
48.0	.67	-.41	-.52	-.59	-.62	.27	-.44	-.63	-.75	15.38	5.78	-.45	2.32	-2.16	.69
49.0	.74	-.31	-.48	-.52	-.66	.57	-.27	-.51	-.64	82.72	40.97	1.38	2.03	-1.49	6.55
50.0	.68	-.14	-.33	-.38	-.60	.72	-.08	-.31	-.44	117.75	61.79	3.49	1.70	-.54	12.38
51.0	.68	.00	-.15	-.14	-.43	.75	.06	-.07	-.14	125.52	64.90	5.07	1.10	.64	16.37
52.0	.77	.11	.01	.14	-.16	.69	.19	.14	.17	119.16	68.04	4.89	.07	1.51	16.45
53.0	.62	.20	.14	.34	.18	.59	.29	.32	.41	104.62	60.50	3.33	-.59	1.41	12.49
54.0	.58	.29	.26	.51	.44	.55	.34	.45	.55	92.50	54.33	.49	-.65	.52	8.07
55.0	.68	.34	.35	.52	.60	.68	.45	.50	.61	76.28	45.26	-1.48	-.73	-.68	3.79
56.0	.64	.43	.42	.67	.68	.65	.59	.50	.63	61.48	37.97	-2.14	-.91	-1.09	.94
57.0	.66	.47	.48	.67	.69	.67	.53	.50	.63	49.73	32.20	-1.59	-.86	-1.50	-.99
58.0	.59	.50	.51	.63	.68	.58	.59	.57	.60	46.61	30.48	-.39	-.59	-1.77	-2.39
59.0	.63	.54	.54	.69	.64	.61	.57	.57	.56	42.99	28.48	.53	-.25	-1.89	-2.44
60.0	.53	.58	.57	.57	.60	.55	.59	.58	.54	32.09	22.46	.90	-.10	-1.76	-1.10
61.0	.53	.60	.62	.66	.60	.55	.58	.59	.57	25.56	19.52	.52	-.06	-1.62	.19
62.0	.53	.62	.64	.71	.62	.59	.57	.59	.60	4.89	7.57	-.58	.10	-1.12	.31
63.0	.62	.60	.62	.76	.66	.55	.52	.58	.60	-34.09	-12.04	-1.75	.31	-1.18	-.70
64.0	.61	.53	.60	.63	.68	.55	.41	.54	.59	-79.16	-35.58	-2.92	.52	-1.51	-2.75
65.0	-1.06	.42	.50	.55	.66	-.75	.26	.49	.52	-133.89	-71.70	-4.20	.76	-1.93	-6.40
66.0	-1.08	.23	.35	.40	.57	-1.13	.06	.24	.36	-152.76	-90.80	-5.22	1.15	-2.56	-11.23
67.0	-1.70	.05	.19	.21	.42	-1.39	-.11	.04	.13	-161.49	-100.21	-6.21	1.69	-3.26	-15.32
68.0	-1.69	-.10	.00	-.03	.20	-1.43	-.26	-.18	-.15	-155.55	-98.93	-6.51	2.07	-3.69	-16.45
69.0	-1.43	-.25	-.16	-.25	-.04	-1.28	-.39	-.37	-.41	-138.90	-88.68	-5.98	2.35	-3.65	-15.57
70.0	-1.01	-.38	-.32	-.45	-.26	-.97	-.50	-.54	-.60	-113.46	-72.81	-4.95	2.35	-3.35	-12.43
71.0	-.51	-.67	-.44	-.58	-.43	-.57	-.56	-.65	-.73	-80.52	-51.98	-3.36	2.35	-2.99	-8.67

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CHANNEL OUTPUT FOR HOUR 30

NTIME= 450

ALL M VALUES IN FEET, ALL W VALUES IN CFS

M	I	J	MX	QXN	QXP	MY	QYN	QYP	MC	QXT	QXF	QYT	QYF
CHANNEL REACH 1													
1	HR	1	.590	98718.	90953.	0.000	0.	0.	.518	0.	0.	0.	0.
2	0	2	.519	98905.	90852.	0.000	0.	0.	.445	0.	0.	0.	0.
3	0	3	.445	98852.	90518.	.375	-87013.	-40538.	.409	0.	-0.	0.	3105.
4	7	3	0.000	0.	0.	0.000	0.	0.	.375	0.	0.	0.	0.
5	7	4	.375	87013.	87151.	.338	-21580.	-21809.	.344	0.	-809.	0.	0.
6	6	4	0.000	0.	0.	0.000	0.	0.	.338	0.	0.	0.	0.
7	6	5	.338	21580.	19164.	.272	0.	0.	.334	-2172.	0.	0.	0.
8	6	6	.334	19164.	18953.	.294	-2163.	-2232.	.328	0.	0.	0.	0.
9	6	7	.328	18953.	18507.	.178	-7511.	-7577.	.319	0.	0.	0.	0.
10	7	7	0.000	0.	0.	.319	5488.	5234.	.332	0.	0.	0.	0.
11	8	7	0.000	0.	0.	.332	5234.	5013.	.346	0.	0.	0.	0.
12	8	8	.346	5013.	4844.	.314	0.	0.	.360	0.	0.	0.	0.
13	9	8	0.000	0.	0.	.360	4844.	4865.	.373	0.	0.	0.	0.
14	9	9	.373	4865.	4893.	0.000	0.	0.	.386	0.	0.	0.	0.
15	9	10	.386	4893.	472.	0.000	0.	0.	.396	0.	3479.	0.	0.
16	10	11	.396	472.	-1295.	.405	1343.	1296.	.401	0.	0.	0.	0.
17	8	11	0.000	0.	0.	.410	1375.	1363.	.405	0.	0.	0.	0.
18	7	11	0.000	0.	0.	0.000	0.	0.	.410	0.	0.	0.	0.
19	7	12	.410	-1375.	-1395.	.418	1404.	1395.	.414	0.	0.	0.	0.
20	6	12	0.000	0.	0.	.422	1404.	1404.	.414	0.	0.	0.	0.
21	5	12	0.000	0.	0.	0.000	0.	0.	.422	0.	0.	0.	0.
22	5	13	.422	-1404.	-1402.	0.000	0.	0.	.425	0.	0.	0.	0.
23	5	14	.425	-1402.	-1394.	.432	1342.	1344.	.429	0.	0.	0.	0.
24	4	14	0.000	0.	0.	0.000	0.	0.	.432	0.	0.	0.	0.
25	4	15	.432	-1394.	-1384.	0.000	0.	0.	.439	0.	0.	0.	0.
26	5	15	.439	-1384.	-1369.	.434	-1369.	-1368.	.442	0.	0.	0.	0.
27	5	16	.442	-1368.	-1324.	.487	1297.	1324.	.444	0.	0.	0.	0.
28	4	16	.444	-1324.	-1297.	0.000	0.	0.	.447	0.	0.	0.	0.
29	4	17	.447	-1297.	-1267.	.451	12.	449.	.449	0.	0.	0.	0.
30	5	17	.449	-1267.	-1207.	.449	-1223.	-1197.	.451	0.	0.	0.	0.
31	5	18	.451	-1207.	-1180.	.453	1130.	1160.	.452	0.	0.	0.	0.
32	4	18	.452	-1180.	-1100.	0.	0.	0.	.453	0.	0.	0.	0.
33	-4	19	.453	-1100.	-1100.	0.000	0.	0.	.454	0.	0.	0.	0.
CHANNEL REACH 2													
34	3	17	0.000	0.	0.	0.000	0.	0.	.451	0.	0.	0.	0.
35	3	18	.451	-32.	-19.	.453	10.	19.	.452	0.	0.	0.	0.
36	-2	18	0.000	0.	0.	.453	0.	10.	.453	0.	0.	0.	0.

WATER LEVEL HYDROGRAPHS (FT) AND KEY FLOWS (1000 CFS)

HOUR	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16
0.0	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
72.0	.04	-.50	-.54	-.64	-.58	-.12	-.55	-.60	-.80	-35.43	-23.90	-1.96	2.34	-2.63	-4.16	0.00
73.0	.04	-.45	-.57	-.64	-.67	.31	-.67	-.67	-.78	27.12	11.46	-.28	2.17	-2.17	-.88	0.00
74.0	.04	-.33	-.52	-.54	-.71	.57	-.27	-.51	-.67	90.96	84.98	1.83	1.92	-1.48	0.87	0.00
75.0	.02	-.18	-.36	-.41	-.65	.74	-.09	-.31	-.48	124.02	88.02	0.04	1.60	-.44	13.15	0.00
76.0	.04	-.02	-.17	-.15	-.45	.80	.08	-.08	-.15	131.81	74.08	9.51	.85	.86	17.08	0.00
77.0	.04	.09	-.02	.15	-.14	.75	.18	.14	.17	125.43	72.07	2.99	-.18	1.49	16.77	0.00
78.0	.04	.20	.13	.39	.19	.61	.30	.32	.41	108.02	63.03	3.19	-.71	1.42	13.18	0.00
79.0	.04	.30	.26	.51	.45	.45	.34	.46	.56	86.31	51.12	.52	-.78	.87	9.30	0.00
80.0	.04	.34	.35	.51	.60	.45	.43	.54	.62	74.22	44.44	-1.01	-.69	-.62	3.08	0.00
81.0	.01	.41	.41	.46	.66	.49	.49	.57	.63	65.23	39.68	-2.53	-1.01	-1.06	.21	0.00
82.0	.04	.46	.44	.40	.70	.57	.53	.58	.61	56.48	37.24	-1.77	-.88	-1.02	-1.70	0.00
83.0	.04	.51	.50	.44	.66	.60	.57	.58	.57	57.11	36.40	-.25	-.56	-1.08	-2.10	0.00
84.0	.04	.56	.56	.47	.61	.73	.60	.56	.56	56.34	36.49	1.12	-.33	-1.84	-1.10	0.00
85.0	.01	.60	.61	.59	.60	.78	.64	.61	.58	59.65	37.05	1.75	-.29	-1.81	-.85	0.00
86.0	.04	.66	.66	.72	.63	.81	.68	.64	.62	58.45	36.55	1.40	-.23	-1.68	1.90	0.00
87.0	.03	.72	.72	.80	.69	.68	.71	.69	.68	42.36	27.00	-.33	-.09	-.92	2.73	0.00
88.0	.29	.75	.77	.84	.76	.60	.69	.73	.76	4.48	4.48	-.73	.01	-.81	2.94	0.00
89.0	-.10	.72	.79	.83	.83	.67	.61	.73	.79	-43.11	-16.30	-1.79	.06	-.80	-.67	0.00
90.0	-.71	.61	.73	.78	.89	-.44	.47	.65	.77	-110.07	-54.85	-3.18	.25	-1.24	-3.71	0.00
91.0	-1.31	.45	.59	.66	.86	-.46	.27	.50	.64	-151.77	-86.42	-.92	.76	-2.26	-6.80	0.00
92.0	-1.09	.28	.41	.57	.71	-1.19	.08	.24	.40	-103.02	-66.66	-.63	1.05	-3.38	-15.59	0.00
93.0	-1.59	.11	.22	.21	.46	-1.31	-.97	.05	.09	-163.83	-100.74	-7.04	1.98	-3.94	-18.49	0.00
94.0	-1.44	-.04	.05	-.04	.20	-1.27	-.22	-.17	-.20	-151.40	-95.13	-.67	2.37	-3.93	-17.45	0.00
95.0	-1.06	-.19	-.11	-.26	-.04	-.98	-.35	-.34	-.43	-127.23	-79.63	-.53	2.47	-3.55	-14.55	0.00

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CHANNEL REACH 3

37	10	10	0.000	0.	0.	.396	2106.	1266.	.390	0.	0.	-.023.	0.
38	11	10	0.000	0.	0.	.390	1266.	366.	.416	0.	0.	-.066.	0.
39	12	10	0.000	0.	0.	.416	366.	-.924.	.467	0.	0.	-.1211.	0.
40	13	11	-.007	-.924.	-1015.	.505	-.355.	-.643.	.496	0.	0.	0.	0.
41	13	11	0.000	0.	0.	.496	-1456.	-1562.	.511	0.	0.	0.	0.
42	14	11	0.000	0.	0.	.511	-1562.	-1627.	.527	0.	0.	0.	0.
43	14	12	-.527	-.452.	-.535.	.543	616.	.535.	.535	0.	0.	0.	0.
44	13	12	.511	0.	0.	0.000	0.	0.	.543	0.	0.	0.	0.
45	13	13	.543	-.616.	-.645.	0.000	0.	0.	.549	0.	0.	0.	0.
46	14	13	.535	0.	0.	.549	-.645.	-.772.	.555	0.	0.	0.	0.
47	14	14	.555	-.772.	-.962.	0.000	0.	0.	.557	0.	0.	0.	0.
48	15	14	0.000	0.	0.	.557	-.962.	-1094.	.560	0.	0.	0.	0.
49	15	15	.560	-1094.	-1224.	.560	1267.	1224.	.562	0.	0.	0.	0.
50	14	15	.557	0.	0.	0.000	0.	0.	.568	0.	0.	0.	0.
51	14	16	.564	-1297.	-1368.	0.000	0.	0.	.573	0.	0.	0.	0.
52	14	17	.573	-1368.	-1439.	0.000	0.	0.	.586	0.	0.	0.	0.
53	14	18	.586	-1439.	-1470.	0.000	0.	0.	.592	0.	0.	0.	0.
54	-14	19	.752	-1470.	-1500.	0.000	0.	0.	.917	0.	0.	0.	0.

CHANNEL REACH 4

55	11	11	.416	0.	0.	0.000	0.	0.	.505	0.	0.	0.	0.
56	11	12	.505	.355.	.266.	.522	-176.	-.266.	.515	0.	0.	0.	0.
57	10	12	0.000	0.	0.	0.000	0.	0.	.522	0.	0.	0.	0.
58	10	13	.522	.176.	.89.	.531	-.89.	-.99.	.526	0.	0.	0.	0.
59	9	13	0.000	0.	0.	0.000	0.	0.	.531	0.	0.	0.	0.
60	-9	14	.531	.48.	0.	0.000	0.	0.	.532	0.	0.	0.	0.

CHANNEL REACH 5

61	15	11	0.000	0.	0.	.527	-1175.	-1304.	.534	0.	0.	0.	0.
62	16	11	0.000	0.	0.	.534	-1304.	-1434.	.538	0.	0.	0.	0.
63	17	11	0.000	0.	0.	.538	-1434.	-1564.	.540	0.	0.	0.	0.
64	18	11	0.000	0.	0.	.540	-1564.	-1693.	.540	0.	0.	0.	0.
65	19	11	.532	1947.	1821.	.540	-1693.	-1821.	.537	0.	0.	0.	0.
66	19	10	0.000	0.	0.	0.000	0.	0.	.532	0.	0.	0.	0.
67	20	10	0.000	0.	0.	.532	-1947.	-2067.	.526	0.	0.	0.	0.
68	21	10	0.000	0.	0.	.526	-2067.	-2180.	.518	0.	0.	0.	0.
69	22	10	0.000	0.	0.	.518	-2180.	-2243.	.508	0.	0.	0.	0.
70	23	10	-.484	4749.	5677.	.509	-2243.	-2373.	.500	0.	0.	0.	0.

CHANNEL REACH 6

71	-22	1	.590	54557.	54640.	0.000	0.	0.	.574	0.	0.	0.	0.
72	23	1	0.000	0.	0.	.574	12749.	12643.	.565	0.	0.	0.	0.
73	23	2	.565	12643.	12666.	.544	-12644.	-12666.	.555	0.	0.	0.	0.
74	22	2	.574	41852.	41875.	0.000	0.	0.	.544	0.	0.	0.	0.
75	22	3	.544	54759.	54752.	0.000	0.	0.	.495	0.	0.	0.	0.
76	22	4	.495	54752.	41901.	0.000	0.	0.	.371	0.	13240.	0.	0.
77	22	5	.371	41901.	25348.	0.000	0.	0.	.372	-11303.	5237.	0.	0.
78	23	5	0.000	0.	0.	.372	25348.	20711.	.393	0.	0.	-.4574.	0.
79	23	6	.393	20711.	20625.	0.000	0.	0.	.415	0.	0.	0.	0.
80	23	7	.415	20625.	20533.	0.000	0.	0.	.438	0.	0.	0.	0.
81	23	8	.438	20533.	20434.	0.000	0.	0.	.461	0.	0.	0.	0.
82	23	9	.461	20434.	20327.	0.000	0.	0.	.484	0.	0.	0.	0.
83	24	9	0.000	0.	0.	.484	14539.	2173.	.495	0.	0.	-.12163.	0.
84	24	10	.495	2173.	1949.	.500	3304.	3027.	.508	0.	0.	0.	0.
85	24	11	.508	4423.	4210.	0.000	0.	0.	.524	0.	0.	0.	0.
86	24	12	.524	4210.	3944.	0.000	0.	0.	.542	0.	0.	0.	0.
87	24	13	.542	3944.	3750.	0.000	0.	0.	.554	0.	0.	0.	0.
88	25	13	0.000	0.	0.	.554	3750.	3532.	.566	0.	0.	0.	0.
89	25	14	.566	3532.	2353.	0.000	0.	0.	.581	0.	1115.	0.	0.
90	26	14	0.000	0.	0.	.581	2353.	-389.	.589	0.	0.	-.2594.	0.
91	27	14	0.000	0.	0.	.589	-389.	-.526.	.591	0.	0.	0.	0.
92	27	15	.591	-.526.	-.665.	0.000	0.	0.	.592	0.	0.	0.	0.
93	-28	15	0.000	0.	0.	.592	-.665.	-.800.	.592	0.	0.	0.	0.

CHANNEL REACH 7

94	25	10	0.000	0.	0.	.598	553.	466.	.523	0.	0.	0.	0.
95	26	10	.547	-.285.	-.377.	.523	466.	377.	.536	0.	0.	0.	0.
96	26	9	0.000	0.	0.	0.000	0.	0.	.547	0.	0.	0.	0.
97	27	9	.541	-.95.	-190.	.547	265.	190.	.554	0.	0.	0.	0.
98	27	8	0.000	0.	0.	0.000	0.	0.	.561	0.	0.	0.	0.
99	-28	8	0.000	0.	0.	.561	95.	0.	.563	0.	0.	0.	0.

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CHANNEL REACH 8

100	-1	4	0.000	0.	0.	.255	0.	-.75.	.254	0.	0.	0.	0.
101	2	4	0.000	0.	0.	.254	-.75.	-.151.	.252	0.	0.	0.	0.
102	3	4	0.000	0.	0.	.252	-.151.	-.230.	.248	0.	0.	0.	0.
103	3	5	.248	-.230.	-.310.	0.000	0.	0.	.244	0.	0.	0.	0.
104	4	5	0.000	0.	0.	.244	0.	-.2015.	.251	0.	0.	0.	1051.
105	5	5	0.000	0.	0.	.251	-.2015.	-.2042.	.272	0.	0.	0.	0.
106	5	6	.272	-.2042.	-.2163.	0.000	0.	0.	.244	0.	0.	0.	0.
107	5	7	.244	0.	0.	0.000	0.	0.	.178	0.	0.	0.	0.
108	5	8	.178	7511.	3773.	.150	-.240.	-.3773.	.154	-.3711.	0.	-.3434.	0.
109	4	8	0.000	0.	0.	.143	0.	0.	.150	0.	0.	0.	0.
110	4	9	.150	240.	241.	.145	-.248.	-.241.	.147	0.	0.	0.	0.
111	3	9	.143	-.77.	-.105.	0.000	0.	0.	.145	0.	0.	0.	0.
112	3	8	0.000	0.	0.	.143	-.50.	-.77.	.145	0.	0.	0.	0.
113	2	8	0.000	0.	0.	.142	-.25.	-.60.	.143	0.	0.	0.	0.
114	-1	8	0.000	0.	0.	.142	0.	-.25.	.142	0.	0.	0.	0.

CHANNEL REACH 9

115	3	10	.145	103.	74.	.142	-.48.	-.74.	.143	0.	0.	0.	0.
116	2	10	0.000	0.	0.	0.000	0.	0.	.142	0.	0.	0.	0.
117	2	11	.142	48.	23.	0.000	0.	0.	.142	0.	0.	0.	0.
118	-2	12	.142	23.	0.	0.000	0.	0.	.141	0.	0.	0.	0.

CHANNEL REACH 10

119	6	8	.319	3491.	3383.	-.154	0.	0.	.317	0.	0.	0.	0.
120	-7	8	.332	0.	0.	.317	3383.	0.	.314	0.	0.	0.	3354.

CHANNEL REACH 11

121	-7	5	.344	65342.	61083.	.334	0.	0.	.246	-.3498.	0.	0.	0.
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VOLUME OF WATER ABOVE MRL = 1535.4 MILLIONS OF CU FT
(THE BEAVERD RO-S THRU Jo 2 ARE EXCLUDED)

CHANNEL OUTPUT FOR HOUR 60

NTIME= 900

ALL H VALUES IN FEET. ALL Q VALUES IN CFS

K	I	J	HX	QXN	QXP	HV	QVN	QVP	HC	QXT	QXF	QYT	QYF
CHANNEL REACH 1													
1	-8	1	.930	32086.	32799.	0.000	0.	0.	.584	0.	0.	0.	0.
2	8	2	.584	32799.	33388.	0.000	0.	0.	.557	0.	0.	0.	0.
3	8	3	.557	33388.	33948.	.578	-.35602.	-.33948.	.569	0.	0.	0.	-1100.
4	7	3	0.000	0.	0.	0.000	0.	0.	.578	0.	0.	0.	0.
5	7	4	.578	34602.	34317.	.569	-.12523.	-.12457.	.564	0.	-.3649.	0.	0.
6	6	4	0.000	0.	0.	0.000	0.	0.	.569	0.	0.	0.	0.
7	6	5	.569	12523.	11319.	.561	0.	0.	.569	0.	0.	0.	0.
8	6	6	.562	11319.	11248.	.562	-.2377.	-.2348.	.564	-.1242.	0.	0.	0.
9	6	7	.568	8400.	8842.	.488	-.6672.	-.6634.	.581	0.	0.	0.	0.
10	7	7	0.000	0.	0.	.561	.35.	-.50.	.579	0.	0.	0.	0.
11	8	7	0.000	0.	0.	.579	-.50.	-.150.	.577	0.	0.	0.	0.
12	8	8	.577	-.150.	-.272.	.584	0.	0.	.574	0.	0.	0.	0.
13	9	8	0.000	0.	0.	.574	-.272.	-.404.	.572	0.	0.	0.	0.
14	9	9	.572	-.404.	-.548.	0.000	0.	0.	.569	0.	0.	0.	0.
15	9	10	.569	-.548.	-.346.	0.000	0.	0.	.568	0.	-.313.	0.	0.
16	9	11	.568	904.	829.	.567	-.745.	-.629.	.567	0.	0.	0.	0.
17	8	11	0.000	0.	0.	.567	-.629.	-.745.	.567	0.	0.	0.	0.
18	7	11	0.000	0.	0.	0.000	0.	0.	.567	0.	0.	0.	0.
19	7	12	.567	.655.	.558.	.567	-.459.	-.558.	.567	0.	0.	0.	0.
20	6	12	0.000	0.	0.	.568	-.362.	-.450.	.567	0.	0.	0.	0.
21	5	12	0.000	0.	0.	0.000	0.	0.	.568	0.	0.	0.	0.
22	5	13	.568	362.	265.	0.000	0.	0.	.568	0.	0.	0.	0.
23	5	14	.568	265.	165.	.570	-.63.	-.165.	.569	0.	0.	0.	0.
24	4	14	0.000	0.	0.	0.000	0.	0.	.570	0.	0.	0.	0.
25	4	15	.570	63.	-.27.	0.000	0.	0.	.572	0.	0.	0.	0.
26	3	15	.569	0.	0.	.572	-.27.	-.169.	.572	0.	0.	0.	0.
27	3	16	.572	-.169.	-.272.	.574	.367.	.272.	.573	0.	0.	0.	0.
28	4	16	.572	0.	0.	0.000	0.	0.	.574	0.	0.	0.	0.
29	4	17	.574	-.397.	-.522.	.576	-.118.	-.161.	.575	0.	0.	0.	0.
30	5	17	.573	0.	0.	.575	-.683.	-.776.	.576	0.	0.	0.	0.
31	5	18	.576	-.776.	-.908.	.578	1053.	.908.	.577	0.	0.	0.	0.
32	6	18	.575	0.	0.	.577	0.	0.	.578	0.	0.	0.	0.
33	-6	19	.578	-.1003.	-.1100.	0.000	0.	0.	.578	0.	0.	0.	0.
CHANNEL REACH 2													
34	3	17	0.000	0.	0.	0.000	0.	0.	.576	0.	0.	0.	0.
35	3	18	.576	118.	65.	.578	-.33.	-.65.	.577	0.	0.	0.	0.
36	-2	18	0.000	0.	0.	.578	0.	-.33.	.578	0.	0.	0.	0.

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CHANNEL REACH 3

37	10	10	0.000	0.	0.	.500	-1200.	-1310.	.502	0.	0.	0.	0.
38	11	10	0.000	0.	0.	.502	-1310.	-1400.	.500	0.	0.	-101.	0.
39	12	10	0.000	0.	0.	.500	-1400.	-1000.	.017	0.	0.	-100.	0.
40	12	11	.017	-1000.	-1070.	.017	0.	0.	.012	0.	0.	0.	0.
41	13	11	0.000	0.	0.	.012	-1000.	-1001.	.000	0.	0.	0.	0.
42	14	11	0.000	0.	0.	.000	-1001.	-1000.	.007	0.	0.	0.	0.
43	14	12	.007	-1700.	-1700.	.005	1700.	1700.	.000	0.	0.	0.	0.
44	13	12	.000	0.	0.	0.000	0.	0.	.005	0.	0.	0.	0.
45	13	13	.005	-1700.	-1700.	0.000	0.	0.	.005	0.	0.	0.	0.
46	14	13	.000	0.	0.	.005	-1700.	-1700.	.000	0.	0.	0.	0.
47	14	14	.000	-1700.	-1000.	0.000	0.	0.	.000	0.	0.	0.	0.
48	15	14	0.000	0.	0.	.000	-1000.	-1000.	.005	0.	0.	0.	0.
49	15	15	.005	-1000.	-1500.	.011	1500.	1500.	.000	0.	0.	0.	0.
50	14	15	.000	0.	0.	0.000	0.	0.	.011	0.	0.	0.	0.
51	14	16	.011	-1500.	-1500.	0.000	0.	0.	.010	0.	0.	0.	0.
52	14	17	.010	-1500.	-1500.	0.000	0.	0.	.010	0.	0.	0.	0.
53	14	18	.010	-1500.	-1500.	0.000	0.	0.	.000	0.	0.	0.	0.
54	-14	19	.000	-1500.	-1500.	0.000	0.	0.	.000	0.	0.	0.	0.

CHANNEL REACH 4

55	11	11	.500	0.	0.	0.000	0.	0.	.000	0.	0.	0.	0.
56	11	12	.000	0.	0.	.000	0.	0.	.000	0.	0.	0.	0.
57	10	12	0.000	0.	0.	0.000	0.	0.	.000	0.	0.	0.	0.
58	10	13	.000	0.	0.	.000	0.	0.	.000	0.	0.	0.	0.
59	9	13	0.000	0.	0.	0.000	0.	0.	.000	0.	0.	0.	0.
60	-9	14	.000	0.	0.	0.000	0.	0.	.000	0.	0.	0.	0.

CHANNEL REACH 5

61	15	11	0.000	0.	0.	.007	-00.	-05.	.000	0.	0.	0.	0.
62	10	11	0.000	0.	0.	.000	-05.	-05.	.005	0.	0.	0.	0.
63	17	11	0.000	0.	0.	.005	-05.	-100.	.000	0.	0.	0.	0.
64	10	11	0.000	0.	0.	.000	-100.	-100.	.000	0.	0.	0.	0.
65	19	11	.002	130.	110.	.000	-100.	-110.	.003	0.	0.	0.	0.
66	19	10	0.000	0.	0.	0.000	0.	0.	.002	0.	0.	0.	0.
67	20	10	0.000	0.	0.	.002	-130.	-102.	.000	0.	0.	0.	0.
68	21	10	0.000	0.	0.	.000	-102.	-102.	.000	0.	0.	0.	0.
69	22	10	0.000	0.	0.	.000	-102.	-102.	.000	0.	0.	0.	0.
70	23	10	.500	571.	560.	.500	-102.	-172.	.500	0.	0.	0.	0.

CHANNEL REACH 6

71	-22	1	.530	2700.	2320.	0.000	0.	0.	.505	0.	0.	0.	0.
72	23	1	0.000	0.	0.	.505	5100.	5500.	.500	0.	0.	0.	0.
73	23	2	.500	5500.	5000.	.500	-0200.	-5000.	.500	0.	0.	0.	0.
74	22	2	.500	10000.	10200.	0.000	0.	0.	.500	0.	0.	0.	0.
75	22	3	.500	20000.	20000.	0.000	0.	0.	.570	0.	0.	0.	0.
76	22	4	.570	20000.	10350.	0.000	0.	0.	.505	0.	0.	0.	0.
77	22	5	.505	10350.	7000.	0.000	0.	0.	.573	-2200.	0.	0.	0.
78	23	5	0.000	0.	0.	.573	7000.	5000.	.500	0.	0.	-2000.	0.
79	23	6	.500	5000.	5000.	0.000	0.	0.	.505	0.	0.	0.	0.
80	23	7	.505	5000.	5000.	0.000	0.	0.	.500	0.	0.	0.	0.
81	23	8	.500	5000.	5000.	0.000	0.	0.	.502	0.	0.	0.	0.
82	23	9	.502	5000.	5000.	0.000	0.	0.	.505	0.	0.	0.	0.
83	24	9	0.000	0.	0.	.505	5000.	-1000.	.505	0.	0.	-5500.	0.
84	24	10	.500	-1000.	-1100.	.505	300.	300.	.500	0.	0.	0.	0.
85	24	11	.500	-1100.	-1100.	0.000	0.	0.	.502	0.	0.	0.	0.
86	24	12	.500	-1100.	-1100.	0.000	0.	0.	.573	0.	0.	0.	0.
87	24	13	.573	-1100.	-1100.	0.000	0.	0.	.500	0.	0.	0.	0.
88	25	13	0.000	0.	0.	.500	-1100.	-1100.	.500	0.	0.	0.	0.
89	25	14	.500	-1100.	-1120.	0.000	0.	0.	.552	0.	-70.	0.	0.
90	26	14	0.000	0.	0.	.552	-1120.	-700.	.507	0.	0.	300.	0.
91	27	14	0.000	0.	0.	.507	-700.	-750.	.505	0.	0.	0.	0.
92	27	15	.505	-750.	-770.	0.000	0.	0.	.500	0.	0.	0.	0.
93	-28	15	0.000	0.	0.	.500	-770.	-000.	.500	0.	0.	0.	0.

CHANNEL REACH 7

94	25	10	0.000	0.	0.	.500	100.	300.	.502	0.	0.	0.	0.
95	26	10	.500	-200.	-200.	.502	300.	200.	.500	0.	0.	0.	0.
96	26	9	0.000	0.	0.	0.000	0.	0.	.500	0.	0.	0.	0.
97	27	9	.500	-00.	-100.	.500	-200.	100.	.505	0.	0.	0.	0.
98	27	8	0.000	0.	0.	0.000	0.	0.	.500	0.	0.	0.	0.
99	-28	8	0.000	0.	0.	.500	0.	0.	.500	0.	0.	0.	0.

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CHANNEL REACH 8

100	-1	4	0.000	0.	0.	.551	0.	-.53.	.551	0.	0.	0.	0.
101	2	4	0.000	0.	0.	.551	-.43.	-.100.	.551	0.	0.	0.	0.
102	3	4	0.000	0.	0.	.551	-.100.	-.100.	.551	0.	0.	0.	0.
103	3	5	.551	-150.	-205.	0.000	0.	0.	.551	0.	0.	0.	0.
104	4	5	0.000	0.	0.	.551	-205.	-2305.	.551	0.	0.	0.	2073.
105	5	5	0.000	0.	0.	.553	-2305.	-2305.	.561	0.	0.	0.	0.
106	5	6	.501	-2345.	-2377.	0.000	0.	0.	.560	0.	0.	0.	0.
107	5	7	.500	0.	0.	0.000	0.	0.	.480	0.	0.	0.	0.
108	5	8	.480	4002.	3371.	.474	-244.	-3371.	.475	-3215.	0.	-3075.	0.
109	4	8	0.000	0.	0.	.475	0.	0.	.474	0.	0.	0.	0.
110	4	9	.474	244.	224.	.475	-140.	-224.	.474	0.	0.	0.	0.
111	3	9	.475	-73.	-98.	0.000	0.	0.	.475	0.	0.	0.	0.
112	3	8	0.000	0.	0.	.475	-.48.	-.73.	.475	0.	0.	0.	0.
113	2	8	0.000	0.	0.	.475	-.24.	-.48.	.475	0.	0.	0.	0.
114	-1	8	0.000	0.	0.	.475	0.	-.24.	.475	0.	0.	0.	0.

CHANNEL REACH 9

115	3	10	.475	98.	72.	.475	-.47.	-.72.	.475	0.	0.	0.	0.
116	2	10	0.000	0.	0.	0.000	0.	0.	.475	0.	0.	0.	0.
117	2	11	.475	47.	23.	0.000	0.	0.	.475	0.	0.	0.	0.
118	-2	12	.475	23.	0.	0.000	0.	0.	.475	0.	0.	0.	0.

CHANNEL REACH 10

119	0	8	.501	2172.	2135.	.475	0.	0.	.503	0.	0.	0.	0.
120	-7	8	.579	0.	0.	.503	2115.	0.	.504	0.	0.	0.	2121.

CHANNEL REACH 11

121	-7	5	.504	23050.	23003.	.502	0.	0.	.505	30.	0.	0.	0.
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VOLUME OF WATER ABOVE MBL 0 3037.0 MILLIONS OF CU FT
(THE SEWARD RO-S THRU Jo 2 ARE EXCLUDED)

CHANNEL OUTPUT FOR HOUR 00

NTIME 1350

ALL M VALUES IN FEET, ALL Q VALUES IN CFS

K	I	J	ME	QXN	QXP	MY	QYN	QYP	MC	QXT	QXF	QYT	QYF
CHANNEL REACH 1													
1	-0	1	-.710	-110047.	-108011.	0.000	0.	0.	-.011	0.	0.	0.	0.
2	0	2	-.411	-170011.	-102003.	0.000	0.	0.	-.131	0.	0.	0.	0.
3	0	3	-.131	-102003.	-100000.	0.000	0.	0.	-.040	0.	0.	0.	-13912.
4	7	3	0.000	0.	0.	0.000	0.	0.	.211	0.	0.	0.	0.
5	7	4	.211	-03005.	-05702.	.410	11010.	12051.	.301	0.	4031.	0.	0.
6	0	4	0.000	0.	0.	0.000	0.	0.	.410	0.	0.	0.	0.
7	0	5	.410	-11010.	-06459.	.617	0.	0.	.069	4703.	0.	0.	0.
8	0	6	.069	-06459.	-5077.	.500	40.	105.	.510	0.	0.	0.	0.
9	0	7	.510	-57002.	-5358.	.604	-3537.	-3042.	.560	0.	0.	0.	0.
10	7	7	0.000	0.	0.	.504	-0603.	-0214.	.502	0.	0.	0.	0.
11	0	7	0.000	0.	0.	.592	-0214.	-7000.	.613	0.	0.	0.	0.
12	0	8	.613	-7000.	-7405.	.604	0.	0.	.633	0.	0.	0.	0.
13	0	8	0.000	0.	0.	.633	-7405.	-7000.	.603	0.	0.	0.	0.
14	0	9	.603	-7000.	-0003.	0.000	0.	0.	.671	0.	0.	0.	0.
15	0	10	.671	-0003.	-0202.	0.000	0.	0.	.685	-1015.	0.	0.	0.
16	0	11	.685	-3101.	-3010.	.710	2045.	3010.	.600	0.	0.	0.	0.
17	0	11	0.000	0.	0.	.721	2045.	2045.	.710	0.	0.	0.	0.
18	7	11	0.000	0.	0.	0.000	0.	0.	.721	0.	0.	0.	0.
19	7	12	.721	-2007.	-2535.	.700	2300.	2535.	.731	0.	0.	0.	0.
20	0	12	0.000	0.	0.	.707	2240.	2300.	.700	0.	0.	0.	0.
21	5	12	0.000	0.	0.	0.000	0.	0.	.707	0.	0.	0.	0.
22	5	13	.707	-2200.	-2153.	0.000	0.	0.	.754	0.	0.	0.	0.
23	5	14	.754	-2153.	-2000.	.707	1930.	2000.	.701	0.	0.	0.	0.
24	0	14	0.000	0.	0.	0.000	0.	0.	.707	0.	0.	0.	0.
25	0	15	.707	-1930.	-1850.	0.000	0.	0.	.770	0.	0.	0.	0.
26	5	15	.701	0.	0.	.770	-1850.	-1750.	.703	0.	0.	0.	0.
27	5	16	.703	-1750.	-1650.	.702	1565.	1600.	.707	0.	0.	0.	0.
28	4	16	.770	0.	0.	0.000	0.	0.	.702	0.	0.	0.	0.
29	4	17	.702	-1565.	-1470.	.700	71.	103.	.706	0.	0.	0.	0.
30	5	17	.701	0.	0.	.706	-1373.	-1300.	.700	0.	0.	0.	0.
31	5	18	.700	-1300.	-1225.	.802	1162.	1225.	.800	0.	0.	0.	0.
32	4	18	.706	0.	0.	.800	0.	0.	.802	0.	0.	0.	0.
33	-0	19	.802	-1102.	-1100.	0.000	0.	0.	.803	0.	0.	0.	0.
CHANNEL REACH 2													
34	3	17	0.000	0.	0.	0.000	0.	0.	.700	0.	0.	0.	0.
35	3	18	.700	-71.	-40.	.802	20.	20.	.800	0.	0.	0.	0.
36	-2	18	0.000	0.	0.	.802	0.	20.	.802	0.	0.	0.	0.

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CHANNEL REACH 3

37	10	10	0.000	0.	0.	.665	-1111.	-636.	.722	0.	0.	206.	0.
38	11	10	0.000	0.	0.	.722	-836.	-898.	.753	0.	0.	-88.	0.
39	12	10	0.000	0.	0.	.753	-858.	-1140.	.705	0.	0.	-300.	0.
40	12	11	.794	-1149.	-1157.	.422	108.	127.	.814	0.	0.	0.	0.
41	13	11	0.000	0.	0.	.814	-1051.	-1009.	.824	0.	0.	0.	0.
42	14	11	0.000	0.	0.	.824	-1005.	-963.	.833	0.	0.	0.	0.
43	14	12	.833	-1235.	-1221.	.802	1210.	1221.	.808	0.	0.	0.	0.
44	13	12	.824	0.	0.	0.000	0.	0.	.862	0.	0.	0.	0.
45	13	13	.802	-1216.	-1220.	0.000	0.	0.	.875	0.	0.	0.	0.
46	14	13	.808	0.	0.	.875	-1220.	-1232.	.886	0.	0.	0.	0.
47	14	14	.886	-1232.	-1270.	0.000	0.	0.	.891	0.	0.	0.	0.
48	15	14	0.000	0.	0.	.891	-1270.	-1317.	.897	0.	0.	0.	0.
49	15	15	.897	-1317.	-1363.	.910	1302.	1363.	.902	0.	0.	0.	0.
50	14	15	.891	0.	0.	0.000	0.	0.	.910	0.	0.	0.	0.
51	14	16	.910	-1302.	-1420.	0.000	0.	0.	.918	0.	0.	0.	0.
52	14	17	.918	-1420.	-1468.	0.000	0.	0.	.932	0.	0.	0.	0.
53	14	18	.932	-1468.	-1481.	0.000	0.	0.	1.088	0.	0.	0.	0.
54	-14	19	1.088	-1481.	-1500.	0.000	0.	0.	1.239	0.	0.	0.	0.

CHANNEL REACH 4

55	11	11	.753	0.	0.	0.000	0.	0.	.822	0.	0.	0.	0.
56	11	12	.822	-100.	-74.	.833	49.	74.	.828	0.	0.	0.	0.
57	10	12	0.000	0.	0.	0.000	0.	0.	.833	0.	0.	0.	0.
58	10	13	.833	-69.	-25.	.839	12.	25.	.836	0.	0.	0.	0.
59	9	13	0.000	0.	0.	0.000	0.	0.	.839	0.	0.	0.	0.
60	-9	14	.839	-12.	0.	0.000	0.	0.	.840	0.	0.	0.	0.

CHANNEL REACH 5

61	15	11	0.000	0.	0.	.833	242.	244.	.823	0.	0.	0.	0.
62	16	11	0.000	0.	0.	.823	244.	320.	.810	0.	0.	0.	0.
63	17	11	0.000	0.	0.	.810	320.	364.	.794	0.	0.	0.	0.
64	18	11	0.000	0.	0.	.794	364.	450.	.775	0.	0.	0.	0.
65	19	11	.730	-610.	-520.	.775	450.	520.	.754	0.	0.	0.	0.
66	19	10	0.000	0.	0.	0.000	0.	0.	.730	0.	0.	0.	0.
67	20	10	0.000	0.	0.	.730	610.	702.	.704	0.	0.	0.	0.
68	21	10	0.000	0.	0.	.704	702.	802.	.676	0.	0.	0.	0.
69	22	10	0.000	0.	0.	.676	802.	911.	.646	0.	0.	0.	0.
70	23	10	.587	-7209.	-2670.	.646	911.	1027.	.613	0.	0.	0.	0.

CHANNEL REACH 6

71	-22	1	-.710	-5887.	-53293.	0.000	0.	0.	-.512	0.	0.	0.	-0.
72	23	1	0.000	0.	0.	-.512	-13162.	-11233.	-.439	0.	0.	0.	0.
73	23	2	-.439	-11233.	-9330.	-.303	7512.	9330.	-.370	0.	0.	0.	0.
74	22	2	-.512	-9330.	-39105.	0.000	0.	0.	-.303	0.	0.	0.	0.
75	22	3	-.303	-9330.	-45377.	0.000	0.	0.	-.091	0.	0.	0.	0.
76	22	4	-.091	-45377.	-25130.	0.000	0.	0.	.161	0.	-19021.	0.	0.
77	22	5	.161	-25130.	-7846.	0.000	0.	0.	.268	-10962.	-27390.	0.	0.
78	23	5	0.000	0.	0.	.268	-7846.	-6313.	.302	0.	0.	3142.	0.
79	23	6	.302	-6313.	-4014.	0.000	0.	0.	.405	0.	0.	0.	0.
80	23	7	.405	-4014.	-3740.	0.000	0.	0.	.467	0.	0.	0.	0.
81	23	8	.467	-3740.	-3519.	0.000	0.	0.	.527	0.	0.	0.	0.
82	23	9	.527	-3519.	-3325.	0.000	0.	0.	.587	0.	0.	0.	0.
83	24	9	0.000	0.	0.	.587	-3325.	-3015.	.609	0.	0.	-3491.	0.
84	24	10	.609	-3015.	-3202.	.613	-1651.	-1249.	.627	0.	0.	0.	0.
85	24	11	.627	-3202.	-3407.	0.000	0.	0.	.654	0.	0.	0.	0.
86	24	12	.654	-3407.	-3699.	0.000	0.	0.	.683	0.	0.	0.	0.
87	24	13	.683	-3699.	-2784.	0.000	0.	0.	.703	0.	0.	0.	0.
88	25	13	0.000	0.	0.	.703	-2784.	-2492.	.722	0.	0.	0.	0.
89	25	14	.722	-2492.	-1850.	0.000	0.	0.	.766	0.	-508.	0.	0.
90	26	14	0.000	0.	0.	.766	-1850.	-1369.	.759	0.	0.	267.	0.
91	27	14	0.000	0.	0.	.759	-1369.	-1179.	.764	0.	0.	0.	0.
92	27	15	.764	-1179.	-909.	0.000	0.	0.	.767	0.	0.	0.	0.
93	-28	15	0.000	0.	0.	.767	-909.	-800.	.768	0.	0.	0.	0.

CHANNEL REACH 7

94	25	10	0.000	0.	0.	.627	-831.	-710.	.644	0.	0.	0.	0.
95	26	10	.670	445.	582.	.644	-710.	-582.	.650	0.	0.	0.	0.
96	26	9	0.000	0.	0.	0.000	0.	0.	.670	0.	0.	0.	0.
97	27	9	.683	153.	302.	.670	-845.	-302.	.678	0.	0.	0.	0.
98	27	8	0.000	0.	0.	0.000	0.	0.	.683	0.	0.	0.	0.
99	-28	8	0.000	0.	0.	.683	-153.	0.	.685	0.	0.	0.	0.

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CHANNEL REACH 8

100	-1	4	0.000	0.	0.	.703	0.	0.	.702	0.	0.	0.	0.
101	2	4	0.000	0.	0.	.702	49.	138.	.699	0.	0.	0.	0.
102	3	4	0.000	0.	0.	.699	138.	208.	.695	0.	0.	0.	0.
103	3	5	.695	208.	260.	0.000	0.	0.	.688	0.	0.	0.	0.
104	4	5	0.000	0.	0.	.688	240.	-141.	.662	0.	0.	0.	.497.
105	5	5	0.000	0.	0.	.662	-141.	-68.	.617	0.	0.	0.	0.
106	5	6	.617	-68.	40.	0.000	0.	0.	.569	0.	0.	0.	0.
107	5	7	.569	0.	0.	0.000	0.	0.	.604	0.	0.	0.	0.
108	5	8	.604	3537.	1958.	.649	71.	-1958.	.635	-1643.	0.	-2068.	0.
109	4	8	0.000	0.	0.	.649	0.	0.	.649	0.	0.	0.	0.
110	4	9	.649	-71.	-56.	.662	45.	56.	.665	0.	0.	0.	0.
111	3	9	.660	16.	23.	0.000	0.	0.	.662	0.	0.	0.	0.
112	3	8	0.000	0.	0.	.695	10.	16.	.695	0.	0.	0.	0.
113	2	8	0.000	0.	0.	.698	5.	10.	.695	0.	0.	0.	0.
114	-1	8	0.000	0.	0.	.699	0.	5.	.698	0.	0.	0.	0.

CHANNEL REACH 9

115	3	10	.662	-22.	-14.	.695	0.	14.	.690	0.	0.	0.	0.
116	2	10	0.000	0.	0.	0.000	0.	0.	.695	0.	0.	0.	0.
117	2	11	.695	-8.	-4.	0.000	0.	0.	.698	0.	0.	0.	0.
118	-2	12	.698	-8.	0.	0.000	0.	0.	.700	0.	0.	0.	0.

CHANNEL REACH 10

119	6	8	.569	-177.	3.	.635	0.	0.	.567	0.	0.	0.	0.
120	-7	8	.592	0.	0.	.597	3.	0.	.604	0.	0.	0.	.93.

CHANNEL REACH 11

121	-7	5	.361	-7329.	-61573.	.469	0.	0.	.566	10634.	0.	0.	0.
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VOLUME OF WATER ABOVE MSL = 4120.4 MILLIONS OF CU FT
(THE BEAARD RUNS THRU Jd 2 ARE EXCLUDED)

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APPENDIX G

IDENTIFICATION OF GAGES FOR SABINE-CALCASIEU HURRICANE CARLA VERIFICATION

Gages for Sabine-Calcasieu Hurricane Carla verification and time sequences of water level and flow at the identified gage for 60 hours are identified. Also included are listings of detailed channel output at 30 and 60 hours. For explanation of each column see Appendix C,7,b.

HURRICANE CARLA CALIBRATION FOR SABINE-CALCASIEU AREA

PERIOD OF RECORD- 0000 SEP 10 TO 0000 SEP 13, 1961

CALCULATIONS ALLOW FOR SUB-GRID SCALE CHANNELS AND BARRIERS

TIME SEQUENCES OF WATER LEVEL AND FLOW ARE SAVED FOR THE FOLLOWING PLACES-

GAGE 1 SABINE PASS, SOUTHWEST JETTY

GAGE 2 PORT ARTHUR, CE AREA OFFICE

GAGE 3 NORTH SABINE LAKE

GAGE 4 BEAUMONT, NECHES RIVER AND BRAKES BAYOU

GAGE 5 ORANGE NAVAL STATION, SABINE RIVER

GAGE 6 CAMERON, CALCASIEU PASS

GAGE 7 WEST END OF INTRACOASTAL WATERWAY

GAGE 8 SABINE PASS, COAST GUARD STATION

GAGE 9 LAKE CHARLES, CALCASIEU RIVER

FLOW 1 SABINE PASS INFLOW

FLOW 2 CALCASIEU PASS INFLOW

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WATER LEVEL HYDROGRAPHS (FT) AND KEY FLOWS (1000 CFS)

HOUR	1	2	3	4	5	6	7	8	9	1	2
0.0	3.20	3.20	3.20	3.20	3.20	3.20	3.20	3.20	3.20	0.00	0.00
1.0	4.83	3.08	3.16	3.27	3.25	4.49	3.55	4.58	3.08	103.46	92.03
2.0	5.17	3.42	3.15	3.30	3.24	4.50	3.75	4.41	3.14	148.22	128.51
3.0	5.50	3.54	3.59	3.32	3.21	4.67	3.88	4.88	2.97	132.58	171.48
4.0	5.73	3.65	3.80	3.41	3.19	4.79	3.94	5.03	2.86	160.98	199.86
5.0	5.97	3.76	3.95	3.53	3.17	4.93	4.04	5.20	2.89	177.48	216.70
6.0	6.20	3.91	4.18	3.69	3.20	5.08	4.19	5.43	2.83	179.68	233.63
7.0	6.13	4.09	4.36	3.83	3.20	5.09	4.38	5.51	2.90	169.05	235.17
8.0	6.07	4.33	4.57	3.96	3.22	5.03	4.53	5.57	2.88	158.33	233.57
9.0	6.00	4.43	4.73	4.12	3.24	4.97	4.70	5.59	2.91	143.54	231.81
10.0	5.93	4.68	4.92	4.28	3.31	4.91	4.90	5.65	2.94	128.34	229.76
11.0	5.87	5.02	5.07	4.44	3.39	4.86	4.97	5.68	2.92	111.24	225.86
12.0	5.80	5.01	5.21	4.62	3.49	4.79	5.30	5.72	2.94	90.78	223.02
13.0	5.93	5.14	5.34	4.82	3.62	4.81	5.33	5.85	2.92	79.31	229.31
14.0	6.07	5.23	5.43	4.99	3.77	4.86	5.52	6.00	2.89	64.62	219.04
15.0	6.20	5.34	5.54	5.11	3.92	4.93	5.71	6.15	2.85	55.37	208.26
16.0	6.30	5.45	5.69	5.16	4.02	4.95	5.87	6.27	2.80	46.64	207.14
17.0	6.40	5.54	5.83	5.23	4.15	4.95	5.98	6.37	2.79	42.11	209.08
18.0	6.50	5.64	5.95	5.30	4.32	4.99	6.05	6.47	2.79	42.56	200.16
19.0	6.40	5.72	6.04	5.36	4.44	4.93	6.16	6.46	2.82	34.18	202.00
20.0	6.30	5.80	6.19	5.42	4.58	4.85	6.28	6.44	2.91	23.28	200.10
21.0	6.20	5.85	6.26	5.47	4.77	4.78	6.37	6.40	3.03	3.39	278.99
22.0	6.13	5.89	6.30	5.57	4.98	4.68	6.44	6.37	3.18	-21.15	279.25
23.0	6.07	5.90	6.31	5.68	5.12	4.57	6.45	6.29	3.32	-42.18	286.38
24.0	6.00	5.92	6.30	5.81	5.23	4.51	6.48	6.24	3.44	-53.90	284.92
25.0	6.40	5.96	6.31	5.94	5.37	4.88	6.58	6.38	3.55	-27.87	293.24
26.0	6.80	6.06	6.40	6.06	5.63	5.00	6.81	6.70	3.68	16.03	312.52
27.0	7.20	6.35	6.63	6.17	5.89	5.28	7.07	7.07	3.82	74.96	330.25
28.0	7.13	6.56	6.82	6.32	6.04	5.36	7.28	7.14	3.98	93.92	328.19
29.0	7.07	6.73	6.94	6.57	6.08	5.35	7.40	7.16	4.10	99.89	322.78
30.0	7.00	6.86	6.98	6.77	6.20	5.33	7.57	7.10	4.18	92.34	317.63
31.0	6.60	6.93	7.01	7.22	6.42	5.15	7.67	6.90	4.26	62.47	300.55
32.0	6.20	6.97	7.08	7.30	6.63	4.94	7.71	6.70	4.36	21.24	278.01
33.0	5.80	6.98	7.02	7.47	6.77	4.73	7.74	6.45	4.48	-51.13	255.08
34.0	5.43	6.98	6.96	7.59	6.83	4.72	7.70	6.50	4.61	-93.79	256.08
35.0	6.07	7.00	6.92	7.85	6.83	4.79	7.68	6.58	4.68	-118.94	258.51
36.0	6.20	7.07	6.97	7.87	6.83	4.88	7.60	6.66	4.71	-120.69	269.57
37.0	6.33	7.11	7.05	7.71	6.89	4.98	7.59	6.78	4.72	-112.30	278.67
38.0	6.47	7.16	7.19	7.78	7.01	5.09	7.65	6.86	4.78	-98.08	286.56
39.0	6.60	7.32	7.38	7.90	7.23	5.18	7.71	6.90	4.89	-80.65	294.90
40.0	6.33	7.45	7.53	8.03	7.42	5.12	7.79	6.90	5.08	-90.35	290.06
41.0	6.07	7.54	7.60	8.17	7.51	5.03	7.92	6.81	5.28	-109.32	279.24
42.0	5.80	7.56	7.59	8.30	7.65	4.94	7.97	6.68	5.40	-141.29	267.21
43.0	5.30	7.57	7.59	8.42	7.78	4.78	8.00	6.47	5.44	-178.72	247.17
44.0	4.80	7.55	7.62	8.51	7.89	4.51	8.00	6.24	5.42	-223.01	229.47
45.0	4.30	7.52	7.62	8.61	7.96	4.24	7.98	6.07	5.41	-269.28	210.62
46.0	4.40	7.53	7.60	8.73	8.02	4.14	7.92	6.05	5.42	-283.19	208.77
47.0	4.50	7.55	7.61	8.84	8.06	4.13	7.82	6.11	5.42	-261.43	214.50
48.0	4.60	7.59	7.68	8.95	8.13	4.09	7.71	6.11	5.41	-240.30	221.54
49.0	4.87	7.62	7.73	9.07	8.24	4.22	7.57	6.23	5.41	-220.05	234.07
50.0	5.13	7.65	7.77	9.20	8.37	4.38	7.38	6.34	5.43	-190.80	255.91
51.0	5.40	7.70	7.87	9.31	8.56	4.56	7.21	6.46	5.47	-175.81	277.15
52.0	5.17	7.71	7.91	9.42	8.80	4.51	7.03	6.31	5.53	-182.30	274.94
53.0	4.93	7.89	7.81	9.52	8.82	4.43	6.98	6.18	5.63	-196.34	261.53
54.0	4.70	7.63	7.63	9.57	8.81	4.40	6.92	5.98	5.70	-210.66	242.78
55.0	4.29	7.57	7.50	9.53	8.72	4.28	6.88	5.82	5.79	-250.55	217.02
56.0	3.70	7.47	7.46	9.40	8.67	4.14	6.61	5.51	5.90	-267.89	177.37
57.0	3.20	7.38	7.39	9.27	8.69	3.93	6.25	5.05	6.13	-278.51	136.78
58.0	3.03	7.30	7.27	9.17	8.79	3.85	5.97	4.91	6.43	-264.63	102.29
59.0	2.87	7.16	7.04	9.06	8.85	3.91	5.84	4.78	6.69	-266.28	54.86

BEST AVAILABLE COPY

CHANNEL OUTPUT FOR HOURS 30

NTIME= 450

ALL H VALUES IN FEET, ALL Q VALUES IN CFS

K	I	J	HY	QHY	QAP	HT	QTN	QYP	HC	QXT	QXF	QYT	QYF
CHANNEL REACH 1													
1	-9	1	7.000	02300.	90785.	3.200	0.	0.	7.030	-25145.	-23084.	0.	0.
2	8	2	7.030	0-785.	95512.	3.200	0.	0.	7.071	-48402.	-52865.	0.	0.
3	8	3	7.071	05512.	124170.	7.156	-115427.	-124170.	7.101	-52801.	-81663.	32926.	41853.
4	7	3	3.200	0.	0.	3.200	0.	0.	7.156	0.	0.	0.	0.
5	7	4	7.156	115427.	00525.	7.162	-81445.	-100131.	7.191	-55840.	-262.	-44157.	-24058.
6	6	4	3.200	0.	0.	3.200	0.	0.	7.162	0.	0.	0.	0.
7	6	5	7.162	81445.	70309.	7.167	0.	0.	7.135	-67279.	-36038.	0.	0.
8	6	6	7.135	70309.	67195.	7.167	-3980.	-7865.	7.070	-65647.	-42090.	18013.	19912.
9	6	7	7.070	58501.	63865.	7.090	602.	-3562.	6.956	-64978.	-49653.	34173.	38299.
10	7	7	3.200	0.	0.	6.956	-14847.	-15132.	6.906	0.	0.	0.	0.
11	8	7	3.200	0.	0.	6.906	-15132.	-15494.	6.857	0.	0.	0.	0.
12	8	8	6.857	-15494.	-15862.	3.340	0.	0.	6.877	0.	0.	0.	0.
13	9	8	3.200	0.	0.	6.877	-15862.	-16177.	6.832	0.	0.	0.	0.
14	9	9	6.832	-16177.	-16428.	3.200	0.	0.	6.847	0.	0.	0.	0.
15	9	10	6.847	-16428.	-16711.	3.200	0.	0.	6.863	0.	-9546.	0.	0.
16	8	11	6.863	-9546.	-2074.	6.925	-8744.	-2074.	6.887	-92762.	-99309.	94131.	91328.
17	8	11	3.200	0.	0.	6.925	-10485.	-8744.	6.925	0.	0.	0.	-5916.
18	7	11	3.200	0.	0.	3.200	0.	0.	6.954	0.	0.	0.	0.
19	7	12	6.954	14485.	18298.	6.969	-18919.	-18298.	6.974	-33927.	-37880.	25680.	25687.
20	6	12	3.200	0.	0.	6.947	-19635.	-18919.	6.989	0.	0.	0.	-920.
21	5	12	3.200	0.	0.	3.200	0.	0.	6.997	0.	0.	0.	0.
22	5	13	6.997	19635.	20304.	3.200	0.	0.	6.987	0.	-976.	0.	0.
23	5	14	6.987	20304.	22691.	6.991	-22657.	-22691.	6.976	-45761.	-48437.	39196.	38930.
24	4	14	3.200	0.	0.	3.200	0.	0.	6.981	0.	0.	0.	0.
25	4	15	6.981	22691.	20181.	3.200	0.	0.	6.974	0.	2270.	0.	0.
26	5	15	6.974	0.	0.	6.974	20181.	18767.	6.946	0.	29302.	30322.	0.
27	5	16	6.946	18767.	15432.	6.946	-14940.	-15432.	6.945	17510.	29478.	-92.	0.
28	4	16	6.874	0.	0.	3.200	0.	0.	6.864	0.	0.	0.	0.
29	4	17	6.864	14940.	624.	6.863	-312.	-456.	6.840	-14055.	0.	0.	0.
30	5	17	6.840	0.	0.	6.840	173.	-130.	6.811	0.	0.	0.	0.
31	5	18	6.811	-130.	-539.	6.839	846.	339.	6.818	0.	0.	0.	0.
32	4	18	6.840	0.	0.	6.866	0.	0.	6.839	0.	0.	0.	0.
33	-9	19	6.839	-866.	-1152.	3.200	0.	0.	6.847	0.	0.	0.	0.
CHANNEL REACH 2													
34	3	17	3.200	0.	0.	3.200	0.	0.	6.863	0.	0.	0.	0.
35	3	18	6.863	312.	174.	6.892	-85.	-174.	6.866	0.	0.	0.	0.
36	-2	18	3.200	0.	0.	6.923	0.	-85.	6.892	0.	0.	0.	0.
CHANNEL REACH 3													
37	10	10	3.200	0.	0.	6.863	-2915.	-268.	6.795	0.	0.	53830.	51051.
38	11	10	3.200	0.	0.	6.795	-268.	218.	6.720	0.	0.	24697.	22353.
39	12	10	3.200	0.	0.	6.720	218.	432.	6.617	0.	0.	34781.	37519.
40	12	11	6.617	432.	6079.	6.721	5033.	8748.	6.618	-23417.	-25233.	-9157.	-12971.
41	13	11	3.200	0.	0.	6.618	14824.	10863.	6.636	0.	0.	26771.	24830.
42	14	11	3.200	0.	0.	6.636	19663.	17614.	6.721	0.	0.	44091.	47048.
43	14	12	6.636	-1396.	-1799.	6.735	653.	1769.	6.727	85475.	65711.	17485.	16328.
44	13	12	6.636	0.	0.	3.200	0.	0.	6.735	0.	0.	0.	0.
45	13	13	6.735	-853.	-252.	3.200	0.	0.	6.736	0.	-518.	0.	0.
46	14	13	6.722	0.	0.	6.736	-252.	290.	6.704	0.	0.	689.	0.
47	14	14	6.704	290.	-1060.	3.200	0.	0.	6.722	0.	1812.	0.	0.
48	15	14	3.200	0.	0.	6.722	-1866.	-2165.	6.191	0.	0.	10228.	10285.
49	15	15	6.191	-2165.	-1245.	6.235	930.	1265.	6.207	1161.	0.	470.	0.
50	14	15	6.222	0.	0.	3.200	0.	0.	6.235	0.	0.	0.	0.
51	14	16	6.235	-930.	-1113.	3.200	0.	0.	6.245	0.	0.	0.	0.
52	14	17	6.245	-1113.	-1325.	3.200	0.	0.	6.259	0.	0.	0.	0.
53	14	18	6.259	-1325.	-1440.	3.200	0.	0.	6.309	0.	0.	0.	0.
54	-14	19	6.309	-1440.	-1560.	3.200	0.	0.	6.373	0.	0.	0.	0.
CHANNEL REACH 4													
55	11	11	6.720	0.	0.	3.200	0.	0.	6.721	0.	0.	0.	0.
56	11	12	6.721	-8033.	-806.	6.708	172.	806.	6.750	0.	-4498.	361.	0.
57	10	12	3.200	0.	0.	3.200	0.	0.	6.788	0.	0.	0.	0.
58	10	13	6.788	-172.	-82.	6.829	36.	82.	6.801	0.	0.	0.	0.
59	9	13	3.200	0.	0.	3.200	0.	0.	6.829	0.	0.	0.	0.
60	-9	14	6.829	-36.	0.	3.200	0.	0.	6.860	0.	0.	0.	0.
CHANNEL REACH 5													
61	15	11	3.200	0.	0.	6.221	19010.	17622.	5.953	0.	0.	0.	1298.
62	16	11	3.200	0.	0.	5.953	17622.	15606.	5.692	0.	0.	0.	1711.
63	17	11	3.200	0.	0.	5.692	15606.	15606.	5.437	0.	0.	0.	0.
64	18	11	3.200	0.	0.	5.437	15606.	15612.	5.182	0.	0.	0.	0.
65	19	11	6.720	-12447.	-15395.	5.182	15512.	15395.	4.927	0.	2962.	0.	0.
66	19	10	3.200	0.	0.	3.200	0.	0.	4.720	0.	0.	0.	0.
67	20	10	3.200	0.	0.	4.720	12460.	12260.	4.506	0.	0.	0.	117.
68	21	10	3.200	0.	0.	4.506	12260.	4817.	4.419	0.	0.	0.	7718.
69	22	10	3.200	0.	0.	4.419	4817.	4511.	4.327	0.	0.	0.	0.
70	23	10	4.327	4511.	5173.	4.327	4511.	7718.	4.206	0.	0.	0.	-3400.

BEST AVAILABLE COPY

CHANNEL REACH 6

71	-22	1	7.000	517637.	535432.	3.200	0.	0.	6.002	-14111.	-37523.	0.	0.
72	23	1	3.200	0.	0.	0.	0.	0.	5.330	0.	0.	32077.	47784.
73	23	2	5.330	90863.	45244.	5.135	9628.	-45264.	5.072	9258.	67216.	-2334.	138703.
74	22	2	6.002	221125.	181244.	3.200	0.	0.	5.135	0.	41793.	0.	0.
75	22	3	5.135	171616.	136749.	3.200	0.	0.	4.768	0.	36195.	0.	0.
76	22	4	4.768	136749.	91654.	3.200	0.	0.	4.318	-27876.	19793.	0.	0.
77	22	5	4.318	91654.	62618.	3.200	0.	0.	4.265	-121662.	-92368.	0.	0.
78	23	5	3.200	0.	0.	0.	0.	0.	4.214	0.	0.	91177.	100122.
79	23	6	4.214	53652.	44880.	3.200	0.	0.	4.205	-6737.	0.	0.	0.
80	23	7	4.205	44880.	41430.	3.200	0.	0.	4.201	-5363.	0.	0.	0.
81	23	8	4.201	41430.	41319.	3.200	0.	0.	4.199	0.	0.	0.	0.
82	23	9	4.199	41319.	31814.	3.200	0.	0.	4.192	-9464.	0.	0.	0.
83	24	9	3.200	0.	0.	0.	0.	0.	4.192	26510.	18303.	0.	-14730.
84	24	10	4.192	18303.	11740.	4.206	12911.	9212.	4.164	-34437.	-27870.	21952.	27297.
85	24	11	4.164	11740.	6463.	3.200	0.	0.	4.163	447.	0.	0.	0.
86	24	12	4.163	6463.	13347.	3.200	0.	0.	4.206	7214.	0.	0.	0.
87	24	13	4.206	13347.	12997.	3.200	0.	0.	4.225	0.	0.	0.	0.
88	25	13	3.200	0.	0.	0.	0.	0.	4.204	0.	0.	0.	0.
89	25	14	4.204	12997.	10648.	3.200	0.	0.	4.220	0.	1897.	0.	0.
90	26	14	3.200	0.	0.	0.	0.	0.	4.220	0.	0.	0.	0.
91	27	14	3.200	0.	0.	0.	0.	0.	4.196	6910.	-462.	0.	-7162.
92	27	15	4.196	-462.	-683.	3.200	0.	0.	4.184	0.	0.	0.	0.
93	-28	15	3.200	0.	0.	0.	0.	0.	4.155	0.	0.	0.	0.

CHANNEL REACH 7

94	25	10	3.200	0.	0.	4.194	14666.	13275.	3.928	0.	0.	-1345.	0.
95	26	10	3.928	-1345.	-11328.	3.928	13275.	11328.	3.736	-11871.	-10231.	-1907.	0.
96	26	9	3.200	0.	0.	3.200	0.	0.	3.608	0.	0.	0.	0.
97	27	9	3.459	-200.	-5435.	3.608	9603.	5435.	3.502	-5108.	0.	13392.	17490.
98	27	8	3.200	0.	0.	3.200	0.	0.	3.459	0.	0.	0.	0.
99	-28	8	3.200	0.	0.	3.459	200.	0.	3.397	0.	0.	0.	0.

CHANNEL REACH 8

100	-1	4	3.200	0.	0.	7.603	0.	4713.	7.573	0.	0.	18551.	13965.
101	2	4	3.200	0.	0.	7.573	4713.	6561.	7.467	0.	0.	24550.	25509.
102	3	4	3.200	0.	0.	7.467	6561.	5027.	7.352	0.	0.	25098.	25645.
103	3	5	7.352	5027.	4621.	3.200	0.	0.	7.385	-25209.	-24380.	0.	0.
104	4	5	7.385	4621.	0.	0.	0.	0.	7.269	0.	0.	22611.	23746.
105	5	5	3.200	0.	0.	7.269	3361.	2408.	7.167	0.	0.	32412.	35683.
106	5	6	7.167	-2408.	-3989.	3.200	0.	0.	7.167	-21252.	-17676.	0.	0.
107	5	7	7.167	0.	0.	3.200	0.	0.	7.090	0.	0.	0.	0.
108	5	8	7.090	-682.	-5247.	7.240	4687.	5207.	7.153	29979.	34510.	63644.	62858.
109	4	8	3.200	0.	0.	7.456	0.	0.	7.240	0.	0.	0.	0.
110	4	9	7.240	-4687.	-3041.	7.412	94.	1061.	7.305	-32403.	-34062.	8351.	5303.
111	3	9	7.456	3557.	6915.	3.200	0.	0.	7.412	-42020.	-45538.	0.	0.
112	3	8	3.200	0.	0.	7.549	229.	3557.	7.456	0.	0.	-35191.	-38636.
113	2	8	3.200	0.	0.	7.732	270.	229.	7.599	0.	0.	0.	0.
114	-1	8	3.200	0.	0.	7.441	0.	270.	7.732	0.	0.	5977.	5704.

CHANNEL REACH 9

115	3	10	7.412	6822.	10088.	7.268	-5073.	-10088.	7.375	0.	-3453.	0.	4156.
116	2	10	3.200	0.	0.	3.200	0.	0.	7.268	0.	0.	0.	0.
117	2	11	7.268	4973.	2157.	3.200	0.	0.	7.256	0.	3805.	0.	0.
118	-2	12	7.256	2157.	0.	3.200	0.	0.	7.268	0.	2082.	0.	0.

CHANNEL REACH 10

119	6	8	6.956	75150.	68077.	7.153	0.	0.	6.423	12609.	20123.	0.	0.
120	-7	8	6.906	0.	0.	6.423	68077.	78401.	3.390	0.	0.	3729.	-7978.

CHANNEL REACH 11

121	-7	5	7.191	-30676.	-53478.	7.135	0.	0.	7.239	-69976.	-75915.	0.	0.
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VOLUME OF WATER ABOVE MSL = 162254.6 MILLIONS OF CU FT
(THE SEA-AND RULS THRU JM 2 ARE EXCLUDED)

BEST AVAILABLE COPY

CHANNEL OUTPUT FOR HOUR 40

NTIME 900

ALL H VALUES IN FEET, ALL G VALUES IN CFS

H	I	J	HX	QX	QAP	HY	QY	QYP	HC	QAT	QXF	QYT	QVF
CHANNEL REACH 1													
1	-8	1	2.704	-277231.	-276172.	3.200	0.	0.	3.501	0.	0.	0.	0.
2	8	2	3.501	-276172.	-275400.	3.200	0.	0.	4.223	0.	0.	0.	0.
3	8	3	4.223	-275400.	-316537.	5.002	30373.	316537.	4.714	0.	45900.	0.	-15033.
4	7	3	3.200	0.	0.	3.200	0.	0.	5.082	0.	0.	0.	0.
5	7	4	5.082	-30373.	-279910.	5.733	117952.	127117.	5.500	27000.	2360.	24313.	15000.
6	6	4	3.200	0.	0.	3.200	0.	0.	5.733	0.	0.	0.	0.
7	6	4	5.733	-117952.	-113370.	6.338	0.	0.	6.074	28000.	24012.	0.	0.
8	6	4	6.074	-113370.	-110470.	6.000	4350.	12300.	6.423	39551.	36001.	-5305.	-13573.
9	6	7	6.423	-48151.	-102754.	7.273	21320.	25057.	6.925	47027.	52305.	-13725.	-17461.
10	7	7	3.200	0.	0.	6.925	-30614.	-30000.	6.971	0.	0.	0.	0.
11	8	7	3.200	0.	0.	6.971	-30000.	-29033.	7.013	0.	0.	0.	0.
12	8	8	7.013	-29033.	-29177.	7.098	0.	0.	7.086	0.	0.	0.	0.
13	9	8	3.200	0.	0.	7.098	-29177.	-28708.	7.120	0.	0.	0.	0.
14	9	9	7.120	-28708.	-28223.	3.200	0.	0.	7.188	0.	0.	0.	0.
15	9	10	7.188	-28223.	-45000.	3.200	0.	0.	7.260	0.	17522.	0.	0.
16	9	11	7.260	-45000.	-60550.	7.067	40861.	46154.	7.469	190213.	191729.	-71043.	-72008.
17	8	11	3.200	0.	0.	7.469	43023.	44861.	7.627	0.	0.	0.	-1670.
18	7	11	3.200	0.	0.	3.200	0.	0.	7.774	0.	0.	0.	0.
19	7	12	7.774	-43023.	-43137.	6.992	41731.	43137.	7.957	33981.	34291.	-29230.	-30488.
20	6	12	3.200	0.	0.	6.992	42281.	41731.	8.192	0.	0.	970.	1000.
21	5	12	3.200	0.	0.	3.200	0.	0.	8.191	0.	0.	0.	0.
22	5	13	8.191	-42281.	-41917.	3.200	0.	0.	8.310	0.	-203.	0.	0.
23	5	14	8.310	-41917.	-40010.	8.513	37275.	40010.	8.430	44351.	42500.	-39092.	-41000.
24	4	14	3.200	0.	0.	3.200	0.	0.	8.513	0.	0.	0.	0.
25	4	15	8.513	-37275.	-36200.	3.200	0.	0.	8.919	0.	-900.	0.	0.
26	5	15	8.919	-36200.	0.	8.919	-36200.	-37260.	8.902	0.	0.	-6880.	-67500.
27	5	16	8.902	-37260.	-36650.	9.199	36410.	36650.	9.055	-25000.	-25189.	-35003.	-36100.
28	4	16	3.200	0.	0.	3.200	0.	0.	9.199	0.	0.	0.	0.
29	4	17	9.199	-36410.	-36700.	9.007	50000.	8221.	9.360	7461.	8000.	0.	-3200.
30	5	17	9.005	0.	0.	9.100	-28527.	-25022.	9.495	0.	0.	-20490.	-23500.
31	5	18	9.495	-28527.	-18774.	9.005	11582.	18774.	9.578	6900.	0.	3600.	-3257.
32	4	18	9.300	0.	0.	9.076	0.	0.	9.605	0.	0.	0.	0.
33	-4	19	9.004	-11582.	-1012.	3.200	0.	0.	9.652	4927.	-4720.	0.	0.
CHANNEL REACH 2													
34	3	17	3.200	0.	-2513.	3.200	0.	0.	9.407	0.	0.	0.	0.
35	3	18	9.407	-5000.	0.	9.005	74.	2513.	9.476	12050.	10235.	-1780.	-4181.
36	-2	18	3.200	0.	0.	9.409	0.	0.	9.465	0.	0.	0.	0.
CHANNEL REACH 3													
37	10	10	3.200	0.	0.	3.200	-215.	-3049.	7.207	0.	0.	-162320.	-159300.
38	11	10	3.200	0.	0.	7.207	-3049.	-8494.	7.308	0.	0.	-165500.	-161900.
39	12	10	3.200	0.	0.	7.308	-8494.	-10325.	7.600	0.	0.	12054.	10321.
40	12	11	7.600	-10325.	-13600.	7.809	11613.	9015.	7.702	-29432.	-25050.	13747.	15043.
41	13	11	3.200	0.	0.	7.702	-4249.	-5029.	7.872	0.	0.	6481.	10050.
42	14	11	3.200	0.	0.	7.872	-5029.	-8000.	7.951	0.	0.	-65100.	-93700.
43	14	12	7.951	-8000.	-10390.	8.310	17062.	10300.	8.101	-20880.	-20900.	-19415.	-18013.
44	13	12	7.872	0.	0.	3.200	0.	0.	8.310	0.	0.	0.	0.
45	13	13	8.310	-17062.	-17634.	3.200	0.	0.	8.527	0.	914.	0.	0.
46	14	13	8.101	0.	0.	8.527	-17634.	-18012.	8.721	0.	0.	36300.	37314.
47	14	14	8.721	-18012.	-18770.	3.200	0.	0.	8.836	44231.	44700.	0.	0.
48	15	14	3.200	0.	0.	8.836	-18770.	-17602.	8.963	0.	0.	-53785.	-50500.
49	15	15	8.963	-17602.	-15000.	9.298	14570.	15000.	9.110	-26370.	-26300.	-19700.	-20701.
50	14	15	8.836	0.	0.	3.200	0.	0.	9.298	0.	0.	0.	0.
51	14	16	9.298	-10570.	-13774.	3.200	0.	0.	9.594	0.	-1070.	0.	0.
52	14	17	9.594	-13774.	-8131.	3.200	0.	0.	9.572	0.	-3710.	0.	0.
53	14	18	9.572	-8131.	-2872.	3.200	0.	0.	9.936	0.	-2750.	0.	0.
54	-14	18	9.936	-2872.	-2000.	3.200	0.	0.	10.020	0.	0.	0.	0.
CHANNEL REACH 4													
55	11	11	7.308	0.	0.	3.200	0.	0.	7.800	0.	0.	0.	0.
56	11	12	7.800	-11613.	-12487.	7.700	8970.	12487.	7.822	314.	2201.	-4352.	-5257.
57	10	12	3.200	0.	0.	3.200	0.	0.	7.780	0.	0.	0.	0.
58	10	13	7.780	-8970.	-600.	8.074	30.	64.	7.900	0.	-7061.	0.	0.
59	9	13	3.200	0.	0.	3.200	0.	0.	8.074	0.	0.	0.	0.
60	-9	14	8.074	-30.	0.	3.200	0.	0.	8.169	0.	0.	0.	0.
CHANNEL REACH 5													
61	15	11	3.200	0.	0.	7.051	9680.	12720.	7.968	0.	0.	0.	-2020.
62	16	11	3.200	0.	0.	7.968	12720.	17030.	7.877	0.	0.	0.	-5090.
63	17	11	3.200	0.	0.	7.877	17030.	21112.	7.726	0.	0.	-17330.	-20577.
64	18	11	3.200	0.	0.	7.726	21112.	27915.	7.463	0.	0.	-10400.	-23000.
65	19	11	6.711	-10401.	-28000.	7.463	27915.	28000.	7.003	0.	9100.	0.	0.
66	19	10	3.200	0.	0.	3.200	0.	0.	7.111	0.	0.	0.	0.
67	20	10	3.200	0.	0.	6.711	19401.	10562.	6.550	0.	0.	0.	5120.
68	21	10	3.200	0.	0.	6.550	10562.	12030.	6.400	0.	0.	0.	2013.
69	22	10	3.200	0.	0.	6.400	12030.	12000.	6.310	0.	0.	0.	-11.
70	23	10	6.000	-30000.	-30750.	6.310	12000.	15070.	6.150	0.	0.	0.	-2853.

CHANNEL REACH 6

71	-22	1	3.475	-16073.	-15829.	3.200	0.	0.	3.858	0.	0.	0.	0.
72	23	1	3.200	0.	0.	3.858	33840.	-283.	4.007	0.	0.	0.	34918.
73	23	2	4.007	-283.	-28350.	4.196	41788.	28350.	4.182	32354.	60717.	4972.	20711.
74	22	2	3.858	-33840.	-64333.	3.200	0.	0.	4.196	0.	20488.	0.	0.
75	22	3	4.196	-41788.	-104547.	3.200	0.	0.	4.534	0.	-2476.	0.	0.
76	22	4	4.534	-104547.	-104670.	3.200	0.	0.	5.071	5752.	1513.	0.	0.
77	22	5	5.071	-104670.	-105541.	3.200	0.	0.	5.289	136440.	137361.	0.	0.
78	23	6	3.200	0.	0.	5.289	-105541.	-105477.	5.408	0.	0.	-103736.	-103290.
79	23	7	5.408	-105477.	-108005.	3.200	0.	0.	5.628	-415.	0.	0.	0.
80	23	8	5.628	-108005.	-108005.	3.200	0.	0.	5.800	5720.	0.	0.	0.
81	23	9	5.800	-108005.	-108780.	3.200	0.	0.	5.953	14151.	0.	0.	0.
82	23	0	5.953	-108780.	-108829.	3.200	0.	0.	6.080	21081.	0.	0.	0.
83	24	0	3.200	0.	0.	6.080	-25941.	-25245.	6.112	0.	0.	-117062.	-118253.
84	24	10	6.112	-25245.	-19686.	6.159	-24243.	-13077.	6.180	-70164.	-75321.	-45760.	-50227.
85	24	11	6.180	-24243.	-7980.	3.200	0.	0.	6.201	-34462.	-47842.	0.	0.
86	24	12	6.201	-7980.	14947.	3.200	0.	0.	6.308	22855.	0.	0.	0.
87	24	13	6.308	14947.	15121.	3.200	0.	0.	6.457	0.	0.	0.	0.
88	25	13	3.200	0.	0.	6.457	15121.	15264.	6.518	0.	0.	0.	0.
89	25	14	6.518	15264.	13190.	3.200	0.	0.	6.622	0.	2180.	0.	0.
90	26	14	3.200	0.	0.	6.622	13190.	6188.	6.678	0.	0.	-7210.	0.
91	27	14	3.200	0.	0.	6.678	6188.	1312.	6.708	0.	0.	-7464.	0.
92	27	15	6.708	-1312.	-1310.	3.200	0.	0.	6.758	0.	0.	0.	0.
93	-28	15	3.200	0.	0.	6.758	-1310.	-1310.	6.770	0.	0.	0.	0.

CHANNEL REACH 7

94	28	10	3.200	0.	0.	6.167	-12249.	-8707.	6.275	0.	0.	20729.	26265.
95	28	10	6.275	12249.	4078.	6.275	-8707.	-4078.	6.303	-40345.	-43033.	-3623.	-8250.
96	28	0	3.200	0.	0.	3.200	0.	0.	6.297	0.	0.	0.	0.
97	27	0	6.271	103.	-130.	6.297	-1249.	130.	6.306	-351.	0.	46643.	45335.
98	27	8	3.200	0.	0.	3.200	0.	0.	6.271	0.	0.	0.	0.
99	-28	8	3.200	0.	0.	6.271	-103.	0.	6.295	0.	0.	0.	0.

CHANNEL REACH 8

100	-1	4	3.200	0.	0.	5.751	0.	-5487.	5.802	0.	0.	-6551.	-958.
101	2	4	3.200	0.	0.	5.802	-5487.	-9014.	5.881	0.	0.	-8345.	-4704.
102	3	4	3.200	0.	0.	5.881	-9014.	-10935.	5.992	0.	0.	-11431.	-9411.
103	3	5	5.992	-10935.	-9793.	3.200	0.	0.	6.173	14330.	13274.	0.	0.
104	4	5	3.200	0.	0.	6.173	-9793.	-6448.	6.265	0.	0.	-18302.	-21673.
105	5	5	3.200	0.	0.	6.265	-6448.	-2200.	6.338	0.	0.	-47862.	-51939.
106	5	6	6.338	-2200.	4350.	3.200	0.	0.	6.466	31888.	25372.	0.	0.
107	5	7	6.466	4350.	0.	3.200	0.	0.	7.273	0.	0.	0.	0.
108	5	8	7.273	-21320.	-18045.	7.400	14477.	18045.	7.345	-13941.	-18634.	-46581.	-49765.
109	4	8	3.200	0.	0.	7.400	0.	0.	7.400	0.	0.	0.	0.
110	4	9	7.400	-14477.	-13041.	7.437	12490.	13041.	7.686	-1371.	40056.	-4913.	-8880.
111	3	9	7.437	-13041.	-3780.	3.200	0.	0.	7.937	67047.	69285.	0.	0.
112	3	8	3.200	0.	0.	7.937	1864.	-1072.	7.859	0.	0.	49891.	53580.
113	2	8	3.200	0.	0.	7.859	1221.	1864.	7.859	0.	15.	-487.	0.
114	-1	8	3.200	0.	0.	7.867	0.	1221.	7.875	0.	0.	18.	-1069.

CHANNEL REACH 9

115	3	10	7.937	-14776.	-18740.	8.809	11104.	18740.	8.522	550.	2676.	0.	-7978.
116	2	10	3.200	0.	0.	3.200	0.	0.	8.809	0.	0.	0.	0.
117	2	11	8.809	-11104.	-6372.	3.200	0.	0.	8.987	9906.	5134.	0.	0.
118	-2	12	8.987	-6372.	0.	3.200	0.	0.	9.062	0.	-6360.	0.	0.

CHANNEL REACH 10

119	6	8	8.925	-47088.	-36475.	7.395	0.	0.	7.171	-31380.	-42017.	0.	0.
120	7	8	6.971	0.	0.	7.171	-36475.	-33139.	7.694	0.	0.	-66590.	-69932.

CHANNEL REACH 11

121	7	5	5.500	-152503.	-158599.	6.024	0.	0.	5.784	137602.	144301.	0.	0.
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VOLUME OF WATER ABOVE PRL # 145950.6 MILLIONS OF CU FT
(THE SEPARATE PRLS IN P ARE EXCLUDED)

BEST AVAILABLE COPY

<p>Reid, Robert O. Development of SURGE II Program with application to the Sabine-Calcasieu area for Hurricane Carla and design hurricanes / by Robert O. Reid, Andrew C. Vastano...[et al.]. - Fort Belvoir, Va. : U.S. Coastal Engineering Research Center ; Springfield, Va. : available from National Technical Information Service, 1977. 218 p. : ill. (Technical paper - U.S. Coastal Engineering Research Center ; no. 77-13) Also (Contract - U.S. Coastal Engineering Research Center DAC64-74-C-0015) Bibliography: p. 117. SURGE II is a program for calculation of storm surges and tides in a bay or estuary of the type where frictional resistance dominates over Coriolis force. 1. Hurricanes. 2. Hurricane Carla. I. Title. II. Title: SURGE II Program. III. Vastano, Andrew C., joint author. IV. Series: U.S. Coastal Engineering Research Center. Technical paper no. 77-13. V. Series: U.S. Coastal Engineering Research Center. Contract DAC64-74-C-0015.</p>	<p>TC203 .U581tp no. 77-13 627</p>
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